

- 1. APPLICATION: THIS STANDARD SHALL BE USED FOR RAILROAD EMBANKMENTS EXPOSED TO OCEAN WAVES. THE RIP-RAP IS USED TO SECURE THE TRACK BALLAST FROM EROSION DUE TO WAVES, AS REQUIRED BY FEDERAL RAILROAD ADMINISTRATION TRACK SAFETY STANDARDS PART 213.103, PROTECTION OF THE BALLAST
- AND EMBANKMENT BEING FUNDAMENTAL IN SUPPORTING THE TRACK STRUCTURE.

  2. DIMENSION LINES: DIMENSIONS FOR STONE RIP-RAP ARE THE AVERAGE OF THE EXPOSED SURFACE OF ROCK. DUE TO THE IRREGULAR SIZE AND SHAPE OF NATURALLY BROKEN ROCK, ANY SPECIFIC POINT MAY VARY TWO FEET FROM THE AVERAGE DIMENSION SHOWN.
- RIP-RAP MATERIAL: GRANITE, BASALT OR SIMILAR IGNEOUS OR METAMORPHIC ROCK NATIVE TO ORANGE OR RIVERSIDE COUNTIES, BROKEN INTO SIZE DISTRIBUTION MEETING ASTM D5519 GRADATION WILL BE USED TO REPLACE ERODED RIP-RAP AREAS, HOWEVER EXISTING INVENTORIES OF LARGER ROCK MAY BE USED UNTIL EXHAUSTED. CONCRETE, ASPHALT, TIMBER OR METAL IS NOT PERMITTED IN THE RIP-RAP.

  A WALKWAY GENERALLY CONFORMING TO SCRRA ES2001 AND ES2002 WILL BE PROVIDED ON THE OCEAN SIDE OF THE TRACKS. THE MINIMUM WIDTH OF THE WALKWAY IN SURF AREAS IS EIGHT FEET AND SIX INCHES
- (8'-6") FROM THE CENTERLINE OF THE TRACK, WITH TWELVE FEET (12'-0") TO BE PROVIDED WHERE FIELD CONDITIONS PERMIT. WALKWAY SURFACE SHALL BE SUBBALLAST.

  MINIMUM AND MAXIMUM REPLACEMENT DIMENSIONS: THE GENERAL CRITERIA FOR INTIATING REPLACEMENT
- OF RIP-RAP IS WHEN EROSION OR SETTLEMENT HAS DEGRADED THE RIP-RAP SUCH THAT THE TOP OF THE RIP-RAP HAS BECOME LOWER THAN THE TOP OF RAIL ELEVATION (AND THEREFORE DOES NOT SHIELD THE TRACK FROM WAYES), WHEN THE THICKNESS OF THE RIP-RAP HAS DETERIORATED SUCH THAT THE NATURAL EMBANKMENT IS EXPOSED TO WAVE ACTION, OR WHEN THE LOWER PORTIONS OF THE RIP-RAP HAVE BECOME ERODED LEAVING AN UNSTABLE (STEEPER THAN 1:1) SLOPE RATIO. RIP-RAP WILL BE REPLENISHED TO THE "REPLACEMENT LINE" SHOWN, GENERALLY TO A 1.5:1 SLOPE RATIO. (AT LOCATIONS WITH WELL-ESTABLISHED LARGE DIMENSION RIP-RAP AT A STEEPER SLOPE, LOCALIZED SEGMENTS OF NEW RIP-RAP MAY BE INSTALLED AT 1:1 SLOPE RATIO). THE NORMAL STATE OF MAINTENANCE WILL BE GRADUALLY ERODING COVER OF RIP-RAP BETWEEN THE "MINIMUM" AND "REPLACEMENT" DIMENSION LINES.
- THE SCRRA AND LOCAL AGENCIES HAVE ESTABLISHED A "LIMIT LINE" TO DEFINE THE MAXIMUM WIDTH OF THE RIP-RAP. THIS LINE IS LOCATED BY REFERENCE TO GPS MEASURED COORDINATES, TO OFFSETS FROM TRACK CENTERLINE, OR BOTH. PLACEMENT OF RIP-RAP SHALL CONFORM TO THE LIMIT LINE UNLESS UNPRECEDENTED EROSION OF THE BEACH LOWERS THE LEVEL OF THE SAND, IN WHICH CASE THE LIMIT LINE WILL BE ADJUSTED SEAWARD AT A 1.5:1 (OR 1:1 AT LOCALIZED SITES) SLOPE RATIO FOR THE ADDED HEIGHT OF THE EMBANKMENT. AFTER RIP-RAP REPLACEMENT OPERATIONS ARE COMPLETE SCRRA WILL MAKE A SURVEY OF THE LIMIT LINE TO DETECT ANY DEVIATIONS FROM THE LIMIT LINE.

#### NOTES: (continued)

- THE BOTTOM OF THE RIP-RAP SHALL BE KEYED INTO THE BEACH SAND BY APPROXIMATELY THE SIZE OF THE RIP-RAP ROCK NOMINAL DIMENSION. EXISTING RIP-RAP OR NATIVE ROCK SHALL NOT BE EXCAVATED TO ESTABLISH A NEW KEY
- UNLESS REQUIRED TO ACHIEVE A STABLE STRUCTURE.

  THE ELEVATION OF THE TRACK SHALL BE MAINTAINED TO WHAT EXISTED UPON PURCHASE OF THE TRACK,
  HOWEVER TRACK RAISE NOT EXCEEDING 3 INCHES AT TIME OF TIE REPLACEMENT MAY BE PERFORMED.

  THE ELEVATION OF THE RIP-RAP SHALL REMAIN AS DIMENSIONED ON THIS STANDARD. IF THE ELEVATION OF THE BEACH SAND RISES OR FALLS, THE EFFECTIVE HEIGHT OF THE RIP-RAP SHALL BE ADJUSTED AT THE 1.5:1 OR 1:1 SLOPE RATIO SHOWN.
- RIP-RAP WILL BE PLACED BY GRAVITY DUMP FROM RAILROAD EQUIPMENT, FOLLOWED BY RE-STACKING WITH EQUIPMENT WORKING FROM THE BEACH THAT IS CAPABLE OF MOVING THE LARGEST ROCKS BEING USED. THE RE-STACKING IS TO PLACE ALL ROCKS IN A STABLE MATRIX, TO RECOVER ANY ROCKS BEYOND THE LIMIT LINE, AND TO FILL VOIDS BETWEEN LARGE ROCKS WITH SMALLER ROCK ELEMENTS. EXISTING RIP-RAP MAY BE MOVED PRIOR TO ADDITION OF REPLENISHMENT ROCK IN ORDER TO FACILITATE DUMPING.

  10. FOR EMBANKMENT DETAILS NOT SHOWN, REFER TO SCRRA ES2001 AND ES2002.

  11. AT LOCATIONS WHERE SAND MOVES TO COVER UP THE RIP-RAP, RIP-RAP SHALL BE LEFT IN PLACE.

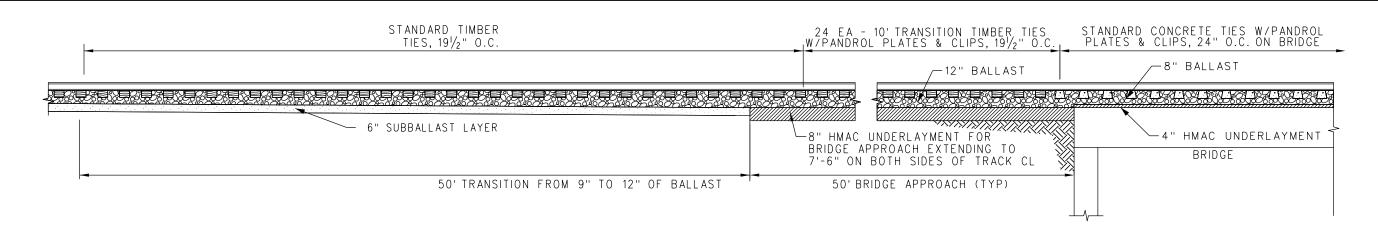
  12. SCRRA MAINTENANCE MANAGER WILL INFORM THE GOVERNING AGENCIES ONE MONTH IN ADVANCE OF

- PLANNED PLACEMENT OF REPLENISHMENT RIP-RAP. IF RAPID EROSION REQUIRES PLACEMENT IN LESS THAN THE FULL MONTH NOTIFICATION PERIOD. NOTICE WILL BE GIVEN AS PROMPTLY AS PRACTICABLE.
- INSTALLATION AND RE-STACKING OF ROCK SHAILL CONFORM TO PERMIT GUIDELINES AND SHALL BE PERFORMED ONLY AFTER PROVIDING PROTECTION FOR MEMBERS OF THE PUBLIC WHO MAY BE USING THE BEACH.

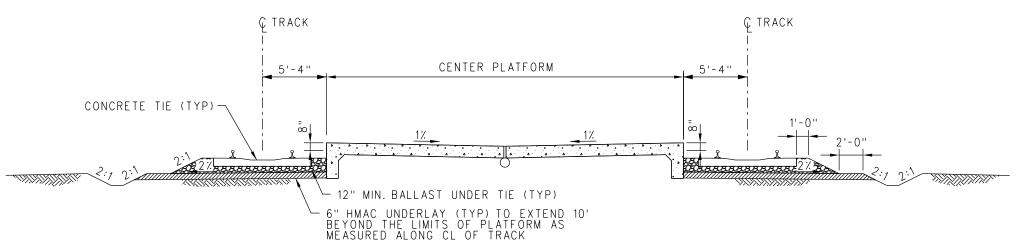
  ROUTINE REPLENISHMENT AND MAINTENANCE OF THE RIP-RAP SHALL BE SCHEDULED TO AVOID PEAK BEACH
- RECREATIONAL USE TIMES.
- LOCALIZED EXCEPTIONS TO THIS STANDARD SHALL BE MADE IN ORDER TO FIT RIP-RAP TO CONFORM TO DRAINAGE STRUCTURES, PUBLIC CROSSINGS, SIGNAL FACILITIES AND OTHER STRUCTURES

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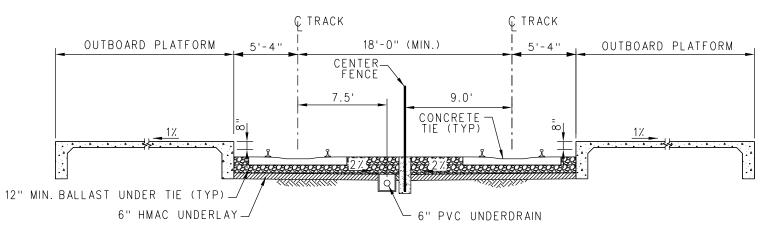
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### ON BRIDGES (TYPICAL FOR BOTH ENDS)



#### AT STATIONS (CENTER ISLAND PLATFORM)



### AT STATIONS (OUTBOARD PLATFORMS)

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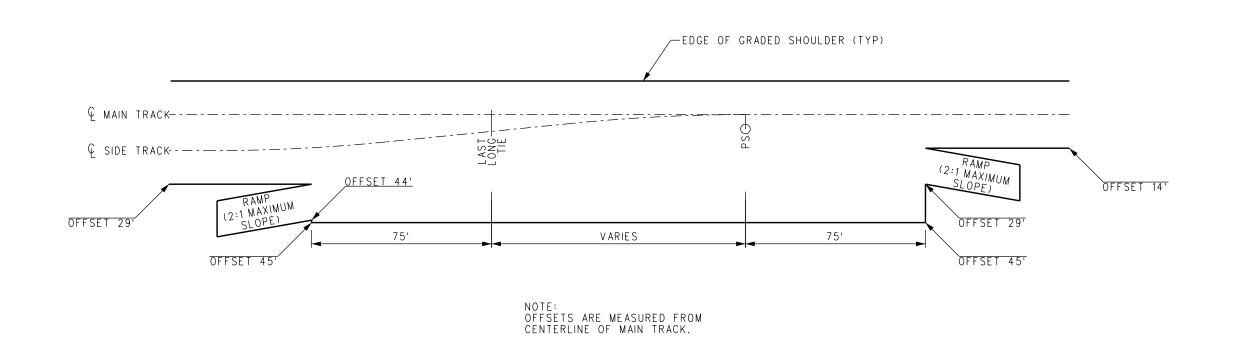
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ENGINEERING STANDARDS
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FOR SELECT CRITICAL LOCATIONS (NEW CONSTRUCTION ONLY)

NTS 1 OF 1 ES2004



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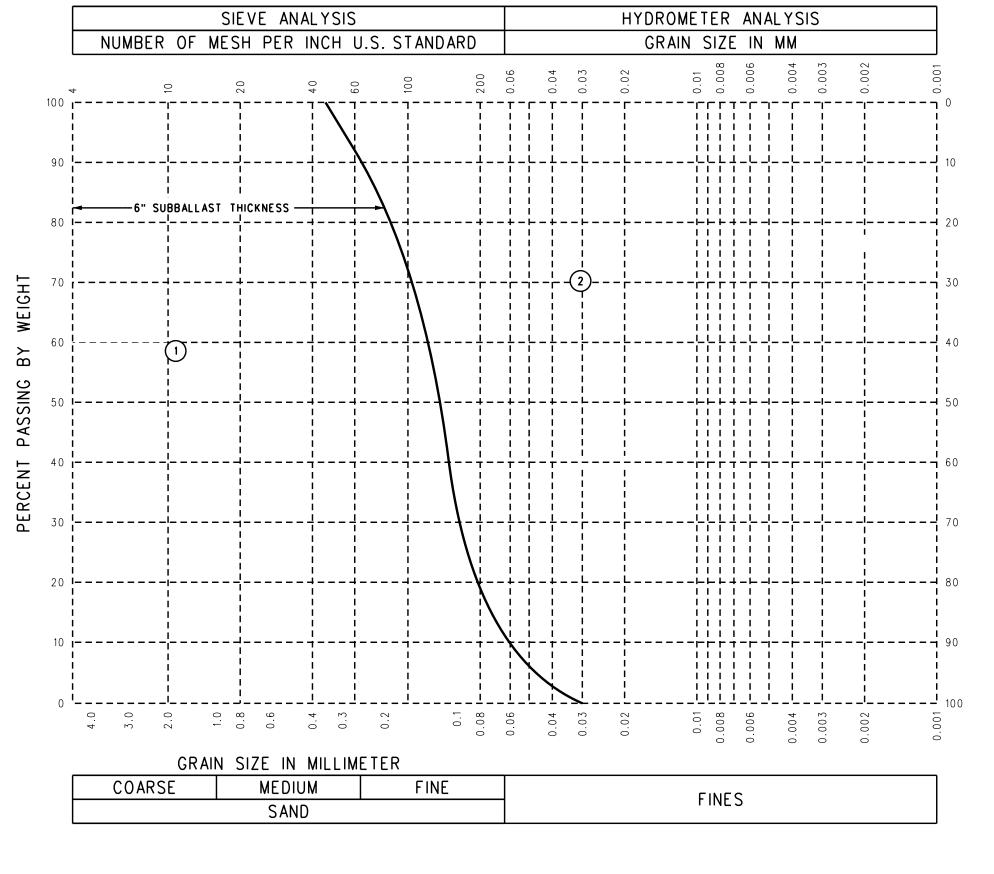
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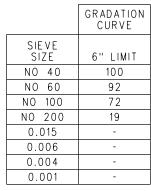
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PERCENT PASSING (BY WEIGHT)
[ALL AGGREGATE SAMPLING AND TESTING PER
ASTM LATEST REVISION.]

#### NOTES:

WEIGHT

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PERCENT

 SOILS ENGINEER WILL USE THESE REQUIREMENTS AS A MINIMUM. ADDITIONAL SUBGRADE SUPPORT MEASURES MAY BE NECESSARY AS DIRECTED BY SOIL ENGINEER'S ANALYSIS.

- ZONE OF SUBGRADE MATERIALS REQUIRING 6" OF SUBBALLAST.
- ADDITIONAL MEASURES MAY BE REQUIRED PER RECOMMENDATIONS OF AN ENGINEERING SOILS ANALYSIS.

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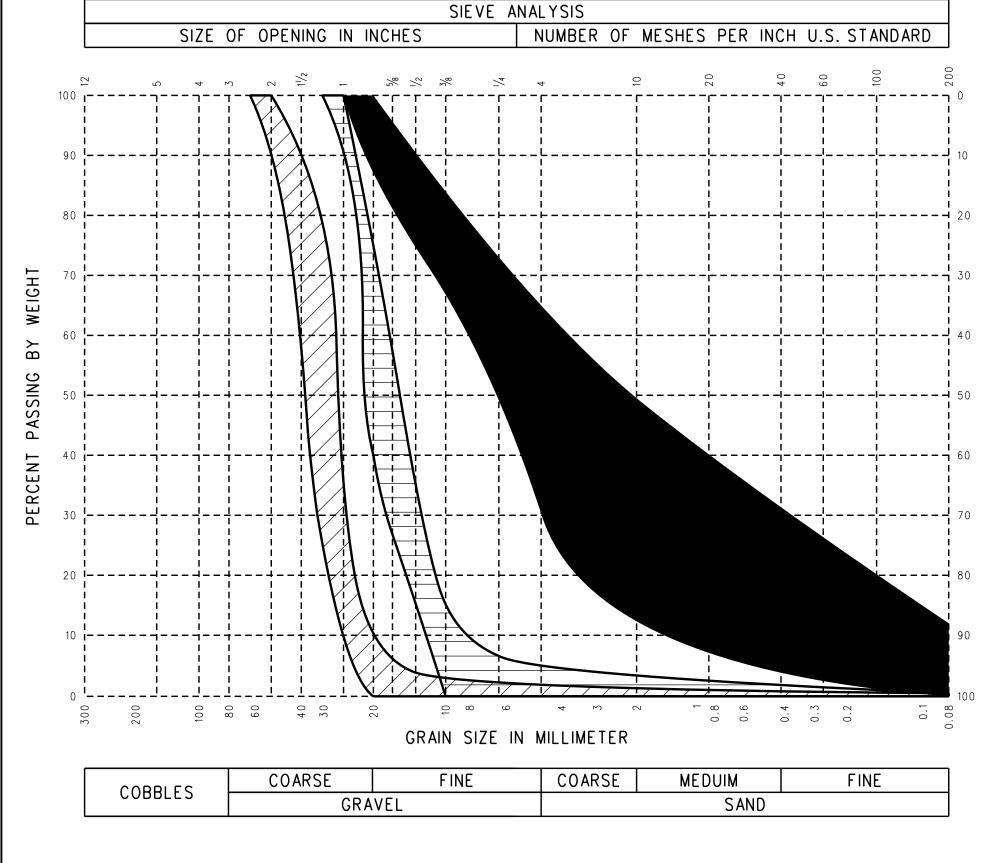
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GRAIN SIZE DISTRIBUTION FOR SUBGRADE SOILS

ENGINEERING STANDARDS

2007 NTS 1 OF 2 ES2007-01



	* 4 A	<b>#</b> 5	SUBBALLAST
SQUARE OPENING	2"-3/4"	1"-3/8"	3/4'' - 0''
21/2"	100	-	-
2"	90-100	-	-
13/4"	-	-	-
11/2"	60-90	-	-
11/4"	-	100	-
1''	10 - 35	90-100	100
3/4''	0 - 10	40-75	87-100
1/2"	-	15 - 35	-
3/8''	0 - 3	0 - 15	-
NO 4	-	0 - 5	30-65
NO 8	-	-	-
NO 10	-	-	-
NO 30	-	-	5-35
NO 200	0-0.5	0-0.5	0-12

PERCENT PASSING (BY WEIGHT)
[ALL AGGREGATE SAMPLING AND TESTING PER
ASTM LATEST REVISION.]

#### NOTES:

WEIGHT

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**RETAINED** 

PERCENT

1. FOR STANDARD CROSS SECTIONS, SEE SCRRA ES2001 & ES2002.

\*4A BALLAST FOR MAIN TRACK



\*5 BALLAST FOR WALKWAY AND YARD TRACK



SUBBALLAST CALTRANS 26-102A

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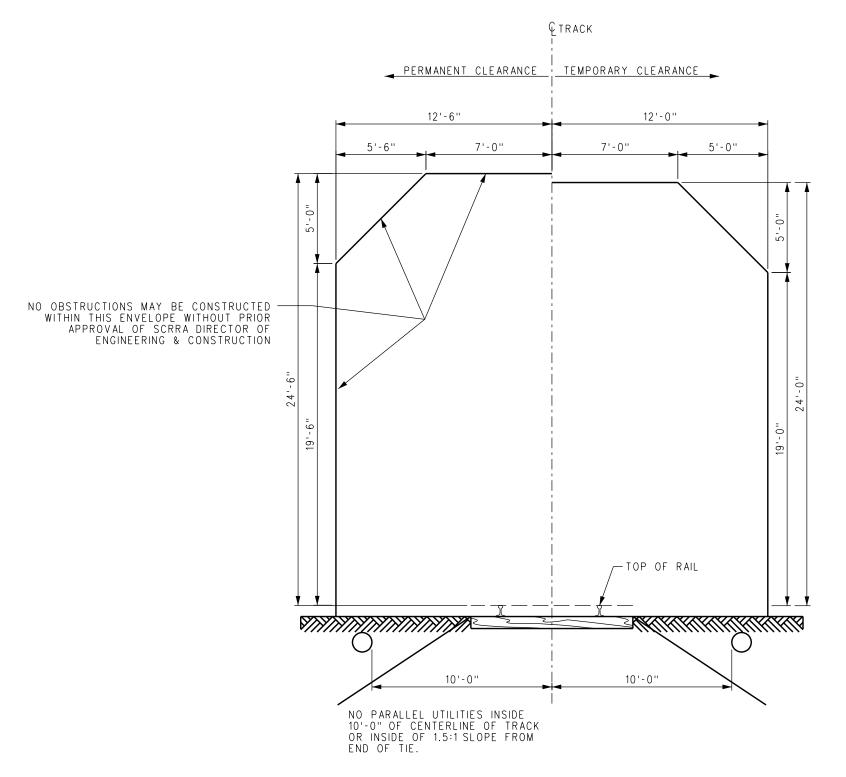
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BALLAST & SUBBALLAST GRADATION TABLE

ENGINEERING STANDARDS

2007 NTS \_2\_OF 2 ES2007-02



#### CLEARANCE REQUIREMENTS FOR NEW CONSTRUCTION OR DESIGN

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### ENGINEERING STANDARDS

 STANDARD PERMANENT AND TEMPORARY CLEARANCES SHOWN ON THIS SHEET SHALL BE USED FOR NEW DESIGN AND CONSTRUCTION WHEREVER PRACTICAL. ANY PERMANENT OR

SEE SCRRA ES2104 FOR MINIMUM VERTICAL CLEARANCES

PASSENGER PLATFORM CLEARANCES.

5. RAIL/HIGHWAY GRADE SEPARATIONS MAY REQUIRE PROVISIONS FOR A MAINTENANCE ROAD AND/OR FUTURE ADDITIONAL TRACK(S).

6. WIDER CLEARANCES MAY BE REQUIRED TO PROVIDE VISIBILITY

7. IN A CURVE ON SUPERELEVATED TRACK THE HORIZONTAL CLEARANCES SHALL BE MEASURED PERPENDICULAR TO THE PLANE

ACROSS THE TOP OF BOTH RAILS AND THE VERTICAL CLEARANCE

4. SEE SCRRA ES3101, ES3201 AND ES3202 FOR REQUIRED

SHALL BE MEASURED FROM THE HIGH RAIL.

TEMPORARY CONSTRUCTION PROPOSED WITHIN THE DIMENSIONS SHOWN SHALL REQUIRE THE PRIOR APPROVAL OF THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.

2. STANDARD PERMANENT CLEARANCE SHALL BE 14'-0" FROM CL OF TRACK FOR CANOPIES, STAIRWAYS AND SUPPORT COLUMNS. PROPOSED CLEARANCES LESS THAN THIS DISTANCE SHALL

CONFORM TO THOSE SHOWN ON SCRRA ES2102 AND WILL REQUIRE

THE PRIOR APPROVAL OF THE SCRRA DIRECTOR OF ENGINEERING

NOTES:

AND CONSTRUCTION.

FOR OVERHEAD WIRES.

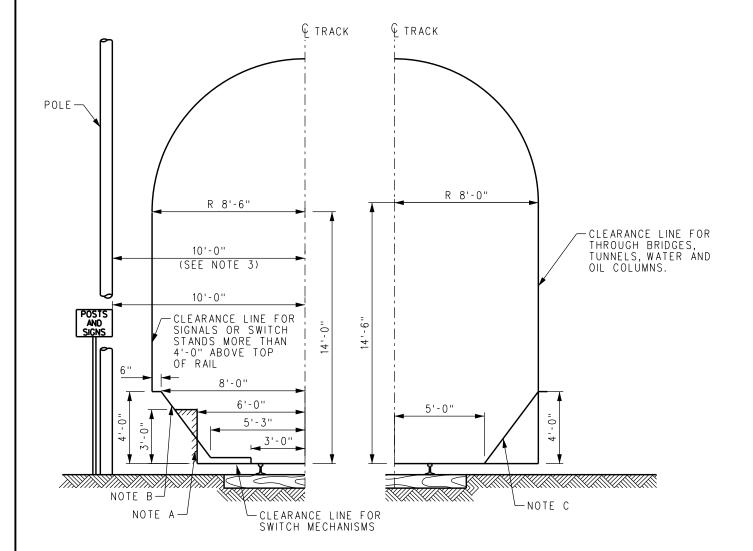
FOR WAYSIDE SIGNALS.

STANDARD CLEARANCE OF STRUCTURES

REVISION SHEET 1 OF 1 CADD FILE: ES2101

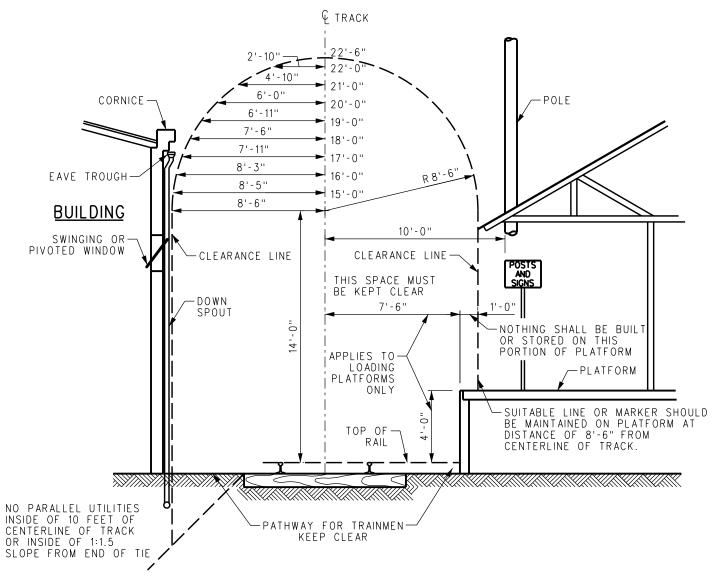
- A. CLEARANCE LINE SHOWN BELOW IS FOR SIGNALS OR SWITCH STANDS 3'-O" OR LESS ABOVE TOP OF RAIL AND LOCATED BETWEEN TRACKS WHERE NOT PRACTICABLE TO MAINTAIN CLEARANCES OTHERWISE PRESCRIBED.
- CLEARANCE LINE SHOWN BELOW IS FOR PORTIONS OF BLOCK SIGNALS 4'-0" OR LESS ABOVE TOP OF RAIL.
- DECREASED CLEARANCES SHOWN BELOW ARE FOR: 1) REFUGE PLATFORMS ON BRIDGES AND TRESTLES NOT PROVIDED WITH WALKWAYS 2) HANDRAILS

MINIMUM CLEARANCES FOR HANDRAILS ON BRIDGES WITH WALKWAYS SHALL BE 8'-6". DECREASED CLEARANCES, EXCEPT AS PROVIDED FOR HANDRAILS ARE NOT PERMITTED ON THROUGH BRIDGES WHERE WORK OF TRAINMEN OR YARDMEN REQUIRE THEM TO BE ON DECK OF BRIDGE FOR PURPOSE OF COUPLING OR UNCOUPLING CARS IN PERFORMING SWITCHING SERVICE ON A SWITCHING LEAD.



#### NOTES:

- SEE SCRRA ES2104 FOR MINIMUM VERTICAL CLEARANCES FOR OVERHEAD WIRES.
- ALL CLEARANCES LISTED ON THIS SHEET ARE MINIMUM REQUIREMENTS. USE
- STANDARD CLEARANCES SHOWN ON SCRRA ES2101 FOR NEW CONSTRUCTION.
  POSTS, POLES, SIGNS AND SIMILAR FACILITIES MAY HAVE MINIMUM CLEARANCE OF
  8'-6", BUT CLEARANCE OF 10'-0" IS RECOMMENDED WHERE PRACTICABLE
  ALL SIDE CLEARANCE DIMENSIONS ARE FOR TANGENT TRACK. IN GENERAL, SIDE CLEARANCE
- FOR CURVED TRACK SHALL BE 1'-0" GREATER THAN THAT FOR TANGENT TRACK.
  PLATFORMS 4'-0" OR LESS IN HEIGHT WITH MINIMUM CLEARANCE OF 7'-3" MAY BE
  EXTENDED AT EXISTING CLEARANCES IF SUCH EXTENSION IS NOT IN CONNECTION WITH RECONSTRUCTION OF ORIGINAL PLATFORM.



#### **TYPICAL** CLEARANCE OF STRUCTURES FROM RAILROAD TRACKS AS GENERALLY PRESCRIBED BY PUBLIC UTILITIES COMMISSION - STATE OF CALIFORNIA GENERAL ORDER NO 26-D

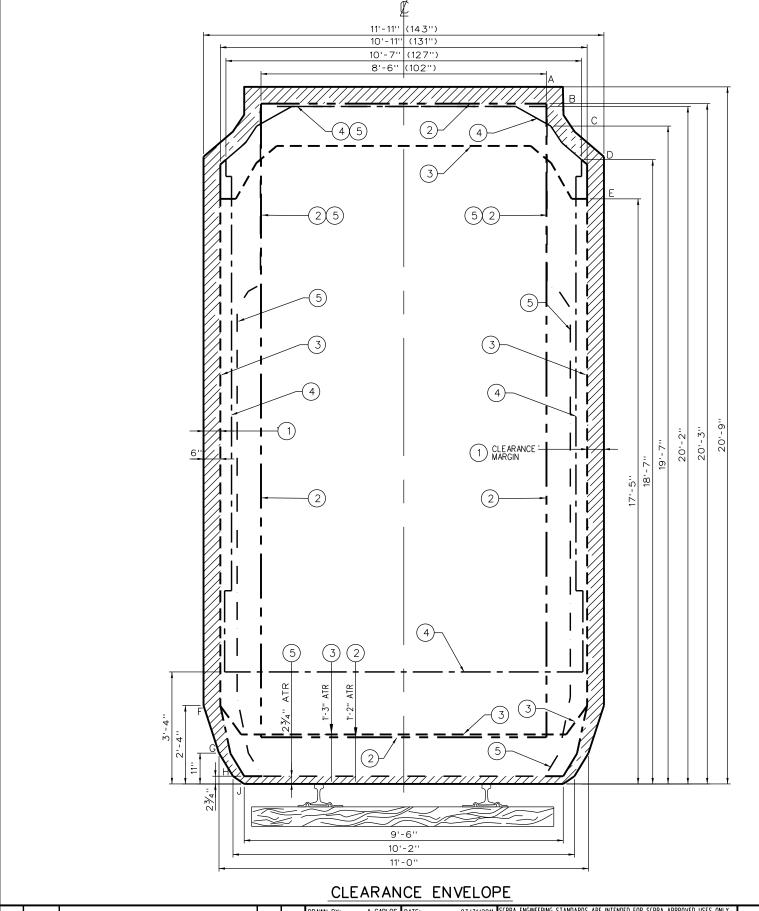
(EFFECTIVE FEBRUARY 1, 1948) FOR NEW WORK AND RECONSTRUCTION OF EXISTING FACILITIES ADJACENT TO STANDARD GUAGE RAILROAD TRACKS TRANSPORTING FREIGHT CARS.

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ENGINEERING STANDARDS	STANDARD 2102
	scale: NTS
MINIMUM CLEARANCE OF STRUCTURES	REVISION SHEET - 1 OF 1
	CADD FILE:



#### LEGEND FOR CLEARANCE ENVELOPE

CLEARANCE MARGIN FOR MAXIMUM DOUBLESTACK CONTAINERS, BI-LEVEL AND TRI-LEVEL CARRIERS. THIS AREA TO BE KEPT FREE AND CLEAR OF ANY PLATFORMS, TUNNELS, BRIDGE OVERHEADS, PASSENGER PLATFORMS, POLES, UTILITY LINES, WAYSIDE SIGNAL DEVICES, AND ALL OTHER NATURAL OR MAN-MADE STRUCTURES AND OBJECTS.

— — — MAXIMUM COMBINATION DOUBLESTACK CARS (8'-6" WIDE BY 9'-6½" HIGH CONTAINERS STACKED TWO HIGH, 1'-2" ATR).

(3) ----- ARTICULATED BI-LEVEL AUTO CARRIER CAR.

TRI-LEVEL AUTO CARRIER CAR (CHRYSLER TYPE).

AAR PLATE H CLEARANCE ENVELOPE (FOR DOUBLESTACK CARS WITHOUT CONTAINERS).

#### NOTES:

- 1. ALL NEW CONSTRUCTION, RECONSTRUCTION, ALTERATIONS AND MODIFICATIONS MUST BE IN COMPLIANCE WITH THE CLEARANCE ENVELOPE REQUIREMENTS FOR UNOBSTRUCTED TRANSPORT OF THIS RAIL EQUIPMENT.
- 2. HORIZONTAL CLEARANCE DISTANCES SHALL BE INCREASED ON CURVES AT RATE OF 1.07" ON INSIDE OF CURVES AND 1.05" ON OUTSIDE OF CURVES PER DEGREE OF CURVE.
- 3. WHEN TRACK SUPERELEVATION IS SET APPROPRIATELY FOR THE AUTHORIZED TRAIN SPEED, ALL CLEARANCE MEASUREMENTS ARE TO BE MADE PARALLEL TO THE PLANE OF THE TOP OF RAIL AND PERPENDICULAR TO THE CENTERLINE OF TRACK.
- 4. DIMENSIONS SHOWN ARE FOR INFORMATION ONLY AND NOT TO BE USED TO ESTABLISH LEGAL CLEARANCE REQUIREMENTS OR FOR HIGH-WIDE LOAD
- 5. IN MANY INSTANCES, STATE LAW MAY REQUIRE GREATER CLEARANCE THAN PROVIDED FOR IN THE COMBINED CLEARANCE ENVELOPE, IN WHICH CASE THE GREATER CLEARANCE SHALL GOVERN.
- 6. CLEARANCE DIMENSION REQUIREMENTS INDICATED EXCEED MOST STATES PERMISSIVE CLEARANCES FOR LOW PLATFORMS HOWEVER, THESE CLEARANCE STANDARDS SHALL GOVERN FOR 8 INCHES OR LOWER PLATFORMS.
- 7. THE PRESCRIBED CLEARANCE MARGIN ENVELOPE MAY BE MODIFIED WHEN APPROVED BY THE DIRECTOR OF ENGINEERING AND CONSTRUCTION.

Jares D. GAR ASSISIANT DIRECTOR: STANDARDS & DESIGN ) ellrom X XX-XX-X REVISION REV. DATE DESCRIPTION DES. ENG.

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**METROLINK** 

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

**ENGINEERING STANDARDS** 

CAR (EQUIPMENT) CLEARANCE ENVELÓPE

NONE 1 OF ES2103

### TABLE 1: CPUC GENERAL ORDER 95 BASIC MINIMUM ALLOWABLE VERTICAL CLEARANCE OF WIRES ABOVE RAILROADS, ROADWAYS, POLES, BUILDINGS, STRUCTURES OR OTHER OBJECTS

		A	В	С	D	E	F
C A S E NO	NATURE OF CLEARANCE	SPAN WIRES (OTHER THAN TROLLEY SPAN WIRES) OVERHEAD GUYS AND MESSENGERS	COMMUNICATION CONDUCTORS (INCLUDING OPEN WIRE, CABLES & SERVICE DROPS)	TROLLEY CONTACT, FEEDER AND SPAN WIRES, UNDER 5 KV	SUPPLY CONDUCTORS & SUPPLY CABLES UNDER 22.5 KV, SIGNAL WIRES	SUPPLY CONDUCTORS & SUPPLY CABLES, 22.5 KV TO UNDER 300 KV	SUPPLY CONDUCTORS & SUPPLY CABLES, 300 KV AND GREATER
1	CROSSING ABOVE TRACKS OF RAILROADS WHICH TRANSPORT OR PROPOSE TO TRANSPORT FREIGHT CARS (MAXIMUM HEIGHT 20'-9" WHERE NOT OPERATED BY OVERHEAD CONTACT WIRES.	25-FT	28-FT*	22.5-FT	28-FT	34-FT	34-FT
2	CROSSING OR PARALLELING ABOVE TRACKS OF RAILROAD OPERATED BY OVERHEAD TROLLEYS.	26-FT	26-FT	22.5-FT	30-FT*	34-FT*	34-FT*
3	CROSSING OR ALONG ROADWAYS IN URBAN DISTRICTS OR CROSSING ROADWAYS IN RURAL DISTRICTS.	18 - F T	18 - F T	19 - F T	25-FT	30-FT	30-FT
4	ABOVE GROUND ALONG ROADWAYS IN RURAL DISTRICTS OR ACROSS OTHER AREAS CAPABLE OF BEING TRAVERSED BY VEHICLES OR AGRICULTURAL EQUIPMENT.	18 - F T*	18-FT*	19-FT	25-FT	30-FT	30-FT
5	ABOVE GROUND IN AREAS ACCESSIBLE TO PEDESTRIANS ONLY.	8-FT	10 - F T	19 - F T	17 - F T	25-FT	25-FT
6	VERTICAL CLEARANCE ABOVE WALKABLE SURFACES ON BUILDINGS, (EXCEPT GENERATING PLANTS OR SUBSTATIONS) BRIDGES OR OTHER STRUCTURES WHICH DO NOT ORDINARILY SUPPORT CONDUCTORS, WHETHER ATTACHED OR UNATTACHED.	8-FT	8-FT	8-FT	12 - F T	12 - F T	20-FT
6A	VERTICAL CLEARANCE ABOVE NON-WALKABLE SURFACES ON BUILDINGS, (EXCEPT GENERATING PLANTS OR SUBSTATIONS) BRIDGES OR OTHER STRUCTURES, WHICH DO NOT ORDINARILY SUPPORT CONDUCTORS, WHETHER ATTACHED OR UNATTACHED.	2 - F T	8-FT	8-FT	8-FT	8-FT	20-FT
7	HORIZONTAL CLEARANCE OF CONDUCTOR AT REST FROM BUILDINGS (EXCEPT GENERATING PLANTS AND SUBSTATIONS), BRIDGES OR OTHER STRUCTURES (UPON WHICH MEN MAY WORK) WHERE SUCH CONDUCTOR IS NOT ATTACHED THERETO.	-	3-FT	3-FT	6-FT	6-FT	15 - F T
8	DISTANCE OF CONDUCTOR FROM CENTER LINE OF POLE, WHETHER ATTACHED OR UNATTACHED.	-	15 - IN	15 - IN	18 - IN	18 - IN	NOT APPLICABLE
9	DISTANCE OF CONDUCTOR FROM SURFACE OF POLE CROSS ARM OR OTHER OVERHEAD LINE STRUCTURE UPON WHICH IT IS SUPPORTED, PROVIDING IT COMPLIES WITH CASE 8 ABOVE.	-	3-IN	3-IN	3 - IN	3-IN	NOT APPLICABLE

### MINIMUM CLEARANCES OF WIRES ABOVE RAILROADS, ROADWAYS, ETC.

1. CLEARANCES BETWEEN OVERHEAD CONDUCTORS, GUYS, MESSENGERS OR TROLLEY SPAN WIRES AND TOPS OF RAILS, SURFACES OF ROADWAYS OR OTHER GENERALLY ACCESSIBLE AREAS ACROSS, ALONG OR ABOVE WHICH ANY OF THE FORMER PASS; ALSO THE CLEARANCES BETWEEN CONDUCTORS, GUYS, MESSENGERS OR TROLLEY SPAN WIRES AND BUILDINGS, POLES, STRUCTURES, OR OTHER OBJECTS, SHALL NOT BE LESS THAN THOSE SET FORTH IN TABLE 1, AT A TEMPERATURE OF 60°F AND NO WIND.

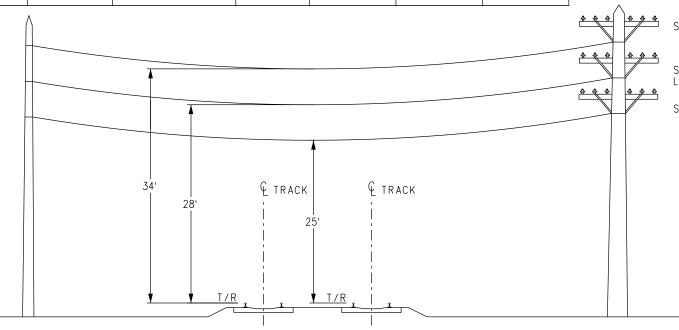
2. THE CLEARANCES SPECIFIED IN TABLE 1, CASE 1, COLUMNS A, B, D, E AND F, SHALL IN NO CASE BE REDUCED MORE THAN 5% BELOW THE TABULAR VALUES BECAUSE OF TEMPERATURE AND LOADING AS SPECIFIED IN CPUC G.O. 95 RULE 43. THE CLEARANCES SPECIFIED IN TABLE 1, CASES 2 TO 6 INCLUSIVE, SHALL IN NO CASE BE REDUCED MORE THAN 10% BELOW THE TABULAR VALUES BECAUSE OF

TEMPERATURE AND LOADING AS SPECIFIED IN CPUC G.O. 95 RULE 43.

THE CLEARANCE SPECIFIED IN TABLE 1, CASE 1, COLUMN C (22.5 FEET), SHALL IN NO CASE BE REDUCED BELOW THE TABULAR VALUE BECAUSE OF TEMPERATURE AND LOADING AS SPECIFIED IN RULE 43.

4. WHERE SUPPLY CONDUCTORS ARE SUPPORTED BY SUSPENSION INSULATORS AT CROSSINGS OVER RAILROADS WHICH TRANSPORT FREIGHT CARS, THE INITIAL CLEARANCES SHALL BE SUFFICIENT TO PREVENT REDUCTION TO CLEARANCES LESS THAN 95% OF THE CLEARANCES SPECIFIED IN TABLE 1, CASE 1, THROUGH THE BREAKING OF A CONDUCTOR IN EITHER OF THE ADJOINING SPANS.

\*EXCEEDS CPUC GENERAL ORDER 95 BASIC MINIMUM ALLOWABLE VERTICAL CLEARANCE



SUPPLY LINES 22.5 KV AND GREATER

SUPPLY LINES UNDER 22.5 KV, SIGNAL LINES, AND COMMUNICATIONS CONDUCTORS

SPAN WIRES, OVERHEAD GUYS, AND MESSENGERS

### MINIMUM CLEARANCES OF WIRES ABOVE RAILROADS - CASE 1

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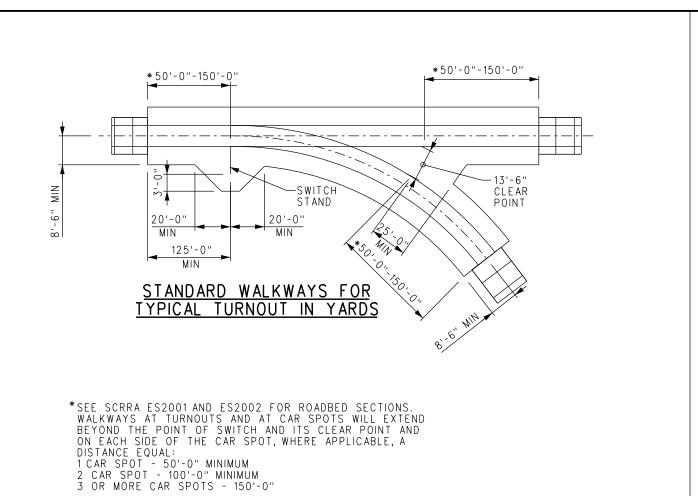
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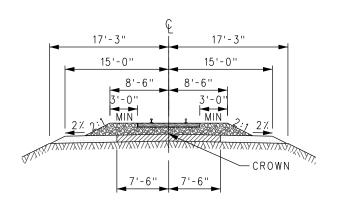
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ENGIN	NEERING	STANDAF	RDS
MINIMUM VE	RTICAL CLEA	ARANCE FOR	WIRES





#### TYPICAL HMAC UNDERLAYMENT SECTION IN YARDS

### 15'-9"\*\* 15'-9"\*\* 13'-6"\*\*\* 13'-6"\*\*\* -CROWN 7'-6"

#### TYPICAL HMAC UNDERLAYMANT SECTION ON MAINLINES

\*\*\*16'-3" IN CONCRETE TIE TERRITORY
\*\*\*14'-0" IN CONCRETE TIE TERRITORY

#### LEGEND



BALLAST

SUBBALLAST



HOT MIX ASPHALT CONCRETE



SUBGRADE

A. CARLOS DATE: A 11-11-16 REVISED TABLES REV. DATE DESCRIPTION DES. ENG.

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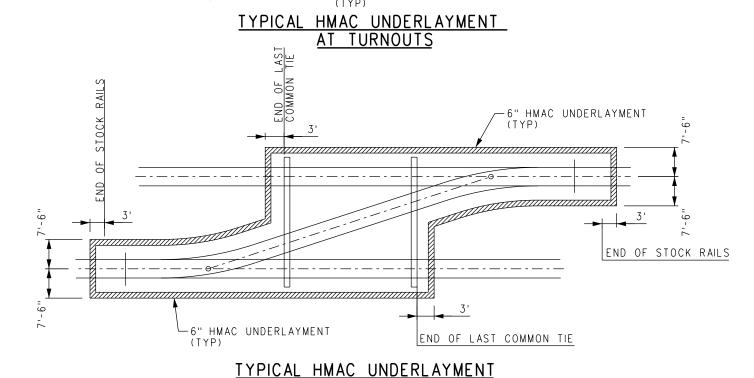
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### Ε

ENGINEERING STANDARDS	2105
	SCALE: NTS
TURNOUT WALKWAYS AND HMAC UNDERLAYMENT	REVISION SHEET  A 1 OF 1

ES2105



6" HMAC UNDERLAYMENT

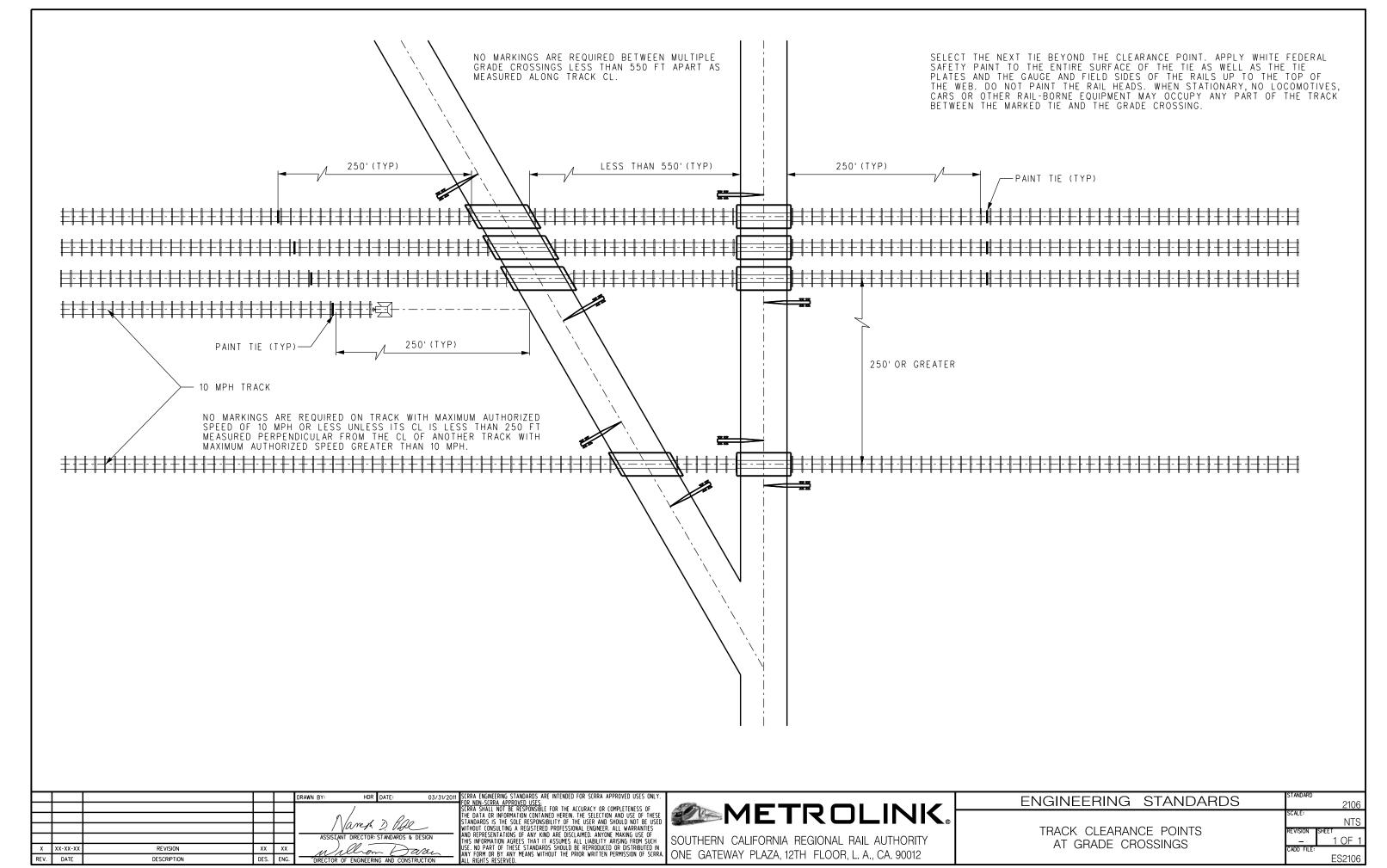
TURNOUT NO	AREA OF HMAC (SF)	VOLUME OF 6" HMAC (CY)	*WEIGHT OF HMAC (TONS)
10	2400	45	78
14	3340	62	109
20	4640	86	151
24	6100	113	198

AT CROSSOVERS

HMAC VOLUME QUANTITIES

CROSSOVER	TRACK CENTER	AREA OF HMAC	VOLUME OF 6" HMAC	*WEIGHT OF HMAC
NO	DISTANCE (FT)	(SF)	(CY)	(TONS)
10	15	4490	84	146
14	15	6210	115	202
20	15	8760	163	285
24	15	11,780	218	383

*	CALCULATIONS FOR	THEORET	ICAL WEIGH	IT OF HMAC
	BASED ON UNIT WEIG	GHT OF 1	30 LBS P	ER CUBIC FEET
	THE QUANTITY WILL	BE INCR	EASED IF	TRACK CENTER
	DISTANCE IS INCREAS	SED.		



UserName=> carlosa

#### INSTRUCTIONS FOR MARKING NO RIDE ZONE FOR SIDE AND SECONDARY TRACKS (BASED ON 13'-6" CLEARANCE POINT)

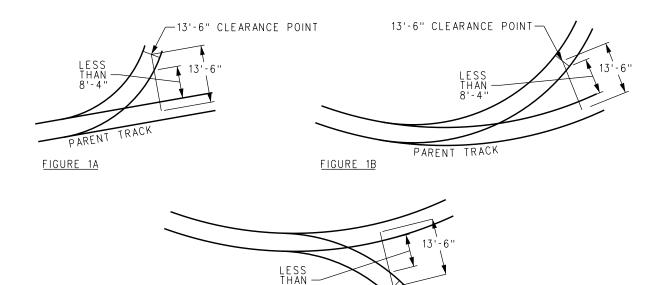
#### CASE 1 DIVERGING TRACKS

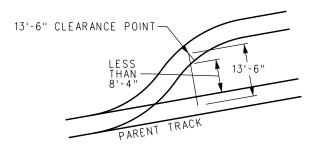
WHERE A TRACK TURNS OUT AND CONTINUES TO DIVERGE FROM THE PARENT TRACK, THE 13'-6" CLEARANCE POINT SHALL BE WHERE THE DISTANCE BETWEEN THE FIELD SIDES OF THE TWO CLOSEST RAILHEADS IS 8'-4" MEASURED PERPENDICULAR TO THE CL OF THE PARENT TRACK.

#### CASE 2 PARALLEL TRACKS - TANGENT OR CURVED

WHERE A TRACK TURNS OUT AND BECOMES PARALLEL TO THE PARENT TRACK, THE 13'-6" CLEARANCE POINT SHALL BE WHERE THE DISTANCE BETWEEN THE FIELD SIDES OF THE TWO CLOSEST RAILHEADS IS 8'-4" MEASURED PERPENDICULAR TO THE CL OF THE PARENT TRACK. SEE FIGURES 2A AND 2C.

WHERE TRACKS ARE PARALLEL, BUT THE FIELD SIDES OF THE TWO CLOSEST RAILS ARE LESS THAN 8'-4" APART, THE CLEARANCE POINT SHALL BE WHERE THE TRACKS BECOME PARALLEL. SEE FIGURES 2B AND 2D.





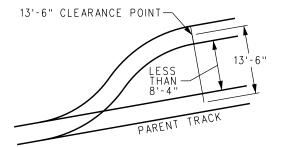
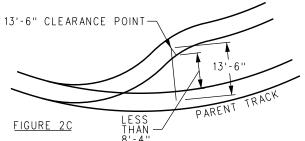
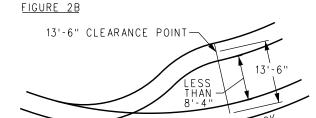


FIGURE 2A





PARENT

FIGURE 2D

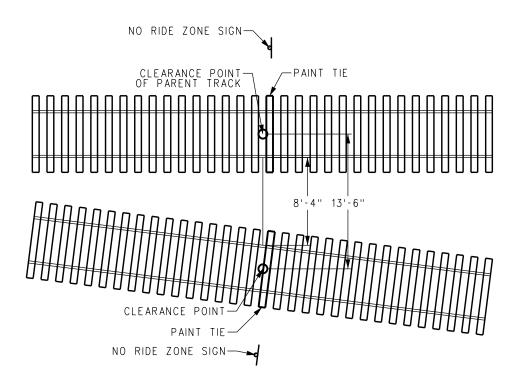


FIGURE 1C

13'-6" CLEARANCE POINT

#### SELECTING AND MARKING NO RIDE ZONE

SELECT THE NEXT TIES BEYOND THE NO RIDE POINT AS DETERMINED BY CASE 1 OR 2. APPLY WHITE PAINT TO THE ENTIRE TOP SURFACE OF THE TIES AS WELL AS THE TIE PLATES AND THE GAUGE AND FIELD SIDES OF THE RAILS UP TO THE TOP OF THE WEB. DO NOT PAINT THE RAIL HEADS.



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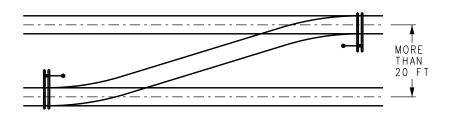
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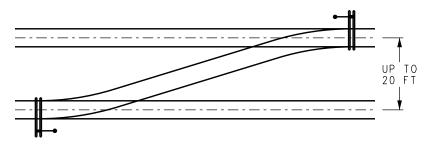
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ENGINEERING STANDARDS	STANDARD 2107
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TRACK NO RIDE ZONE AT TURNOUTS	REVISION SHEET  - 1 OF 1
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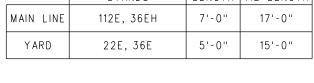
#### TRACKS MORE THAN 20 FT ON CENTER (INSIDE PLACEMENT)

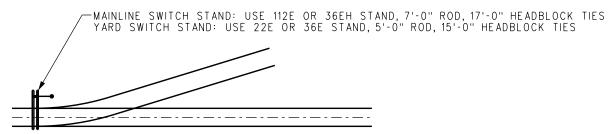




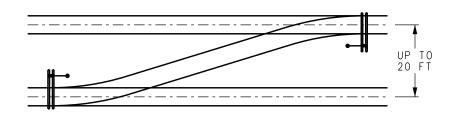
TRACKS 13 FT TO 20 FT ON CENTER (OUTSIDE PLACEMENT)

	S WIT CH S T AND S	ROD LENGTH	HEADBLOCK TIE LENGTH
MAIN LINE	112E, 36EH	7'-0"	17'-0"
YARD	22E, 36E	5'-0"	15'-0"





TYPICAL ALIGNMENT WITH NO CLEARANCE RESTRICTIONS



#### TRACKS 13 FT TO 20 FT ON CENTER (INSIDE PLACEMENT)

S WIT CH STANDS			HEADBLOCK TIE LENGTH	
MAIN LINE	36E WITH SWITCH HANDLE	3'-4"	14'-0"	
YARD	22E, 36E WITH SWITCH HANDLE	3'-4"	14'-0"	

#### NOTES:

- 1. SWITCH STANDS SHALL BE:

  A. WHERE SPACE PERMITS, MOUNTED ON THE CLOSED POINT SIDE
  OF THE SWITCH WHEN LINED FOR THE MAIN TRACK.

  B. NO LESS THAN 8'-6" (HIGH STANDS) OR 6'-0" (LOW STANDS)
  FROM THE CENTER OF ANY TRACK TO ANY PART OF THE STAND
  OR TARGET IN ITS MOST RESTRICTIVE POSITION.
  C. POSITIONED WITH THE HANDLE POINTING TOWARD THE FROG
  WHEN THE SWITCH IS LINED FOR THE MAIN TRACK.
  D. FIRMLY ATTACHED TO THE HEADBLOCK TIES.
  2. WHERE TRACKS ARE 20 FT OR LESS ON CENTER, OUTSIDE
  PLACEMENT OF SWITCH STANDS IS PREFERRED. INSIDE
  PLACEMENT SHALL BE USED ONLY WHERE FIELD CONDITIONS
  MAKE OUTSIDE PLACEMENT IMPRACTICAL. MAKE OUTSIDE PLACEMENT IMPRACTICAL.

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					/ Varen D. Pall
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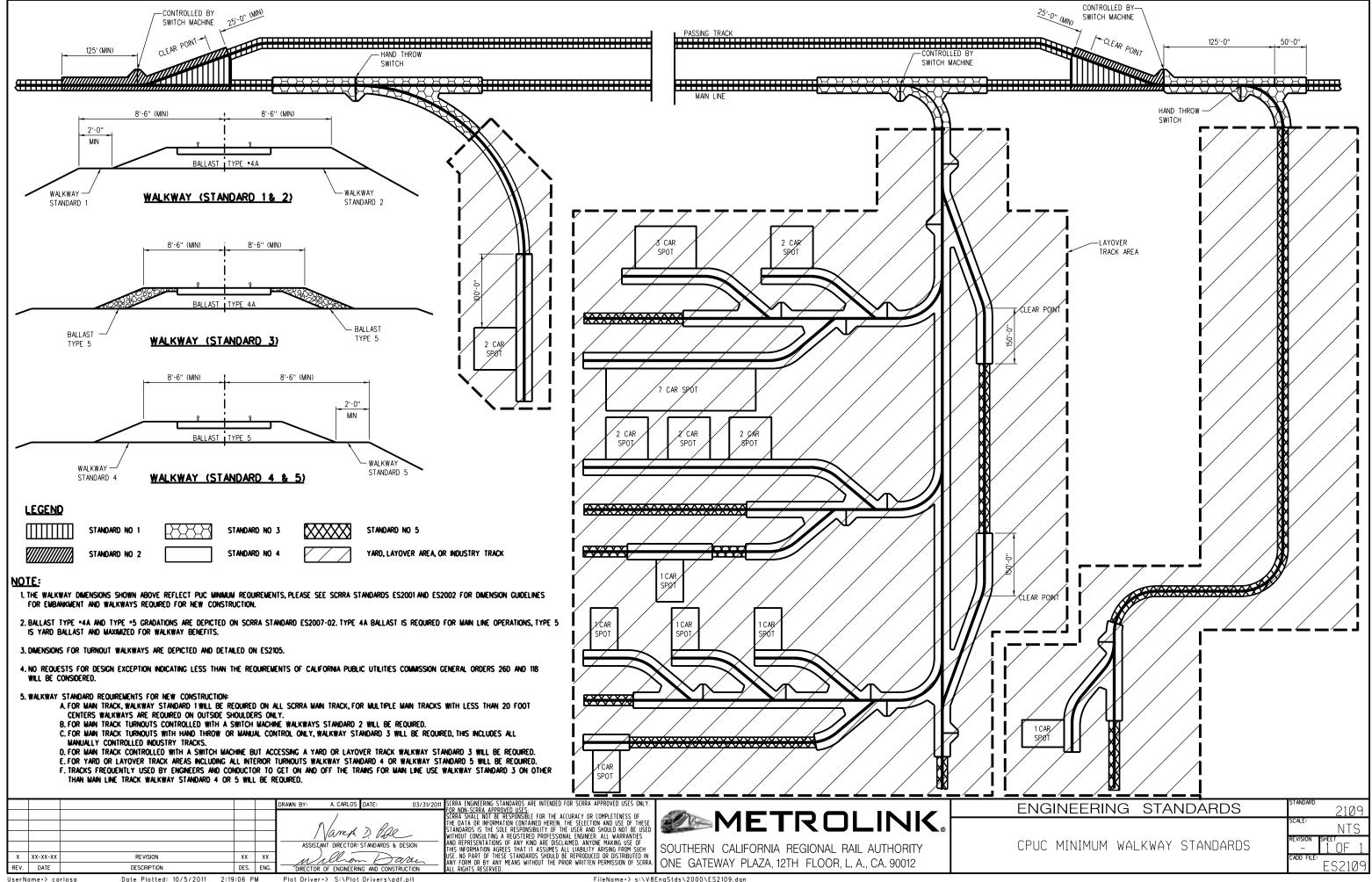
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ENGINEERING STANDARDS	STANDARD	2108
	SCALE:	NTS
SWITCH STAND PLACEMENT	REVISION —	SHEET 1 OF 1
	CADD FILE:	ES2108



#### STANDARD VERTICAL CURVES (AREMA SECTION 3.6)

- VERTICAL CURVES AS CALCULATED IN ITEM 6 BELOW SHALL BE USED TO CONNECT ALL CHANGES IN GRADIENTS.
- 2. THE LENGTH OF VERTICAL CURVES IS DETERMINED BY CHANGES IN GRADIENT, VERTICAL ACCELERATION AND THE SPEED OF THE TRAIN.
- THE PURPOSE OF VERTICAL CURVES IS TO EASE THE CHANGE OF THE GRADIENTS IN ORDER TO REDUCE COUPLER AND DIAPHRAGM BINDING AND ELIMINATE THE DANGER OF BREAKING THE TRAIN IN TWO AS A DIRECT RESULT OF TRAIN ACTION. PROPERLY DESIGNED VERTICAL CURVES WILL PROVIDE FOR PASSENGER COMFORT. VERTICAL CURVES SHALL BE DESIGNED LONG ENOUGH TO MATCH THE HIGHEST SPEEDS CONTEMPLATED FOR THE LINES.
- 4. A VERTICAL CURVE WHICH IS CONCAVE UPWARD SHALL BE DENOTED AS A SAG. A VERTICAL CURVE WHICH IS CONCAVE DOWNWARD SHALL BE DENOTED AS A SUMMIT (SEE DIAGRAMS BELOW).
- VERTICAL CURVES SHALL BE PARABOLIC
- THE MINIMUM LENGTH OF VERTICAL CURVES FOR BOTH SAGS AND SUMMITS IS DETERMINED BY THE FOLLOWING FORMULA

$$LVC = D \times V^2 \times K$$

WHERE:

- A = VERTICAL ACCELERATION IN FEET/SEC/SEC (FT/SEC<sup>2</sup>)
- D = ABSOLUTE VALUE OF THE DIFFERENCE IN RATES OF GRADES EXPRESSED AS A DECIMAL
- K = 2.15 CONVERSION FACTOR TO GIVE LVC IN FEET
- V = DESIGN SPEED IN MILES PER HOUR

IT IS RECOMMENDED PRACTICE TO ROUND THE CALCULATED MINIMUM LVC UP TO A CONVENIENT WHOLE NUMBER. ON TRACKS WITH DESIGN SPEEDS GREATER THAN OR EQUAL TO 25 MPH, ANY CALCULATED MINIMUM LVC OF LESS THAN 100 FT SHALL BE ROUNDED UP TO AT LEAST 100 FT

THE RECOMMENDED VERTICAL ACCELERATION (A) SHALL BE SELECTED BASED ON THE TYPE OF OPERATIONS AND IS THE SAME FOR BOTH SAGS AND SUMMITS. DEVIATIONS FROM THESE ACCELERATION CRITERIA MAY BE AUTHORIZED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION. THE LONGEST VERTICAL CURVE COMPUTED BY THESE METHODS WITH EACH CRITERIA WILL GOVERN.

FREIGHT OPERATIONS: A=0.10 FEET/SEC/SEC

PASSENGER OPERATIONS: A=0.60 FEET/SEC/SEC

MIXED PASSENGER WITH FREIGHT TRAFFIC NOT EXCEEDING 4000 TON TRAINS OR 8 MILLION GROSS TONS ANNUAL FREIGHT TRAFFIC A=0.30 FEET/SEC/SEC FREIGHT SPEED A=0.60 FEET/SEC/SEC PASSENGER SPEED

WHEN DESIGNING VERTICAL CURVES ON MIXED USE FREIGHT AND PASSENGER OPERATIONS, THE DESIGNER SHALL CALCULATE MINIMUM LVC'S USING THE APPLICABLE VALUES OF "A" AND "V" AND SELECT THE LONGEST VALUE YIELDED. THE MINIMUM DISTANCE BETWEEN VERTICAL CURVES SHALL BE 3V OR 100 FT, WHICHEVER IS GREATER.

- (V=DESIGN SPEED IN MPH.)
- TURNOUTS SHALL NOT BE PLACED WITHIN THE LIMITS OF ANY VERTICAL CURVE.
  THE DESIRABLE LENGTH OF VERTICAL CURVES IN YARD TRACKS SHALL BE NOT LESS THAN 100 FT. THE MINIMUM LENGTH OF VERTICAL CURVES IN YARD TRACKS SHALL BE 30 FT.
- THE GOAL OF DESIGN OF THE VERTICAL ALIGNMENT IS TO REDUCE THE NUMBER OF VERTICAL CURVES, CONSISTENT WITH ENGINEERING ECONOMY AND SITE CONSTRAINTS.
- VERTICAL CURVES SHALL BE DESIGNED USING THE FUTURE MAXIMUM DESIGN SPEED FOR PASSENGER AND FREIGHT TRAINS EXPECTED ON A GIVEN SUBDIVISION. FUTURE MAXIMUM SPEEDS FOR PASSENGER TRAINS MAY EXCEED SPEEDS CURRENTLY IN EFFECT. DESIGNERS SHALL CONSULT WITH THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION FOR THE FUTURE MAXIMUM PASSENGER SPEED AT EACH LOCATION.
  SPEED RESTRICTIONS DUE TO SIGNAL/STOPPING DISTANCE WILL NOT BE CONSIDERED.
- PLANS FOR NEW CONSTRUCTION, REHABILITATION, AND TEMPORARY TRACK SHALL CLEARLY SHOW THE PERCENT GRADE CHANGE, DESIGN SPEED, BEGINNING AND ENDS, AND LENGTH OF EACH VERTICAL CURVE, AND MUST SHOW CONSTRAINTS TO VERTICAL PROFILE SUCH AS EXISTING OR FUTURE BRIDGES, TURNOUTS OR STATION PLATFORMS.

  VERTICAL CURVES WITHIN 100 FEET OF A STATION PLATFORM SHALL BE AVOIDED.

#### EXAMPLE CALCULATION FOR FREIGHT OPERATIONS

CREST CURVE WITH +0.50% APPROACHING GRADE MEETING A -0.50% DEPARTING GRADE. MAXIMUM DESIGN SPEED IS 50 MPH.

A=0.10 FEET/SEC/SEC VERTICAL ACCELERATION (FREIGHT) D-ABSOLUTE VALUE OF ((+0.005)-(-0.005))-0.01 K-2.15 CONVERSION FACTOR TO GIVE LVC IN FEET V=50 MPH DESIGN SPEED

LVC= D x V<sup>2</sup>x K = MINIMUM LENGTH OF VERTICAL CURVE IN FEET

LVC=  $(0.01) \times (50MPH)^2 \times 2.15 = 537.50$  FEET SAY 540 FEET 0.10 FEET/SEC/SEC

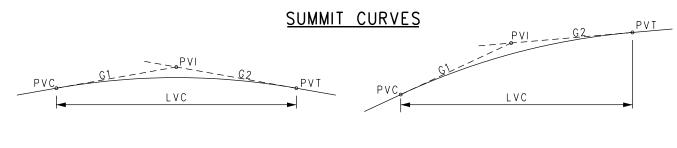
#### EXAMPLE CALCULATION FOR PASSENGER OPERATIONS

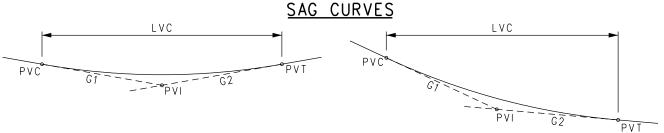
CREST CURVE WITH +0.50% APPROACHING GRADE MEETING A -0.50% DEPARTING GRADE. MAXIMUM DESIGN SPEED IS 75 MPH.

A=0.60 FEET/SEC/SEC VERTICAL ACCELERATION (PASSENGER) D=ABSOLUTE VALUE OF ((+0.005)-(-0.005))=0.01 K=2.15 CONVERSION FACTOR TO GIVE LVC IN FEET V=75 MPH DESIGN SPEED

LVC= D x  $V^2$ x K = MINIMUM LENGTH OF VERTICAL CURVE IN FEET

LVC=  $(0.01) \times (75 \text{MPH})^2 \times 2.15$  = 201.56 FEET SAY 205 FEET 0.60 FEET/SEC/SEC





#### **ABBREVIATIONS**

APPROACHING GRADE DEPARTING GRADE LENGTH OF VERTICAL CURVE POINT OF VERTICAL CURVATURE POINT OF VERTICAL INTERSECTION PVIPOINT OF VERTICAL TANGENCY PVT VERTICAL CURVE

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					/ Varen D. Poll
					ASSISIANT DIRECTOR: STANDARDS & DESIGN
Х	xx-xx-xx	REVISION	XX	XX	William Daran
REV.	DATE	DESCRIPTION	DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION

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VERTICAL CURVE GEOMETRY

ENGINEERING STANDARDS

NTS 1 OF FS2201

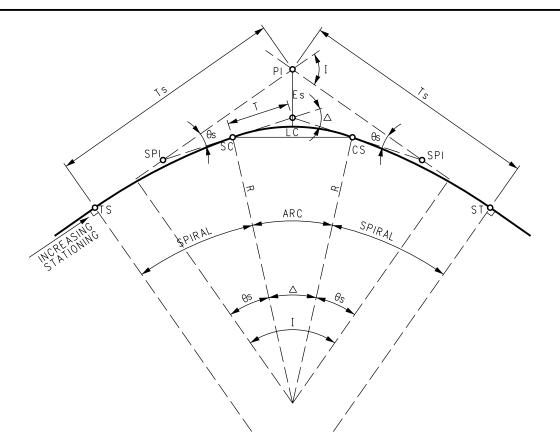


FIGURE A CIRCULAR CURVE WITH SPIRAL TRANSITIONS

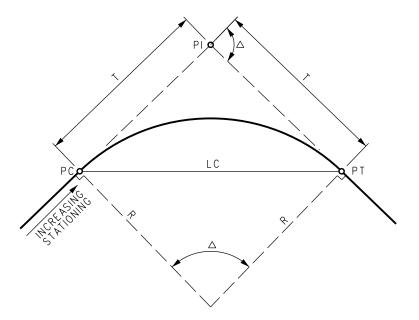


FIGURE B SIMPLE CIRCULAR CURVE

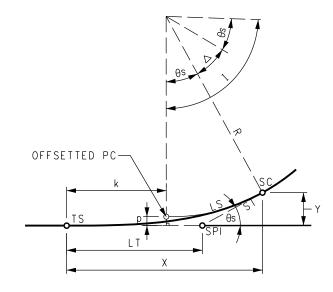


FIGURE C SPIRAL TRANSITION CURVE

#### ABBREVIATIONS AND SYMBOLS

C C C S	COMPOUND CURVE CURVE TO SPIRAL	R
∆ Dc E	CENTRAL ANGLE OF CIRCULAR CURVE DEGREE OF CURVATURE (CHORD DEFINITION) EQUILIBRIUM ELEVATION (Ea + Eu)	Δ
Ea Es Eu	ACTUAL ELEVATION EXTERNAL DISTANCE FROM PI TO CIRCULAR CURVE UNBALANCED ELEVATION (CANT DEFICIENCY)	L
I K k	TOTAL INTERSECTION ANGLE (DEFLECTION ANGLE AT THE PI) INCREASE IN DEGREES OF CURVATURE PER 100 FT STATIONS ALONG SPIRAL TANGENT DISTANCE FROM THE TS TO THE OFFSETTED PC	T
Į L	LENGTH FROM THE TS OR ST TO ANY POINT ON THE SPIRAL HAVING COORDINATES X AND Y CHORDED LENGTH OF CIRCULAR CURVE	LC
LC LS LT	LONG CHORD LENGTH OF SPIRAL LONG TANGENT (DISTANCE FROM THE TS TO THE SPI)	LS
p PC	ORDINATE OF THE OFFSETTED PC POINT OF CURVATURE	S
PCC PI PRC	POINT OF COMPOUND CURVATURE POINT OF INTERSECTION POINT OF REVERSE CURVATURE	θs
PT R s S	POINT OF TANGENCY RADIUS LENGTH   IN 100 FT STATIONS	K
S SC SPI	LENGTH OF SPIRAL (LS) IN 100 FT STATIONS SPIRAL TO CURVE POINT OF INTERSECTION BETWEEN TS AND SC	LT
SS ST ST	SPIRAL TO SPIRAL SPIRAL TO TANGENT SHORT TANGENT (DISTANCE FROM SPI TO SC)	ST
⊕s TS Ts	TANGENT LENGTH OF CIRCULAR CURVE TANGENT TO SPIRAL TOTAL TANGENT DISTANCE OF A SPIRALED CURVE	
V	SPIRAL ANGLE	

#### KEY FORMULAE

$R = \frac{50'}{SIN(\frac{Dc}{2})}$	$T_{S} = (R+p)TAN(\frac{I}{2})+k$
$\Delta = I - 2\theta s$	$E_S = (R+p)EX SEC(\frac{1}{2})+p$
$L = \frac{\triangle}{Dc} X 100$	χ = 1 - 0.003048θs s
$T = R TAN(\frac{\Delta}{2})$	$\gamma = 0.582 \theta s s - 0.00001264 \theta s s$
$LC = 2R SIN(\frac{\triangle}{2})$	$k = \frac{LS}{2} - 0.000508\Delta^2S$
$LS = \frac{200\theta s}{Dc}$	p = 0.1454 θs S
$S = \frac{LS}{100}$	
$\theta s = \frac{LSDc}{200}$	
$K = \frac{100 Dc}{LS}$	

#### NOTES:

- 1. CIRCULAR CURVES ARE DEFINED BY THE CHORD DEFINITION (CENTRAL ANGLE SUBTENDED BY A CHORD OF 100 FEET) OF CURVATURE AND SPECIFIED BY DEGREE.

  2. SPIRALS ARE DEFINED BY THE CLOTHOID DEFINITION. AUTHORIZATION FROM SCRRA SHALL BE OBTAINED IF ANY DIFFERENT METHOD OR PARAMETERS ARE UTILIZED FOR SPIRAL TRANSITION CURVES. THE REQUEST SHALL BE FULLY DOCUMENTED WITH DESIGN DATA, CALCULATIONS AND OTHER PERTINENT INFORMATION.

  3. THE TRACK GEOMETRY DATA TABLE, SHOWN IN ES2202-2, SHALL BE COMPLETED AND SUBMITTED TO SCRRA FOR REVIEW, COMMENT AND APPROVAL FOR ALL CURVES.

  4. ALL ANGLES ARE IN DEGREES, DISTANCES AND LENGTHS ARE IN FEET, EXCEPT SUPERELEVATIONS ARE IN INCHES AND SPEEDS ARE IN MILES PER HOUR (MPH).

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TANGENT DISTANCE FROM TS TO SC TANGENT OFFSET TO THE SC

ENGINEERING	CTVNDVDDC
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HORIZONTAL CURVE GEOMETRY

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REV.	DATE	DESCRIPTION	DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	ALL RIGHTS RESERVED.

1 OF 2 ES2202-01

									Т	RACK	GEOM	IETRY	DATA	A TABL	.E											
			STAT	IONING	DATA					INP	JT DA	ΤΑ			С	URVE	DATA	4			S	PIRAL	DAT	А		
	CURVE OR TURNOUT NO	DESC	BEARING	DISTANCE	STATION	NORTHING	EASTING	Dc DEGREE:	E a S INCHES		V (PAS) MPH	V (FRT) MPH		I DEGREES	R FEET	 DEGREES	L FEET	T FEET	Θs FEET	X FEET	Y FEET	k FEET	p FEET	LT FEET	ST FEET	Ts FEET
		POB TS	Х	X	X	X	X																			
CIRCULAR CURVE	M1	SC PI			X	X	X	X	X	X	Y	X	X	Х	Y	X	X	X	X	X	X	X	X	Х	X	X
TRANSITIONS	191 1	CS ST			X	X	X			^	^	^	X	Λ		Α	^	, , , , , , , , , , , , , , , , , , ,	X	X	X	X	X	Х	X	X
TURNOUT	C 3 3	PITO PS	X	X	X	X	X																			
		TS SC	Χ	Х	X	X	X						X						X	X	X	X	X	Х	X	X
COMPOUND CIRCUI AR		PI			X	X	X	X	X	X	X	X		X	Χ	X	X	X								
CURVE WITH SPIRAL TRANSITIONS	M2	CS SC			X	X	X						X						X	Х	Х	Х	X	Χ	X	Х
WITH SPIRAL TRANSITIONS  TURNOUT  COMPOUND CIRCULAR CURVE WITH		PI CS			X	X	X	X	X	X	X	X	X	Х	Х	X	X	X	X	X	X	X	X	X	X	X
-		ST PC	Х	X	X	X	X														^		^			\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
SIMPLE CIRCULAR CURVE	М3	PI PT	V	V	X	X	X	X	X	X	X	X		Х	Х	X	Х	X								
		PC PI	X	X	X	X	X	X	X	X	Х	X		Х	Х	Х	Х	X								
COMPOUND CIRCULAR CURVE	M 4	PCC PI			X	X	X	X	X	X	X	X		Х	Х	Х	X	X								
		PT POE	Х	X	X	X	X																			

- TRACK GEOMETRY DATA TABLES SHALL BE COMPLETED AND INCLUDED WITH DESIGN DRAWINGS SUBMITTED TO SCRRA FOR REVIEW, COMMENT, AND APPROVAL. EACH PROPOSED OR REALIGNED TRACK SHALL REQUIRE A SEPARATE TABLE.
   CELLS MARKED WITH AN "X" WILL NORMALLY CONTAIN DATA.
   IN PRACTICE, COMPOUND CURVES WITH MORE THAN TWO CIRCULAR ARCS ARE RARE. IN THEORY, A COMPOUND

- CURVE CAN HAVE AN INFINITE NUMBER OF CIRCULAR ARCS.
- 4. FOR FREIGHT-ONLY OPERATIONS, COLUMN "V (PAS)" WILL REMAIN BLANK. FOR PASSENGER-ONLY OPERATIONS,
- COLUMN "V (FRT)" WILL REMAIN BLANK.
- 5. IN THE EVENT A DESIGNER MUST PROPOSED A CURVE THAT DOES NOT MEET DESIGN REQUIREMENTS PER SCRRA ES2203 AND ES2204, THE DESIGNER SHALL CLEARLY INDICATE IT ON THE GEOMETRY TABLE. THE DESIGNER SHALL, FOR EACH PROPOSED SUBSTANDARD CURVE, SUBMIT TO SCRRA A WRITTEN REQUEST AND JUSTIFICATION FOR A DESIGN WAIVER.

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ENGINEERING	STANDARDS

TRACK GEOMETRY DATA TABLE

NTS 2 OF 2 ES2202-02

#### GENERAL

1. THERE ARE SIX TABLES OF DESIGN AND MAINTENANCE STANDARDS FOR SCRRA TRACK ALIGNMENT:

3.5 - INCH UNBALANCED ELEVATION - STANDARD SPIRAL LENGTH TABLE FOR PASSENGER OPERATIONS TABLE P3.5:

TABLE F2.0: 2.0 - INCH UNBALANCED ELEVATION - STANDARD SPIRAL LENGTH TABLE FOR FREIGHT OPERATIONS

3.5 - INCH UNBALANCED ELEVATION - MINIMUM TABLE P3.5M: SPIRAL LENGTH TABLE FOR PASSENGER OPERATIONS

2.0 - INCH UNBALANCED ELEVATION - MINIMUM TABLE F2.0M: SPIRAL LENGTH TABLE FOR FREIGHT OPERATIONS

TABLE PML: 4.0 - INCH UNBALANCED ELEVATION - MAINTENANCE LIMIT

FOR PASSENGER OPERATIONS

3.0 - INCH UNBALANCED ELEVATION - MAINTENANCE LIMIT TABLE FML: FOR FREIGHT OPERATIONS

2. FOR THE OPERATION OF PASSENGER EQUIPMENT NORMALLY USED IN SCRRA AND AMTRAK TRAINS;

THE DESIGN AND MAINTENANCE OF CURVE GEOMETRY IS CONTROLLED BY FRA TRACK SAFETY STANDARDS (49CFR213.57), WHICH ESTABLISHES THE MAXIMUM SPEED FOR ANY COMBINATION OF CURVATURE AND SUPERELEVATION FOR PASSENGER TRAINS AS RESULTING IN 4 INCHES OF UNDERBALANCE. TO ASSURE THAT NORMAL MAINTENANCE VARIATIONS DO NOT INADVERTENTLY RESULT IN CURVE GEOMETRY THAT CAUSES MORE THAN 4 INCHES OF UNBALANCED ELEVATION, THE DESIGN UNDERBALANCE IS SET AT 3.5 INCHES FOR TABLES P3.5 AND P3.5M. THE FRA TABLES AND FORMULAS DEFINE 4 INCHES OF UNBALANCED ELEVATION AS THE THRESHOLD OF FAILURE; THESE SCRRA TABLES DESIGNATE DESIGN PRACTICE THAT FITS WITHIN THE FRA LIMITS. DESIGNERS AND MAINTENANCE PERSONNEL WILL CONSTRUCT AND MAINTAIN TRACK TO THOSE VALUES EXCEPT AS AUTHORIZED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION, OR AS EXCEPTED BELOW.

SCRRA PASSENGER EQUIPMENT IS SUBJECT TO LESSENED STABILITY IN WINDS EXCEEDING 45 MPH AND ARE LIMITED TO AN UNDERBALANCE OF 3 INCHES UNDER THOSE CONDITIONS. INSTRUCTION TO TRAIN CREWS ARE TO REDUCE SPEED BY 5 MPH UNDER STRONG WIND CONDITIONS.

FREIGHT TRAIN SPEEDS ARE GOVERNED BY 49CFR213.57 TO NOT RESULT IN MORE THAN 3 INCHES OF UNDERBALANCE. FREIGHT TRAIN SPEEDS FOR NEW CURVES WILL BE DESIGNED PER TABLES F2.0 AND F2.0M, WHICH HAVE 2 INCHES UNDERBALANCE; EXISTING CURVES MAY BE MAINTAINED WITH UP TO 3 INCHES OF UNDERBALANCE AND REMEDIAL ACTION MUST BE TAKEN FOR ANY CURVE FOUND TO EXCEED 3 INCHES OF UNDERBALANCE FOR FREIGHT TRAIN SPEED.

SPIRAL EASEMENT CURVES WILL BE USED TO CONNECT CURVES TO TANGENT TRACK WHENEVER THERE IS SUPERELEVATION IN THE CURVE. THE SUPERELEVATION IS TO BE UNIFORMLY INCREASED FROM THE TANGENT TO THE CURVE THROUGHOUT THE LENGTH OF THE SPIRAL. THE SPIRAL IS ALSO A HORIZONTAL ALIGNMENT ELEMENT OF GRADUALLY DECREASING RADIUS, WHICH MATCHES THE RADIUS OF THE CIRCULAR CURVE ELEMENT AT THE POINT IT MEETS THE CURVE.

THE LENGTH OF THE SPIRALS IN THE TABLES HAS BEEN CALCULATED BASED UPON THE SPEED OF THE TRAIN AND ON THE MAXIMUM TWIST THAT ROLLING STOCK CAN SAFELY NEGOTIATE. LONG CARS THAT TRAVERSE SPIRALS THAT HAVE MORE THAN 1 INCH OF ELEVATION CHANGE IN 62 FEET BEGIN TO UNLOAD SOME OF THE VERTICAL LOAD ON WHEELS IF THEIR SIDE BEARING CLEARANCE IS AT MINIMUMS. THEREFORE STANDARD LENGTH SPIRALS DO NOT EXCEED THIS RATE OF CHANGE. A MAXIMUM CHANGE OF 1 INCH PER 50 FEET IS PERMITTED UNDER THE "MINIMUM" TABLES, BECAUSE SPIRALS WITH THESE PARAMETERS ARE FOUND ON SOME LINES AND CANNOT BE CHANGED DUE TO GEOGRAPHIC LIMITATIONS. THE MINIMUM SPIRAL LENGTHS FOUND IN TABLES P3.5M AND F2.0M MAY ONLY BE USED ON THE VENTURA AND ANTELOPE VALLEY SUBDIVISION. AT SPEEDS IN EXCESS OF 50 MPH, THE LENGTH OF SPIRALS IS INCREASED TO MINIMIZE TRANSIENT DYNAMIC LOADS AND PASSENGER DISCOMFORT.

SPIRALS MAY BE LONGER THAN THE STANDARD LENGTHS SHOWN. LONGER SPIRALS THAT EXIST FROM ORIGINAL CONSTRUCTION WILL NOT BE SHORTENED UNLESS NECESSARY TO OBTAIN REVERSING TANGENT LENGTH. SPIRALS FOR CURVES WHICH MAY BE DESIGNED FOR HIGHER SPEED IN THE FUTURE (E.G. NEAR PRESENT SPEED RESTRICTIONS SUCH AS TUNNELS) SHOULD BE DESIGNED WITH SPIRAL LENGTHS FOR FUTURE HIGHER SPEED AND SUPERELEVATION; AND PRESENTLY NEEDED SUPERELEVATION RUNOFF OVER THE LENGTH OF THE SPIRAL.

NEW CONSTRUCTION WILL BE DESIGNED WITH STANDARD LENGTH SPIRALS PER THE EXAMPLE SHOWN ON THIS SHEET FOR THE MAXIMUM FUTURE DESIGN SPEED FOR THE LOCATION.

#### CURVE DESIGN PROCEDURE

- 1. REFER TO AREMA CHAPTER 5.3 FOR A COMPLETE DISCUSSION OF CURVE DESIGN.
- 2. IN ORDER TO SELECT THE SUPERELEVATION AND SPIRAL LENGTHS FOR CURVES, THE DESIGN SPEEDS FOR FREIGHT AND PASSENGER TRAINS MUST BE DEVELOPED. A SERIES OF TRIAL SOLUTIONS IS USUALLY NECESSARY. EVERY CURVE MUST MEET THE STANDARDS OF SPIRAL LENGTH AND SUPERELEVATION FOR THE SPEED CHOSEN THE GOAL IS TO OBTAIN THE MAXIMUM SPEED FOR PASSENGER TRAINS CONSISTENT WITH GOOD TRAIN HANDLING, SIGNAL SPACING AND PRACTICAL LIMITS OF EQUIPMENT PERFORMANCE AND TO HAVE THE RESULTING DESIGN PROVIDE AN ACCEPTABLE FREIGHT TRAIN OPERATION AND MAINTENANCE ENVIRONMENT.
- 3. HORIZONTAL CURVES SHALL BE DESIGNED USING THE FUTURE MAXIMUM DESIGN SPEED FOR PASSENGER AND FREIGHT TRAINS EXPECTED ON A GIVEN SUBDIVISION. FUTURE MAXIMUM SPEEDS FOR PASSENGER TRAINS MAY EXCEED SPEEDS CURRENTLY IN EFFECT. THIS MAY RESULT IN SPIRAL LENGTHS THAT ARE LONGER THAN REQUIRED TO PROVIDE FOR PROPOSED SUPERELEVATION RUNOFF FOR NEW CONSTRUCTION. DESIGNERS WILL CONSULT WITH THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION FOR THE FUTURE PASSENGER SPEED AT EACH LOCATION. THE SPIRAL LENGTH DESIGN SHALL BE SUFFICIENT TO ALLOW SUPERELEVATION RUNOFF FOR THE FUTURE MAXIMUM DESIGN SPEED EVEN IF THE ACTUAL DESIGN OPERATING SPEED IS LESS THAN THE FUTURE MAXIMUM DESIGN SPEED.
- 4. THE MAXIMUM SPEED FOR FREIGHT TRAINS IS 60 MILES PER HOUR.
- 5. ALL NEW WORK SHOULD USE TABLES P3.5 AND F2.0 TO SPECIFY STANDARD LENGTH SPIRALS. TABLES WITH SUFFIX "M" ARE TO BE USED ONLY ON THE VENTURA AND ANTELOPE VALLEY SUBDIVISIONS AND ONLY AT LOCATIONS CONSTRAINED BY EXISTING TRACK GEOMETRY. CURVES WHICH DO NOT MEET THE STANDARDS OF TABLES P3.5, F2.0, P3.5M AND F2.0M MUST BE CORRECTED THROUGH REDUCTION OF TRAIN SPEED AND ALTERATION TO THE TRACK CHARACTERISTICS.
- 6. FOR MAXIMUM DESIGN SPEEDS UP TO 35 MPH, CURVES IN OPPOSITE DIRECTIONS SHALL BE SEPARATED BY A REVERSING TANGENT WITH A MINIMUM LENGTH OF 100' FOR DESIGN SPEEDS GREATER THAN 35 MPH, CURVES IN OPPOSITE DIRECTIONS SHALL BE SEPARATED BY A REVERSING TANGENT WITH A MINUMUM LENGTH EQUAL TO 3 TIMES THE MAXIMUM DESIGN SPEED AS STATED IN MILES PER HOUR.FOR EXAMPLE, A DESIGN SPEED OF 50 MPH WILL REQUIRE A REVERSING TANGENT WITH A MINIMUM LENGTH OF 150' (3 TIMES 50). EXCEPTIONS WILL REQUIRE THE APPROVAL OF THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- 7. REVERSING TANGENTS MAY BE REDUCED TO HALF OF THE ABOVE WHERE THERE IS LESS THAN 1 INCH OF SUPERELEVATION IN BOTH CURVES.
- 8. ALL DESIGN SPEEDS MUST BE APPROVED BY BOTH THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION AND THE SCRRA MANAGER OF SIGNAL AND COMMUNICATIONS.
- 9. SPEEDS SHOULD BE ESTABLISHED IN CONSIDERATION OF PLACEMENT OF SPEED SIGNS PER SCRRA ES5213, SUCH THAT THERE IS NO OVERLAP BETWEEN SIGNS FOR REDUCTION AND INCREASE OF SPEED IN THE SAME DIRECTION.
- 10. SPEED AND SUPERELEVATION WILL BE CONSISTENT THROUGH CURVES UNLESS AUTHORIZED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION. ALL COMPOUND CURVES WILL BE SEPARATED WITH A SPIRAL OF AT LEAST 31 FEET. IN COMPOUND CURVES WHERE SUPERELEVATION DIFFERS IN EACH CURVE, A SPIRAL OF APPROPRIATE LENGTH WILL BE REQUIRED AT THE POINT OF COMPOUND CURVATURE. THE SPIRAL LENGTH WILL BE DESIGNED TO ACCOMMODATE THE DIFFERENCE OF THE COMPOUND CURVE'S SUPERELEVATIONS. A COMPOUND SPIRAL IS NOT REQUIRED WHERE THE SPIRAL OFFSET IS LESS THAN 0.25".
- 11. ACTUAL ELEVATION GREATER THAN 5 INCHES IS NOT PERMITTED WITHOUT PRIOR APPROVAL OF THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- 12. SUPERELEVATION THROUGH GRADE CROSSINGS WILL BE DESIGNED WITH CONSIDERATION OF THE STREET PROFILE, WHICH MAY CONSTRAIN THE SUPERELEVATION AND THEREFORE THE CURVE SPEED. THE STREET PROFILE SHOULD BE CONSIDERED TO BE CHANGED IF PRACTICAL TO ACCOMODATE SUPERELEVATION FOR THE PROPOSED MAXIMUM SPEED.
- 13. SPEEDS FOR FREIGHT TRAINS SHOULD BE AS UNIFORM AS PRACTICABLE. FREIGHT TRAINS GENERALLY CANNOT UTILIZE HIGHER SPEEDS THAT ARE LESS THAN 2 MILES IN LENGTH. DUE TO BRAKING DISTANCES AND SIGNAL SPACING, FREIGHT TRAIN SPEEDS MAY BE SET WHICH ARE SUBSTANTIALLY LESS THAN PASSENGER TRAIN SPEEDS. OPERATION OF FREIGHT TRAINS AT

#### CURVE DESIGN PROCEDURE (CONT)

SPEEDS LESS THAN EQUILIBRIUM (NO UNDERBALANCE) RESULTS IN HEAVY WEAR ON THE LOW RAIL AND LOW VERTICAL LOADS TO THE HIGH WHEELS

- 14. DESIGNERS SHOULD AVOID SUPERELEVATIONS IN EXCESS OF 4 INCHES WHERE GRADES OR OTHER RESTRICTIONS CAUSE TRAINS TO RUN A SPEED LESS THAN 25 MILES PER HOUR.
- 15. FREIGHT TRAIN MAXIMUM AUTHORIZED SPEED SHALL BE BASED ON A STANDARD UNBALANCED ELEVATION BETWEEN 1 AND 2 INCHES. SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION MUST APPROVE ANY COMBINATION OF FREIGHT SPEED AND CURVE SUPERELEVATION OUTSIDE THESE LIMITS.
- 16. THE PRIORITIES FOR DESIGNERS ARE:
  - SET MAXIMUM DESIGN SPEED AND DEGREE OF CURVATURE FOR PASSENGER AND FREIGHT TRAINS ON A GIVEN SUBDIVISION AFTER CONSULTATION WITH SCRRA.
  - ASSURE ADEQUATE REVERSING TANGENTS AND SPIRAL LENGTHS - ASSURE ACTUAL ELEVATIONS AND STANDARD SPIRAL LENGTHS FOR HIGHEST PASSENGER AND FREIGHT TRAIN SPEEDS.
  - ASSURE UNIFORM FREIGHT TRAIN SPEED THAT CAN BE SUSTAINED FOR AT LEAST TWO (2) MILES.
  - ASSURE MAXIMUM FREIGHT TRAIN SPEED IS 60 MPH.
  - SET ACTUAL ELEVATION AND SPIRAL LENGTHS FOR FASTEST PRACTICABLE PASSENGER TRAIN OPERATION CONSISTENT WITH SCRRA AND FRA STANDARDS.
- 17. THESE DESIGN STANDARDS DO NOT REPLACE FRA TRACK SAFETY STANDARDS PART 49CFR213.57. IN ADDITION TO COMPLYING WITH THE OVERALL PARAMETERS OF SUPERELEVATION AND SPIRAL LENGTH CURVES MUST ALSO COMPLY WITH ALL PARTS OF 213.5 THRU 213.63. IN PRACTICE, DESIGNERS SET THE OVERALL PARAMETERS AND MAINTENANCE PERSONNEL PREVENT ANY IRREGULARITIES WHICH COULD BECOME EXCEPTIONS TO THE FRA STANDARDS.
- 18. THE HORIZONTAL ALIGNMENT OF SPIRAL CURVES MAY BE DESIGNED BY: -TEN CHORD SPIRAL
  - -AREMA CHAPTER 5.3.1.2
  - -CLOTHOID SPIRAL GENERATED UNDER CADD DESIGN, WHICH MEETS AREMA CRITERIA
- 19. WHEN THE CURVE CHARACTERISTICS ARE CHANGED AND APPROVED, THE NEW DATA SHOULD BE ENTERED ONTO THE TRACK CHARTS AND THE FIELD MARKING WILL BE UPDATED.
- 20. RUNOFF OF SUPERELEVATION ON TANGENT TRACK IS NOT PERMITTED.

#### SAMPLE CURVE DESIGN PROBLEM

A CURRENT RAIL LINE OPERATES PASSENGER SERVICE AT 70 MPH AND FREIGHT AT 50 MPH. A 2°-0'-0" HORIZONTAL CURVE HAS BEEN PROPOSED. WHAT SUPERELEVATION AND SPIRAL LENGTHS DO YOU USE? WILL PASSENGER AND FREIGHT BE ABLE TO MAINTAIN THEIR CURRENT SPEEDS?

1. LOOK UP THE Ea AND Ls FOR A 2°-0'-0" CURVE AT 70 MPH IN THE STANDARD SPIRAL LENGTH TABLE FOR PASSENGER OPERATIONS, TABLE P3.5.

Ea = 3.50", Ls = 300'

2. NOW CHECK CURVE FREIGHT SPEED AND ACTUAL ELEVATION FOR A 2°-0'-0" CURVE IN THE STANDARD SPIRAL LENGTH TABLE FOR FREIGHT OPERATIONS, TABLE F2.0.

FOR 65 MPH: Ea = 4.00" AND Ls = 320' FOR 60 MPH: Ea = 3.25" AND Ls = 240' FOR 50 MPH: Ea = 1.50" AND Ls = 100'

3. THE CURVE WILL NEED TO HAVE 3.50 INCHES OF SUPERELEVATION AND THE SPIRALS WILL NEED TO BE 300 FEET BECAUSE THE PASSENGER REQUIREMENTS GOVERN IN THIS SITUATION. FREIGHT CAN CONTINUE TO OPERATE AT 50 MPH OR MAY BE INCREASED TO 60 MPH IFTHIS SPEED CAN BE SUSTAINED FOR AT LEAST 2 MILES (CURVE DESIGN PROCEDURE NO. 13).

Х	xx-xx-xx	REVISION	XX	XX
REV.	DATE	DESCRIPTION	DES.	ENG.



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**ENGINEERING STANDARDS** 

NTS

1 OF

FS2203

CURVE SPEED, SUPERELEVATION AND SPIRAL LENGTH NOTES

#### TABLE P3.5 - 3.5 INCH UNBALANCED ELEVATION FOR PASSENGER OPERATIONS - STANDARD SPIRAL LENGTHS

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN) Vmax = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

Eo = ACTUAL ELEVATION OF OUTSIDE RAIL (IN) D = DEGREE OF CURVATURE (DECIMAL DEGREES)

FORMULAS

E = 0.0007DVmax<sup>2</sup>
Ea = E - Eu

SPIRAL LENGTH; THE LONGEST OF: Ls = 1.2VmaxEa
Ls = 62Ea
Lsmin = 40'

									MAXII	MUM AL	LOWABLE	PASSE	NGER O	PERATIN	IG SPEE	O - MILE	ES PER	HOUR								
	2	0	2	5	3	0	3	5	1	0	1	- 5	1	0	1	0	1	'0	8	0	g	10	10	00	11	10
	Εa	Ls	Εa	Ls	Εa	Ls	Ea	Ls	Ea	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Εa	Ls
0°15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.00"	60'	0.00"	60'	0.00"	70'
0°30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.00"	60'	0.00"	60'	0.75"	100'
0°45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	1.00"	110'	1.75"	210'	3.00"	400'
1° 00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	1.00"	100'	2.25"	250'	3.50"	420'	5.00"	660'
1° 15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	1.00"	90'	2.25"	220'	3.75"	410'	5.25"	630'		<del></del>
1° 30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.75"	150'	3.25"	320'	5.25"	570'				
1° 45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	1.00"	80'	2.75"	240'	4.50"	440'	-					
2° 00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	1.75"	130'	3.50"	300'	5.50"	530'						
2° 15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40' 40'	0.00"	40'	1.00"	70'	3.00"	170'	4.25" 5.25"	360' 450'								-
2°45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.50"	100'	3.50"	260'	6.00"	510'								
3°00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	1.00"	70'	1.75"	110'	4.25"	310'	0.00	310								
3° 15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00	40'	1.25"	80'	2.25"	140'	4.75"	350'										
3° 30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.50"	100'	2.75"	180'	5.50"	400'										
3° 45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.75"	50'	2.00"	130'	3.25"	210'	6.00"	440'										
4°00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	1.00"	70'	2.25"	140'	3.50"	220'	0.00	1										
4° 15'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	1.50"	100'	2.75"	180'	4.00"	250'												
4°30'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.75"	110'	3.00"	190'	4.50"	280'												
4°45'	0.00"	40'	0.00"	40'	0.00"	40'	0.75"	50'	2.00"	130'	3.25"	210'	5.00"	310'												
5°00'	0.00"	40'	0.00"	40'	0.00"	40'	1.00"	70'	2.25"	140'	3.75"	240'	5.25"	330'												
5° 15'	0.00"	40'	0.00"	40'	0.00"	40'	1.25"	80'	2.50"	160'	4.00"	250'	5.75"	360'												
5° 30'	0.00"	40'	0.00"	40'	0.00"	40'	1.25"	80'	2.75"	180'	4.50"	280'														
5° 45'	0.00"	40'	0.00"	40'	0.25"	40'	1.50"	100'	3.00"	190'	4.75"	300'														
6°00'	0.00"	40'	0.00"	40'	0.50"	40'	1.75"	110'	3.25"	210'	5.25"	330'														
6° 15'	0.00"	40'	0.00"	40'	0.50"	40'	2.00"	130'	3.50"	220'	5.50"	350'														
6° 30'	0.00"	40'	0.00"	40'	0.75"	50'	2.25"	140'	4.00"	250'	5.75"	360'														
6° 45'	0.00"	40'	0.00"	40'	1.00"	70'	2.50"	160'	4.25"	270'																
7°00'	0.00"	40'	0.00"	40'	1.00"	70'	2.75"	180'	4.50"	280'																
7° 15'	0.00"	40'	0.00"	40'	1.25"	80'	2.75"	180'	4.75"	300'																
7° 30'	0.00"	40'	0.00"	40'	1.25"	80'	3.00"	190'	5.00"	310'																
7°45'	0.00"	40'	0.00"	40'	1.50"	100'	3.25"	210'	5.25"	330'																
8,00,	0.00"	40'	0.00"	40'	1.75"	110'	3.50"	220'	5.50"	350'																
8° 15'	0.00"	40'	0.25"	40'	1.75"	110'	3.75"	240'	5.75"	360'																
8° 30'	0.00"	40'	0.25"	40'	2.00"	130'	4.00"	250'																		
8° 45'	0.00"	40'	0.50"	40'	2.25"	140'	4.25"	270'																		
9°00'	0.00"	40'	0.50"	40'	2.25"	140'	4.25"	270'																		-
9° 15'	0.00"	40'	0.75"	50'	2.50"	160'	4.50"	280'																		<u> </u>
9° 30'	0.00"	40'	0.75"	50'	2.50"	160'	4.75"	300'		<u>NO1</u>	[ES:															
9° 45'	0.00"	40'	1.00"	70' 70'	3.00"	180'	5.00"	310' 330'	1	1	NO SPIR	PALS OR	SUPERF	FIFVATI	ONS WII	I RF PI	FRMITTFI	т от п	HE RIGH	T OF HI	EAVY LII	JF WITH	OUT PRI	OR APP	ROVAL	-
10° 15'	0.00"	40'	1.00"	70'	3.00"	190' 190'	5.50"	350'			FROM T	HE SCRI	RA DIREC	CTOR O	F ENGINE	EERING	AND CON	NSTRUC'	TION.							
10° 30'	0.00"	40'	1.25"	80'	3.25"	210'	5.75"	360'		2.							MORE T	HAN A	LISTED F	FIGURE,	THE NEX	KT HIGH	ER ELEV	'ATION	AND	
10° 45'	0.00"	40'	1.25"	80'	3.50"	220'	5.75"	360'	+		KE2ULII	NG SELF	AL LEN	JI⊓ WIL	L BE US	DEU.										
11° 00'	0.00"	40'	1.50"	100'	3.50"	220'	6.00"	380'	+					1		ſ						ſ				
11° 15'	0.00"	40'	1.50"	100'	3.75"	240'	0.00	500	+		+															
11° 30'	0.00"	40'	1.75"	110'	3.75"	240'			+		+					-						-		-		<u> </u>
11° 45'	0.00"	40'	1.75"	110'	4.00"	250'			+																	
12°00'	0.00"	40'	1.75"	110'	4.25"	270'			+								+									
	0.00	10	1.75	110	1 1.20	1 2 , 0			1			1		1		L		1		<u> </u>	1	L	1	1	1	

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TABLE P3.5 – 3.5 INCH UNBALANCED ELEVATION STANDARD SPIRAL LENGTH TABLE FOR PASSENGER OPERATIONS

STANDARD		
	2204	
SCALE:		
	NTS	
REVISION	SHEET	
_	1 OF 6	
CADD FILE:		
	ES2204-01	

#### TABLE F2.0 - 2.0 INCH UNBALANCED ELEVATION FOR FREIGHT OPERATIONS - STANDARD SPIRAL LENGTHS

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN) Vmax = MAXIMUM ALLOWABI

Vmax = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

Ea = ACTUAL ELEVATION OF OUTSIDE RAIL (IN) D = DEGREE OF CURVATURE (DECIMAL DEGREES)

FORMULAS

E = 0.0007DVmax<sup>2</sup>
Ea = E - Eu

SPIRAL LENGTH; THE LONGEST OF: Ls = 1.2VmaxEa
Ls = 62Ea
Lsmin = 40'

									M A )	(IMUM A	JIOWAR	I F FRFI	GHT OPF	FRATING	SPEED	- MILES	S PER H	OUR								
	2	0	25	)	3	0	3	55	1	0	45		1	0		5		0	6	5	7	0	7	5	8	0
	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Εa	Ls	Ea	Ls	Ea	Ls	Εa	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Εa	Ls
0°15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.00"	50'
0°30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.25"	50'
0°45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	0.75"	70'	1.00"	90'	1.50"	150'
1° 00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	0.75"	60'	1.00"	80'	1.50"	130'	2.00"	180'	2.50"	240'
1° 15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	0.75"	50'	1.25"	90'	1.75"	140'	2.50"	210'	3.00"	270'	3.75"	360'
1° 30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	0.75"	50'	1.25"	90'	2.00"	150'	2.50"	200'	3.25"	280'	4.00"	360'	4.75"	460'
1° 45'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.25"	80'	1.75"	120'	2.50"	180'	3.25"	260'	4.25"	360'	5.00"	450'	6.00"	580'
2°00'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	1.00"	70'	1.50"	100'	2.25"	150'	3.25"	240'	4.00"	320'	5.00"	420'	6.00"	540'		
2°15'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'	0.75"	50'	1.25"	80'	2.00"	130'	3.00"	200'	3.75"	270'	4.75"	380'	5.75"	490'				
2°30'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'	1.00"	70'	1.75"	110'	2.50"	160'	3.50"	240'	4.50"	330'	5.50"	430'						
2°45'	0.00"	40'	0.00"	40'	0.00"	40'	0.50"	40'	1.25"	80'	2.00"	130'	3.00"	190'	4.00"	270'	5.00"	360'								
3°00'	0.00"	40'	0.00"	40'	0.00"	40'	0.75"	50'	1.50"	100'	2.50"	160'	3.25"	210'	4.50"	300'	5.75"	420'								
3° 15'	0.00"	40'	0.00"	40'	0.25"	40'	1.00"	70'	1.75"	110'	2.75"	180'	3.75"	240'	5.00"	330'										
3° 30'	0.00"	40'	0.00"	40'	0.25"	40'	1.25"	80'	2.00"	130'	3.00"	190'	4.25"	270'	5.50"	370'										
3° 45'	0.00"	40'	0.00"	40'	0.50"	40'	1.25"	80'	2.25"	140'	3.50"	220'	4.75"	300'	6.00"	400'										
4°00'	0.00"	40'	0.00"	40'	0.75"	50'	1.50"	100'	2.50"	160'	3.75"	240'	5.00"	310'												
4°15'	0.00"	40'	0.00"	40'	0.75"	50'	1.75"	110'	3.00"	190'	4.25"	270'	5.50"	350'												
일 4°30'	0.00"	40'	0.00"	40'	1.00"	70'	2.00"	130'	3.25"	210'	4.50"	280'	6.00"	380'												
4°45'	0.00"	40'	0.25"	40'	1.00"	70'	2.25"	140'	3.50"	220'	4.75"	300'														
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	0.00"	40'	0.25"	40'	1.25"	80'	2.50"	160'	3.75"	240'	5.25"	330'														
_ 5° 15'	0.00"	40'	0.50"	40'	1.50"	100'	2.75"	180'	4.00"	250'	5.50"	350'														
€ 5°30'	0.00"	40'	0.50"	40'	1.50"	100'	2.75"	180'	4.25"	270'	6.00"	380'														
S 45'	0.00"	40'	0.75"	50'	1.75"	110'	3.00"	190'	4.50"	280'																
문 6°00'	0.00"	40'	0.75"	50'	2.00"	130'	3.25"	210'	4.75"	300'																
일 6° 15'	0.00"	40'	0.75"	50'	2.00"	130'	3.50"	220'	5.00"	310'	<u> </u>															
○ 6°30'	0.00"	40'	1.00"	70'	2.25"	140'	3.75"	240'	5.50"	350'																
6° 45'	0.00"	40'	1.00"	70'	2.50"	160'	4.00"	250'	5.75"	360'																
7° 00'	0.00"	40'	1.25"	80'	2.50"	160'	4.25"	270'	6.00"	380'																
₹ 7° 15'	0.25"	40'	1.25"	80'	2.75"	180'	4.25"	270'																		
≥ 7° 30'	0.25"	40'	1.50"	100'	2.75"	180'	4.50"	280'																		
S  7° 45'	0.25"	40'		100'	3.00"	190'	4.75"	300'																		
8,00,	0.25"	40'	1.50"	100'	3.25"	210'	5.00"	310'																		
8° 15'	0.50"	40'	1.75"	110'	3.25"	210'	5.25"	330'					1													
8° 30'	0.50"	40'	1.75"	110'	3.50"	220'	5.50"	350'					1										-			
8° 45'	0.50"	40'	2.00"	130'	3.75"	240'	5.75"	360'					1													
9°00'	0.75"	50'	2.00"	130'	3.75"	240'	5.75"	360'					1										1			
9° 15'	0.75"	50'		140'	4.00"	250'	6.00"	380'					1													
9° 30'	0.75"	50'		140'	4.00"	250'				NOT	ES:															
9° 45'	0.75"	50'		160'	4.25"	270'							) CIIDEDE	E  F\/	UNIS WII	l BE D	ERMITTE	י דר דו	HE BIUH.	L UE he		IE WITL	INIIT DDI		ROVAL	
10° 00'	1.00"	70'		160'	4.50"	280'											AND CON			i or ne	.AVI LII	N∟ WIII⊓	IVUI FRI	ON AFF	NOVAL	
10° 15'	1.00"	70'		160'	4.50"	280'				2.	WHERE	CURVAT	URE IS N	MORE TH	HAN 5 M	INUTES	MORE T			FIGURE,	THE NEX	(T HIGH	ER ELEV	ATION	AND	
10° 30'	1.00"	70'		180'	4.75"	300'					KESULTI	NG SPIF	RAL LENG	H WIL ان	r RE A2	ELD.										
10° 45'		80'		180'	5.00"	310'	1				1	I	1	I	1	I			1		1	I	1	I		
11° 00'	1.25"	80'		190'	5.00"	310'	4				1		1						-				-			
11° 15'	1.25"	80'		190'	5.25"	330'	1				-		1						-				-			
11° 30'	1.25"	80'		210'	5.25"	330'	1						1				-		1				1			
11° 45'	1.50"	100'		210'	5.50"	350'	1	-					1	-												
12°00'	1.50"	100'	3.25"	210'	5.75"	360'																				

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Х	xx-xx-xx	REVISION	XX	XX	]
REV.	DATE	DESCRIPTION	DES.	ENG.	1-

AWN BY: A. CARLOS DATE: 03/31/2011

ASSISTANT DIRECTOR: STANDARDS & DESIGN

DIRECTOR OF ENGINEERING AND CONSTRUCTION

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SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

TABLE F2.0 – 2.0 INCH UNBALANCED ELEVATION STANDARD SPIRAL LENGTH TABLE FOR FREIGHT OPERATIONS

STANDARD	
	2204
SCALE:	
	NTS
REVISION	SHEET
_	2 OF 6
CADD FILE:	
	ES2204-02

#### TABLE P3.5M - 3.5 INCH UNBALANCED ELEVATION FOR PASSENGER OPERATIONS - MINIMUM SPIRAL LENGTHS

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN) Vmox = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

Ea = ACTUAL ELEVATION OF OUTSIDE RAIL (IN) D = DEGREE OF CURVATURE (DECIMAL DEGREES)

FORMULAS

E = 0.0007DVmax<sup>2</sup>
Ea = E - Eu

FORMULAS

SPIRAL LENGTH; THE LONGEST OF: Ls = 1.0VmaxEa
Ls = 50Ea
Lsmin = 30'

										MAXIN	JUM AL	LOWARIE	PASSE	NGER O	PFRATIN	G SPFF	) - MII	ES PER	HOUR								
		2	0	2	5	3	0	3	5	4			· 5		0		0		0	8	0	9	0	10	00	11	0
		Εa	Ls	Εσ	Ls	Ea	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Εa	Ls	Εa	Ls	Εa	Ls	Ea	Ls
	)° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.00"	60'
	)° 30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	0.00"	40'	0.00"	50'	0.00"	50'	0.75"	90'
	)° 45'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	0.00"	40'	1.00"	90'	1.75"	180'	3.00"	330'
I -	1° 00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	1.00"	80'	2.25"	210'	3.50"	350'	5.00"	550'
<b>⊢</b>	1° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	1.00"	70'	2.25"	180'	3.75"	340'	5.25"	530'		
1 H	1° 30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	1.75"	130'	3.25"	260'	5.25"	480'				
1 <del>-</del>	1° 45'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	1.00"	60'	2.75"	200'	4.50"	360'						
ı ⊢	2°00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	1.75"	110'	3.50"	250'	5.50"	440'						
l —	2° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	2.25"	140'	4.25"	300'					-			
I —	2° 30' 2° 45'	0.00"	30' 30'	0.00"	30' 30'	0.00"	30' 30'	0.00"	30' 30'	0.00"	30' 30'	0.25"	30'	1.00"	50' 80'	3.00"	180' 210'	5.25"	370' 420'								
I -	3°00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	1.00"	50'	1.75"	90'	4.25"	260'	0.00	420								
l <b>–</b>	3° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00	30'	1.25"	70'	2.25"	120'	4.75"	290'										
l	3° 30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	1.50"	80'	2.75"	140'	5.50"	330'	4				+		+			
l ⊢	3° 45'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.75"	40'	2.00"	100'	3.25"	170'	6.00"	360'										
1 <del>-</del>	1°00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	1.00"	50'	2.25"	120'	3.50"	180'	1	230					†					
l —	4°15'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	1.50"	80'	2.75"	140'	4.00"	200'												
ES	1°30'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	1.75"	90'	3.00"	150'	4.50"	230'												
	۱° 45'	0.00"	30'	0.00"	30'	0.00"	30'	0.75"	40'	2.00"	100'	3.25"	170'	5.00"	250'												
	5°00'	0.00"	30'	0.00"	30'	0.00"	30'	1.00"	50'	2.25"	120'	3.75"	190'	5.25"	270'												
	5° 15'	0.00"	30'	0.00"	30'	0.00"	30'	1.25"	70'	2.50"	130'	4.00"	200'	5.75"	290'												
\\ \!	5° 30'	0.00"	30'	0.00"	30'	0.00"	30'	1.25"	70'	2.75"	140'	4.50"	230'														
SI :	5° 45'	0.00"	30'	0.00"	30'	0.25"	30'	1.50"	80'	3.00"	150'	4.75"	240'														
	5°00'	0.00"	30'	0.00"	30'	0.50"	30'	1.75"	90'	3.25"	170'	5.25"	270'														
	6°15'	0.00"	30'	0.00"	30'	0.50"	30'	2.00"	100'	3.50"	180'	5.50"	280'														
I . ⊢	5° 30'	0.00"	30'	0.00"	30'	0.75"	40'	2.25"	120'	4.00"	200'	5.75"	290'														
11.11	6° 45'	0.00"	30'	0.00"	30'	1.00"	50'	2.50"	130'	4.25"	220'													-			
$1\supset \vdash$	7°00'	0.00"	30'	0.00"	30'	1.00"	50'	2.75"	140'	4.50"	230'	1															
I∢⊢	7° 15'	0.00"	30'	0.00"	30'	1.25"	70'	2.75"	140'	4.75"	240'	1												1			
102	7° 30' 7° 45'	0.00"	30' 30'	0.00"	30' 30'	1.25"	70' 80'	3.00"	150' 170'	5.00" 5.25"	250' 270'	-															
$10$ $\vdash$	3°00'	0.00"	30'	0.00"	30'	1.75"	90'	3.50"	180'	5.50"	280'																
1 <del>-</del>	3° 15'	0.00"	30'	0.25"	30'	1.75"	90'	3.75"	190'	5.75"	290'	+			+	+			+					+			
I —	3° 30'	0.00"	30'	0.25"	30'	2.00"	100'	4.00"	200'	3.73	230											1		1			
I -	3° 45'	0.00"	30'	0.50"	30'	2.25"	120'	4.25"	220'			<del>                                     </del>												†			
I —	9°00'	0.00"	30'	0.50"	30'	2.25"	120'	4.25"	220'			<u> </u>										1		<u> </u>			
	9°15'	0.00"	30'	0.75"	40'	2.50"	130'	4.50"	230'																		
9	9°30'	0.00"	30'	0.75"	40'	2.50"	130'	4.75"	240'		NO	ES:						•				•					
9	9°45'	0.00"	30'	1.00"	50'	2.75"	140'	5.00"	250'		INO	<u> </u>															
[1	0°00'	0.00"	30'	1.00"	50'	3.00"	150'	5.25"	270'												r of H	EAVY LIN	IE WITH	OUT PRI	OR APP	ROVAL	
1	0°15'	0.00"	30'	1.00"	50'	3.00"	150'	5.50"	280'									AND CON			ALLEY S	SUBDIVIS	ONS AT	LOCAT	IONS WE	IFRF	
_	0°30'	0.00"	30'	1.25"	70'	3.25"	170'	5.75"	290'			STANDAF	RD SPIR	AL LENG	THS CA	N NOT E	BE OBT.	AINED DU	JE TO E	XISTING	FIELD	CONDITIO	NS.				
l —	0°45'	0.00"	30'	1.25"	70'	3.50"	180'	5.75"	290'		3.					HAN 5 M L BE US		MORE T	HAN A	LISTED F	FIGURE,	THE NE	CT HIGH	ER ELEV	'ATION A	AND	
_	1° 00'	0.00"	30'	1.50"	80'	3.50"	180'	6.00"	300'			NESULII	ויט ארור	AL LEN	ı III WIL	1 01 03	, L U .								ı		
ı ⊢	1° 15'	0.00"	30'	1.50"	80'	3.75"	190'																				
l —	1° 30'	0.00"	30'	1.75"	90'	3.75"	190'															1		1			
ı ⊨	1° 45'	0.00"	30'	1.75"	90'	4.00"	200'															1		1			
	2°00'	0.00"	30'	1.75"	90'	4.25"	220'																				

Х	xx-xx-xx	REVISION	XX	XX
REV.	DATE	DESCRIPTION	DES.	ENG.

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## METROLINIC .

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY
ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS

TABLE P3.5M – 3.5 INCH UNBALANCED ELEVATION MINIMUM SPIRAL LENGTH TABLE FOR PASSENGER OPERATIONS

STANDARD

2204

SCALE:
NTS

REVISION SHEET

- 3 OF 6

CADD FILE:
ES2204-03

#### TABLE F2.0M - 2.0 INCH UNBALANCED ELEVATION FOR FREIGHT OPERATIONS - MINIMUM SPIRAL LENGTHS

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN) Vmax = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

D = DEGREE OF CURVATURE (DECIMAL DEGREES) Ea = ACTUAL ELEVATION OF OUTSIDE RAIL (IN)

FORMULAS SPIRAL LENGTH; THE LONGEST OF: Ls = 1.0VmaxEa  $E = 0.0007DVmax^2$ Ls = 50Ea Ea = E - Eu Lsmin = 30'

									MAX	(IMIIM A	ALLOWAR	IF FRFI	GHT OPE	RATING	SPEED	- MILES	S PER H	OUR								
	2	0	2	5	3	50			.5		0	1	5	1	50	1 6	55	7	'0	7	5	8	30			
	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls	Ea	Ls
0° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	0.00"	40'	0.00"	40'	0.00"	40'
0°30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	40'	0.00"	40'	0.00"	40'	0.25"	40'
0°45'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	40'	0.75"	60'	1.00"	80'	1.50"	120'
1° 00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	0.75"	50'	1.00"	70'	1.50"	110'	2.00"	150'	2.50"	200'
1° 15 '	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	0.75"	50'	1.25"	80'	1.75"	120'	2.50"	180'	3.00"	230'	3.75"	300'
1° 30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	0.75"	40'	1.25"	70'	2.00"	120'	2.50"	170'	3.25"	230'	4.00"	300'	4.75"	380'
1° 45'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	1.25"	70'	1.75"	100'	2.50"	150'	3.25"	220'	4.25"	300'	5.00"	380'	6.00"	480'
2°00'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	1.00"	50'	1.50"	80'	2.25"	130'	3.25"	200'	4.00"	260'	5.00"	350'	6.00"	450'		
2° 15'	0.00"	30'	0.00"	30'	0.00"	30'	0.00"	30'	0.75"	40'	1.25"	70'	2.00"	100'	3.00"	170'	3.75"	230'	4.75"	310'	5.75"	410'				
2°30'	0.00"	30'	0.00"	30'	0.00"	30'	0.25"	30'	1.00"	50'	1.75"	90'	2.50"	130'	3.50"	200'	4.50"	270'	5.50"	360'						
2°45'	0.00"	30'	0.00"	30'	0.00"	30'	0.50"	30'	1.25"	70'	2.00"	100'	3.00"	150'	4.00"	220'	5.00"	300'								
3°00'	0.00"	30'	0.00"	30'	0.00"	30'	0.75"	40'	1.50"	80'	2.50"	130'	3.25"	170'	4.50"	250'	5.75"	350'								
3° 15'	0.00"	30'	0.00"	30'	0.25"	30'	1.00"	50'	1.75"	90'	2.75"	140'	3.75"	190'	5.00"	280'										
3° 30'	0.00"	30'	0.00"	30'	0.25"	30'	1.25"	70'	2.00"	100'	3.00"	150'	4.25"	220'	5.50"	310'										
3° 45'	0.00"	30'	0.00"	30'	0.50"	30'	1.25"	70'	2.25"	120'	3.50"	180'	4.75"	240'	6.00"	330'										
4°00'	0.00"	30'	0.00"	30'	0.75"	40'	1.50"	80'	2.50"	130'	3.75"	190'	5.00"	250'												
4°15'	0.00"	30'	0.00"	30'	0.75"	40'	1.75"	90'	3.00"	150'	4.25"	220'	5.50"	280'												
4°30'	0.00"	30'	0.00"	30'	1.00"	50'	2.00"	100'	3.25"	170'	4.50"	230'	6.00"	300'												
[ 4° 45'	0.00"	30'	0.25"	30'	1.00"	50'	2.25"	120'	3.50"	180'	4.75"	240'														
\(\begin{array}{c} \equiv 5 \cdot 00' \\ \equiv \equiv \equiv   \qu	0.00"	30'	0.25"	30'	1.25"	70'	2.50"	130'	3.75"	190'	5.25"	270'														
_ 5° 15'	0.00"	30'	0.50"	30'	1.50"	80'	2.75"	140'	4.00"	200'	5.50"	280'														
₹ 5°30'	0.00"	30'	0.50"	30'	1.50"	80'	2.75"	140'	4.25"	220'	6.00"	300'														
S 5° 45'	0.00"	30'	0.75"	40'	1.75"	90'	3.00"	150'	4.50"	230'																
문 6°00'	0.00"	30'	0.75"	40'	2.00"	100'	3.25"	170'	4.75"	240'																
음 6° 15'	0.00"	30'	0.75"	40'	2.00"	100'	3.50"	180'	5.00"	250'																
□ 6° 30'	0.00"	30'	1.00"	50'	2.25"	120'	3.75"	190'	5.50"	280'																
6° 45'	0.00"	30'	1.00"	50'	2.50"	130'	4.00"	200'	5.75"	290'																
를 <u>7°00'</u>	0.00"	30'	1.25"	70'	2.50"	130'	4.25"	220'	6.00"	300'																
₹ 7° 15'	0.25"	30'	1.25"	70'	2.75"	140'	4.25"	220'																		
≥ 7° 30'	0.25"	30'	1.50"	80'	2.75"	140'	4.50"	230'																		
공 <u>7°45'</u>	0.25"	30'	1.50"	80'	3.00"	150'	4.75"	240'																		
8,00,	0.25"	30'	1.50"	80'	3.25"	170'	5.00"	250'																		
8° 15'	0.50"	30'	1.75"	90'	3.25"	170'	5.25"	270'			1															
8° 30'	0.50"	30'	1.75"	90'	3.50"	180'	5.50"	280'																		
8° 45'	0.50"	30'	2.00"	100'	3.75"	190'	5.75"	290'		NOT	TES:															
9°00'	0.75"	40'	2.00"	100'	3.75"	190'	5.75"	290'				NIS OF	SIIDEDE	F \/ \ T	ONS WII	l BE D	FRMITTEI	п то т	HE BIUM.	ד עב הו	EAVY LIN	VE WIT□	INIIT DDI		ROVAL :	
9° 15'	0.75"	40'	2.25"	120'	4.00"	200'	6.00"	300'									AND CO			ı or mi	LAVI LII	N∟ WIII⊓	IOUI FRI	ON AFF	NOVAL	
9° 30'	0.75"	40'	2.25"	120'	4.00"	200'				2.	THIS TA	BLE MA	Y ONLY	BE USE	D ON T	HE VEN	TURA AN	ID ANTE	ELOPE V		SUBDIVIS		T LOCAT	IONS WE	HERE	
9° 45'	0.75"	40'	2.50"	130'	4.25"	220'				3	STANDAI	RD SPIR	AL LENG IRF IS N	THS CA	N NOTE	BE OBL	AINED DU MORF T	JE 10 1 HANIA	EXISTING	FIELD	CONDITION THE NEX	ONS. Yt hich	FR FIF	' A T I O N	A NI D	
10°00'	1.00"	50'	2.50"	130'	4.50"	230'	-								L BE US		WIOIL I	IIUN W	LIJILU	i idont,	IIIL NE/	хт шоп	LN LLLV	ATTON	TIND	
10° 15'	1.00"	50'	2.50"	130'	4.50"	230'	-					1	1		1	ı		1			1	1	1			
10° 30'	1.00"	50'	2.75"	140'	4.75"	240'	-				1															
10° 45'	1.25"	70'	2.75"	140'	5.00"	250'	-				-												-			
110 00'	1.25"	70'	3.00"	150'	5.00"	250'					1	-											-			
11° 15'	1.25"	70'	3.00"	150'	5.25"	270'					1	-											1			
11° 30'	1.25"	70'	3.25"	170'	5.25"	270'											1	-		-	1	-	1			
	1.50"	80'	3.25"	170' 170'	5.50"	280'												-		-		-	1			
12°00'	1.50"	80'	3.25"	170	5.75"	290'										<u> </u>										

Х	xx-xx-xx	REVISION	XX	XX
REV.	DATE	DESCRIPTION	DES.	ENG.

UserName=> carlosa

A. CARLOS DATE: Varet D. Pale ASSISIANT DIRECTOR: STANDARDS & DESIGN Illian Davar

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### **METROLINK**

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS

TABLE F2.0M - 2.0 INCH UNBALANCED ELEVATION MINIMUM SPIRAL LENGTH TABLE FOR FREIGHT OPERATIONS

STANDARD	
	2204
CONE	2207
SCALE:	
	NTS
BELLIOIS I	
REVISION	SHEET
_	4 OF 6
CADD FILE:	
	FS2204-04

#### TABLE PML - 4.0 INCH UNBALANCED ELEVATION FOR PASSENGER OPERATIONS - MAINTENANCE LIMIT

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN)

Vmax = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

6.5

0.00'

0.00'

0.00'

0.00'

0.00

0.50"

1.25"

2.00'

2.75'

3.50'

4.25'

5.00'

5.75"

70

0.00"

0.00'

0.00'

0.00'

0.50

1.25

2.25'

3.00'

3.75

4 75

5.50

7.5

0.00'

0.00"

0.00'

0.00"

1 00'

2.00"

3.00'

4.00'

5.00'

6.00

0.00

0.00

0.00

0.50"

1.75"

2.75"

4.00"

5.00"

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

Ea = ACTUAL ELEVATION OF OUTSIDE RAIL (IN) D = DEGREE OF CURVATURE (DECIMAL DEGREES)  $E = 0.0007DVmax^2$ Ea = E - Eu

85

0.00"

0.00'

0.00'

1.25"

2.50'

3.75'

5.00"

FORMULAS

90

0.00'

0.00'

0.50"

1.75"

3.25"

4.75

6.00"

95

0.00"

0.00"

0.75"

2.50"

4.00'

5.50"

100

0.00'

0.00"

1.25"

3.00'

4.75

105

0.00'

0.00'

2.00'

5.75"

110

0.00'

0.25'

2.50'

4.50"

MAXIMUM ALLOWABLE PASSENGER OPERATING SPEED - MILES PER HOUR 20 25 30 35 40 45 50 55 0° 15' 0.00' 0.00' 0.00 0.00' 0.00" 0.00 0.00' 0.00' 0°30 0.00' 0.00' 0.00' 0.00' 0.00" 0.00" 0.00' 0.00' 0°45' 0.00' 0.00 0.00' 0.00' 0.00' 0.00 0.00" 0.00 1°00' 0.00" 0.00' 0.00 0.00' 0.00' 0.00" 0.00' 0.00' 1° 15' 0.00" 0.00' 0.00 0.00' 0 00' 0.00" 0.00' 0.00 1° 30' 0.00' 0.00" 0.00' 0.00' 0.00' 0.00" 0.00" 0.00" 1° 45' 0.00" 0.00' 0.00' 0.00" 0.00' 0.00' 0.00 0.00' 2°00 0.00' 0.00" 0.00' 0.00' 0.00' 0.00" 0.00" 0.25" 2° 15' 0.00 0.00 1.00' 0.00' 0.00' 0.00 0.00" 0.00' 2°30 0.00' 0.00 0.00 0.00 1.50' 0.00' 0.00' 0.50' 2°45' 0.00 2.00' 0.00 0.00 0.00 0.00' 0.00 1.00 3°00' 0.00' 0.00' 0.00 0.00' 0.00' 0.50" 1.25 2.50 3° 15' 0.00' 0.00' 0.00 0.00' 0.00' 3.00' 0.75'1.75 3° 30' 0.00' 0.00' 0.00 0.00' 0.00' 1.00' 2.25' 3.50' 3° 45' 0.00' 0.25" 1.50' 0.00 0.00' 0.00' 2.75" 4.00' 4°00' 0.00 0.00' 0.00' 0.00' 0.50" 1.75 3.00 4.50' 5.00" 4°15' 0.00' 0.00' 0.00' 1.00" 2.25" 3.50 0.00 4°30' 5.75 0.00' 0.00' 0.00 0.00' 1.25" 2.50" 4.00' 4°45' 0.00' 0.00' 0.00 0.25 1.50" 2.75" 4.50' 5°00 0.00" 0.00' 0.00 0.50' 1.75 3.25" 4.75" 0.75 5° 15 0.00" 0.00" 0.00' 2.00' 3.50" 5.25" 5°30 0.00" 0.00" 0.00' 0.75" 2.25" 4.00" 5.75" 5° 45' 0.00" 0.00" 0.00' 1 0 0 2.50" 4.25" 6°00 0.00' 0.00' 0.00 1.25 2.75" 4.75 5.00" 6° 15' 1.50' 0.00' 0.00 0.00 3.00' 6°30 0.00' 0.25 1.75 3.50' 5.25 0.00 6°45 0.00' 0.00' 0.50 2.00' 3.75 5.75" 2.25 4.00' 7000 0.00 0.00 0.50' 6.00" 7°15' 2.25' 4.25' 0.00" 0.00' 0.75' 7°30' 0.75" 2.50" 4.50" 0.00' 0.00' 7°45' 0.00' 0.00' 1.00' 2.75" 4.75" 1.25 3.00' 5.00" 8°00' 0.00 0.00' 1.25 3.25 8° 15' 0.00' 0.00' 5.25' 8°30' 1.50" 3.50" 5.75" 0.00 0.00' 8°45' 0.00" 0.00' 1.75 3.75" 6.00" 1.75" 3.75" 9°00' 0.00" 0.00' 2.00" 4.00" 9° 15 0.00" 0.25" 9°30' 4.25" 0.00" 0.25" 2.00" 9°45' 0.00' 0.50" 2.25" 4.50" 10°00 2.50' 4.75' 0.00' 0.50' 2.50' 5.00' 10° 15' 0.00' 0.50' 10°30' 0.00' 0.75'2.75' 5.25 10° 45' 0.00' 0.75 3.00' 5.25' 11° 00' 0.00' 1.00 3.00' 5.50" 11° 15' 0.00" 1.00" 3.25" 5.75" 11° 30' 0.00' 1.25" 3.25" 6.00" 11° 45' 0.00 1.25" 3.50' 12°00' 0.00' 1.25 3.75"

#### NOTES:

0.00"

0.00"

0.00"

0.00'

0.00"

0.00"

0.50"

1.25"

1.75

2.50

3.00'

3.75

4.25"

5.00"

5.50"

- AT ALL TIMES THE TRACK MUST BE IN CONFORMANCE WITH 49CFR213. TABLES P3.5 AND P3.5M DEFINE THE LIMITING DESIGN SPEED FOR PASSENGER TRAINS. TABLES F2.0 AND F2.0M DEFINE THE LIMITING DESIGN SPEED FOR FREIGHT TRAINS. OPERATION AT SPEEDS RESULTING IN 4 INCHES UNDERBALANCE IS PERMITTED FOR SCRRA AND AMTRAK PASSENGER TRAINS EXCEPT WHEN ADVISED THAT SEVERE WIND CONDITIONS EXIST. 3 INCHES UNDERBALANCE IS THE LIMITING CONDITION FOR ALL FREIGHT TRAINS AND FOR PASSENGER TRAINS UNDER SEVERE WIND CONDITIONS. ANY COMBINATION OF CURVATURE OR ACTUAL ELEVATION THAT IS DISCOVERED OR CREATED THAT RESULTS IN THE OPERATING SPEED TO EXCEED THE SPEED PERMITTED BY THESE TABLES REQUIRES IMMEDIATE REMEDIAL ACTION.
- SOME CURVES WERE CONSTRUCTED AND SPEEDS ESTABLISHED WITH UNDERBALANCE FOR PASSENGER SPEEDS BETWEEN THE 3.5 INCH DESIGN VALUE OF TABLES P3.5 AND P3.5M AND THE 4 INCH LIMITING VALUE PER THE FRA. CURVES WITH THESE CHARACTERISTICS WILL BE MAINTAINED AS DESIGNED. SUPERELEVATION AND SPIRAL LENGTHS WILL BE MAINTAINED TO THE VALUES RECORDED IN THE SCRRA TRACK CHARTS. SOME OF THESE DO NOT MEET THE LENGTH REQUIREMENTS FOR THE TABLES FOR NEW
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- THE SHARPNESS OF THE CURVE IN THE CENTRAL BODY WILL BE INCREASED IF THE SPIRALS ARE EXTENDED INTO THE BODY OF THE CURVE.
- CONTRACT TRACK INSPECTORS WILL FIELD VERIFY THE CHARACTERISTICS OF AT LEAST TWO CURVES EACH MONTH, USING TRACK LEVEL AND STRING LINE, REPORTING THE OBSERVED 62-FOOT CHORD MID-ORDINATE AND SUPERELEVATION AT 15.5-FOOT INTERVALS FOR THE LENGTH OF THE CURVE. THE MANAGERS OF TRACK MAINTENANCE AND THE CONTRACT PROJECT MANAGER WILL REVIEW AND COMPARE THE PRECEDING TWO YEARS OF TRACK GEOMETRY DATA TO THE TRACK CHART DATA, AND WILL ARRANGE FOR FIELD VERIFICATION OF ALIGNMENT BASED UPON THESE REVIEWS
- MANAGERS OF TRACK MAINTENANCE MUST RIDE WITH EACH OPERATION OF TRACK GEOMETRY CARS. THEY MUST MONITOR AND ENSURE THAT THE MAINTENANCE CONTRACTOR INVESTIGATES ANY NOTED REPORTS OF WARP OR UNDERBALANCE EXCEPTIONS AND TAKES THE REQUIRED REMEDIAL ACTIONS (SPOT REPAIRS OR REDUCTION IN SPEED). THEY MUST ALSO PROMPTLY REVIEW THE CURVE DATA GENERATED BY THE TRACK GEOMETRY CAR AND COMPARE THE AVERAGE CURVATURE, AVERAGE ELEVATION, LIMITING CURVATURE AND LIMITING ELEVATION FOR EACH CURVE TO THE RECORDS IN THE TRACK CHARTS WHETHER AN EXCEPTION
- COMPOUND CURVES DESCRIBED IN THE TRACK CHARTS THAT HAVE DIFFERING TRAIN SPEED, SUPERELEVATION, AND/OR CURVATURE NOTED FOR TWO OR MORE SEGMENTS OF ONE CURVE HAVE BEEN APPROVED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- IF THE ACTUAL SUPERELEVATION AND CURVATURE MEASURED IN THE FIELD BY GEOMETRY CARS OR BY MANUAL INSPECTION PER NOTE 5 ABOVE ARE FOUND TO RESULT IN AN ALLOWABLE SPEED LESS THAN PERMITTED BY TABLES PML AND FML, A TEMPORARY SPEED REDUCTION MUST BE IMPOSED TO THE NEXT LOWER SPEED THAT WILL ACCOMMODATE THE ACTUAL MEASURED SUPERELEVATION. THE TEMPORARY SPEED REDUCTION MUST REMAIN UNTIL THE SUPERELEVATION LIMITS ARE RAISED TO THE VALUES SHOWN IN TABLES P3.5, F2.0, P3.5M AND F2.0M FOR THE DESIGN SPEED.

TABLE

					DRAWN BY:	A. CARLOS	DATE:	03/31/20
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					/	jaresh	2. Vall	_
					ASSISTAN	IT DIRECTOR:	STANDARDS & DE	SIGN
Х	xx-xx-xx	REVISION	XX	XX	u)	llon	- Dav	an
REV.	DATE	DESCRIPTION	DES.	ENG.	DIRECTOR	OF ENGINEERI	NG AND CONSTRUC	TION

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ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

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SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

PML - 4.0	INCH	UNBALANCED	<b>ELEVATION</b>

ENGINEERING STANDARDS

MAINTENANCE LIMIT FOR PASSENGER OPERATIONS

		NTS
REVISION	SHEET	
_	5	OF 6
CADD FILE:		
	ES22	04-05

2204

#### TABLE FML - 3.0 INCH UNBALANCED ELEVATION FOR FREIGHT OPERATIONS - MAINTENANCE LIMIT

60

0.00"

0.00"

0.00"

0.00"

0.25"

1.00"

1.50"

2.25"

2.75"

3.50"

4.00"

4.75"

5.25" 6.00"

ABBREVIATIONS

E = EQUILIBRIUM ELEVATION OF OUTSIDE RAIL (IN) Vmax = MAXIMUM ALLOWABLE OPERATING SPEED (MPH)

Eu = UNBALANCED ELEVATION OF OUTSIDE RAIL (IN) Ls = SPIRAL LENGTH (FT)

Ea = ACTUAL ELEVATION OF OUTSIDE RAIL (IN)

D = DEGREE OF CURVATURE (DECIMAL DEGREES)

6.5

0.00"

0.00"

0.00"

0.00"

0.75"

1.50"

2.25"

3.00"

3.75

4.50"

5.25

6.00"

FREIGHT OPERATING SPEED - MILES PER HOUR

70

0.00"

0.00'

0.00'

0.50'

1.50

2.25'

3.25'

4.00'

4.75

75

0.00"

0.00"

0.00"

1.00"

2.00"

3.00"

4.00"

5 00'

6.00'

80

0.00

0.00'

0.50

1.50'

2.75"

3.75"

5.00"

6.00"

 $E = 0.0007DVmax^2$ Ea = E - Eu

FORMULAS

								MAXIMUN	M ALLOWA	BLE
		20	25	30	35	4 0	4.5	50	55	T
	0°15'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	+
	0°30'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	+
	0° 45'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	+
	1° 00'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	T
	1° 15 '	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	
	1° 30'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.25"	
	1° 45'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.25"	0.75"	
	2°00'	0.00"	0.00"	0.00"	0.00"	0.00"	0.00"	0.50"	1.25"	
	2°15'	0.00"	0.00"	0.00"	0.00"	0.00"	0.25"	1.00"	2.00"	
	2° 30'	0.00"	0.00"	0.00"	0.00"	0.00"	0.75"	1.50"	2.50"	
	2°45'	0.00"	0.00"	0.00"	0.00"	0.25"	1.00"	2.00"	3.00"	
	3°00'	0.00"	0.00"	0.00"	0.00"	0.50"	1.50"	2.25"	3.50"	
	3° 15'	0.00"	0.00"	0.00"	0.00"	0.75"	1.75"	2.75"	4.00"	
	3° 30'	0.00"	0.00"	0.00"	0.25"	1.00"	2.00"	3.25"	4.50"	
	3° 45'	0.00"	0.00"	0.00"	0.25"	1.25"	2.50"	3.75"	5.00"	
	4°00'	0.00"	0.00"	0.00"	0.50"	1.50"	2.75"	4.00"	5.50"	J
1,0	4°15'	0.00"	0.00"	0.00"	0.75"	2.00"	3.25"	4.50"	6.00"	- <u>N</u>
LES	4°30'	0.00"	0.00"	0.00"	1.00"	2.25"	3.50"	5.00"		_ ''
NUT	4°45'	0.00"	0.00"	0.00"	1.25"	2.50"	3.75"	5.50"		
≥	5°00'	0.00"	0.00"	0.25"	1.50"	2.75"	4.25"	5.75"		
	5° 15'	0.00"	0.00"	0.50"	1.75"	3.00"	4.50"			
AN	5° 30'	0.00"	0.00"	0.50"	1.75"	3.25"	5.00"			
ES	5° 45'	0.00"	0.00"	0.75"	2.00"	3.50"	5.25"			
GREI	6°00'	0.00"	0.00"	1.00"	2.25"	3.75"	5.75"			
لبا	6° 15'	0.00"	0.00"	1.00"	2.50"	4.00"	6.00"			
	6° 30'	0.00"	0.00"	1.25"	2.75"	4.50"				
١.'.	6° 45'	0.00"	0.00"	1.50"	3.00"	4.75"				
JRE	7°00'	0.00"	0.25"	1.50"	3.25"	5.00"				
ATU	7°15'	0.00"	0.25"	1.75"	3.25"	5.25"				
CURV	7°30'	0.00"	0.50"	1.75"	3.50"	5.50"				
CO	7°45'	0.00"	0.50"	2.00"	3.75"	5.75"				
	8,00,	0.00"	0.50"	2.25"	4.00"	6.00"				
	8° 15'	0.00"	0.75"	2.25"	4.25"					_
	8°30'	0.00"	0.75"	2.50"	4.50"					_
	8° 45'	0.00"	1.00"	2.75"	4.75"					_
	9°00'	0.00"	1.00"	2.75"	4.75"					_
	9°15'	0.00"	1.25"	3.00"	5.00"					_
	9°30'	0.00"	1.25"	3.00"	5.25"					_
	9°45'	0.00"	1.50"	3.25"	5.50"					_
	10°00'	0.00"	1.50"	3.50"	5.75"					_
	10° 15'	0.00"	1.50"	3.50"	6.00"					_
	10° 30'	0.00"	1.75"	3.75"						-
	10° 45'	0.25"	1.75"	4.00"						_
	11° 00'	0.25"	2.00"	4.00"						-
	11° 15'	0.25"	2.00"	4.25"						_
	11° 30'	0.25"	2.25"	4.25"						-
	11° 45'	0.50"	2.25"	4.50"						_
	12°00'	0.50"	2.25"	4.75"					<u> </u>	

#### NOTES:

- 1. AT ALL TIMES THE TRACK MUST BE IN CONFORMANCE WITH 49CFR213. TABLES P3.5 AND P3.5M DEFINE THE LIMITING DESIGN SPEED FOR PASSENGER TRAINS. TABLES F2.0 AND F2.0M DEFINE THE LIMITING DESIGN SPEED FOR FREIGHT TRAINS. OPERATION AT SPEEDS RESULTING IN 4 INCHES UNDERBALANCE IS PERMITTED FOR SCRRA AND AMTRAK PASSENGER TRAINS EXCEPT WHEN ADVISED THAT SEVERE WIND CONDITIONS EXIST. 3 INCHES UNDERBALANCE IS THE LIMITING CONDITION FOR ALL FREIGHT TRAINS AND FOR PASSENGER TRAINS UNDER SEVERE WIND CONDITIONS. ANY COMBINATION OF CURVATURE OR ACTUAL ELEVATION THAT IS DISCOVERED OR CREATED THAT RESULTS IN THE OPERATING SPEED TO EXCEED THE SPEED PERMITTED BY THESE TABLES REQUIRES IMMEDIATE REMEDIAL ACTION.
- SOME CURVES WERE CONSTRUCTED AND SPEEDS ESTABLISHED WITH UNDERBALANCE FOR PASSENGER SPEEDS BETWEEN THE 3.5 INCH DESIGN VALUE OF TABLES P3.5 AND P3.5M AND THE 4 INCH LIMITING VALUE PER THE FRA. CURVES WITH THESE CHARACTERISTICS WILL BE MAINTAINED AS DESIGNED. SUPERELEVATION AND SPIRAL LENGTHS WILL BE MAINTAINED TO THE VALUES RECORDED IN THE SCRRATRACK CHARTS. SOME OF THESE DO NOT MEET THE LENGTH REQUIREMENTS FOR THE TABLES FOR NEW
- DESIGN, P3.5 AND F2.0. HOWEVER, THEY DO MEET THE REQUIREMENTS FOR THE P3.5M AND P2.0M TABLES. SPIRAL LENGTHS MUST NOT BE INCREASED EXCEPT AS PART OF AN ENGINEERED REALIGNMENT OF A CURVE THE SHARPNESS OF THE CURVE IN THE CENTRAL BODY WILL BE INCREASED IF THE SPIRALS ARE EXTENDED
- INTO THE BODY OF THE CURVE. CONTRACT TRACK INSPECTORS WILL FIELD VERIFY THE CHARACTERISTICS OF AT LEAST TWO CURVES EACH MONTH, USING TRACK LEVEL AND STRING LINE, REPORTING THE OBSERVED 62-FOOT CHORD MID-ORDINATE AND SUPERELEVATION AT 15.5-FOOT INTERVALS FOR THE LENGTH OF THE CURVE. THE MANAGERS OF TRACK MAINTENANCE AND THE CONTRACT PROJECT MANAGER WILL REVIEW AND COMPARE THE PRECEDING TWO YEARS OF TRACK GEOMETRY DATA TO THE TRACK CHART DATA, AND WILL ARRANGE FOR FIELD VERIFICATION OF ALIGNMENT BASED UPON THESE REVIEWS.
- 6. MANAGERS OF TRACK MAINTENANCE MUST RIDE WITH EACH OPERATION OF TRACK GEOMETRY CARS. THEY MUST MONITOR AND ENSURE THAT THE MAINTENANCE CONTRACTOR INVESTIGATES ANY NOTED REPORTS OF WARP OR UNDERBALANCE EXCEPTIONS AND TAKES THE REQUIRED REMEDIAL ACTIONS (SPOT REPAIRS OR REDUCTION IN SPEED). THEY MUST ALSO PROMPTLY REVIEW THE CURVE DATA GENERATED BY THE TRACK GEOMETRY CAR AND COMPARE THE AVERAGE CURVATURE, AVERAGE ELEVATION, LIMITING CURVATURE AND LIMITING ELEVATION FOR EACH CURVE TO THE RECORDS IN THE TRACK CHARTS WHETHER AN EXCEPTION
- 7. COMPOUND CURVES DESCRIBED IN THE TRACK CHARTS THAT HAVE DIFFERING TRAIN SPEED, SUPERELEVATION, AND/OR CURVATURE NOTED FOR TWO OR MORE SEGMENTS OF ONE CURVE HAVE BEEN APPROVED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.

8. IF THE ACTUAL SUPERELEVATION AND CURVATURE MEASURED IN THE FIELD BY GEOMETRY CARS OR BY MANUAL INSPECTION PER NOTE 5 ABOVE ARE FOUND TO RESULT IN AN ALLOWABLE SPEED LESS THAN PERMITTED BY TABLES PML AND FML, A TEMPORARY SPEED REDUCTION MUST BE IMPOSED TO THE NEXT LOWER SPEED THAT WILL ACCOMMODATE THE ACTUAL MEASURED SUPERELEVATION. THE TEMPORARY SPEED REDUCTION MUST REMAIN UNTIL THE SUPERELEVATION LIMITS ARE RAISED TO THE VALUES SHOWN IN TABLES P3.5, F2.0, P3.5M AND F2.0M FOR THE DESIGN SPEED.

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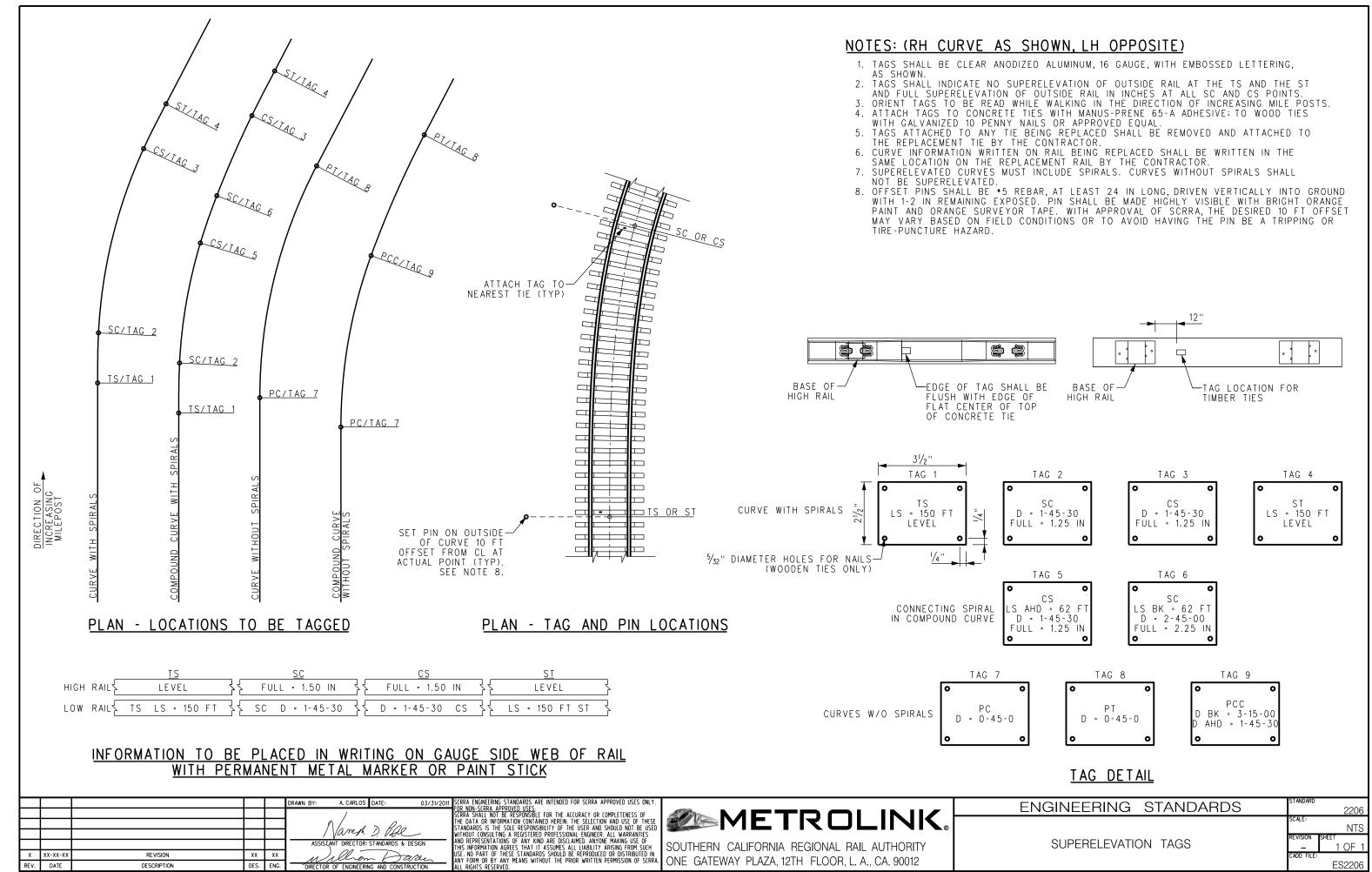
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SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

<b>TABLE</b>	FML	- 3.0	INC	H UI	NBALANC	CED	ELEVA	TION
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ENGINEERING STANDARDS

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	SPACING OF TRA	CKS ON CURVES				
DEGREE		DISTANCE BETWEEN TRACK CENTERS- SUPERELEVATION SAME ON EACH TRACK- SEE NOTE 2				
OF CURVE		JNNING AND T TRACKS	INDUSTRY AND YARD TRACKS			
TANGENT	15'-0"	16'-0"	15'-0"			
1°	15'-2"	16'-2"	15'-0"			
2°	15'-4"	16'-4"	15'-0"			
3°	15'-6"	16'-6"	15'-0"			
4 °	15'-8"	16'-8"	15'-0"			
5°	15'-10"	16'-10"	15'-0"			
6°	16'-0"	17'-0"	15'-0"			
7°	16'-2"	17'-2"	15'-2"			
8°	16'-4"	17'-4"	15'-4"			
9°	16'-6"	17'-6"	15'-6"			
10°	16'-8"	17'-8"	15'-8"			
11°	16'-10"	17'-10"	15'-10"			
12°	17'-0"	18'-0"	16'-0"			
13°	17'-2"	18'-2"	16'-2"			
14°	17'-4"	18'-4"	16'-4"			
15°	17'-6"	18'-6"	16'-6"			
OVER 15°	INCREASE BY 1/2	INCH PER 15 MINU	JTES OF CURVE			

DEGREE	SEE NOTE 2				
OF CURVE		MAIN OR RUNNING AND ADJACENT TRACKS			
TANGENT	15'-0"	16'-0"	15'-0"		
1°	15'-2"	16'-2"	15'-0"		
2°	15'-4"	16'-4"	15'-0"		
3°	15'-6"	16'-6"	15'-0"		
4 °	15'-8"	16'-8"	15'-0"		
5°	15'-10"	16'-10"	15'-0"		
6°	16'-0"	17'-0"	15'-0"		
7°	16'-2"	17'-2"	15'-2"		
8°	16'-4"	17'-4"	15'-4"		
9°	16'-6"	17'-6"	15'-6"		
10°	16'-8"	17'-8"	15'-8"		
11°	16'-10"	17'-10''	15'-10"		
12°	17'-0"	18'-0"	16'-0"		
13°	17'-2"	18'-2"	16'-2"		
14°	17'-4"	18'-4"	16'-4"		
15°	17'-6"	18'-6"	16'-6"		
OVER 15°	INCREASE BY 1/2	INCH PER 15 MINU	JTES OF CURVE		

NEW
APPLY
DIDEC

1. MINIMUM DISTANCE BETWEEN CENTER LINES OF ADJACENT TRACKS ON ALL CONSTRUCTION SHALL BE AS FOLLOWS: (THIS MINIMUM DISTANCE WILL ALSO Y TO EXISTING TRACKS WHEN RESPACING IS AUTHORIZED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.)

A. MAIN TRACKS --15'-0" MINIMUM, 25'-0" WHERE SPACE PERMITS B. MAIN SIDING, RUNNING AND DRILL TRACKS AND ADJACENT TRACK (EXCEPT YARD TRACK)

C. LADDER TRACK AND ADJACENT TRACK -15'-0"

-20'-0" D. INDUSTRY, YARD AND HOUSE TRACKS 15'-0"

E. YARD TRACK AND ADJACENT MAIN OR RUNNING TRACK—25'-0"

E. YARD TRACK AND ADJACENT MAIN OR RUNNING TRACK—25'-0"

F. ON CURVES, TRACK CENTERS AS SHOWN ABOVE SHALL BE
INCREASED AS FOLLOWS (ALSO SEE TABLE ON THIS SHEET):

a. TRACKS PER NOTES A, B AND E - INCREASE BY ½ INCH PER
EACH 15 MINUTES OF CURVE.

b. TRACKS PER NOTE D (YARD TRACKS) - INCREASE BY ½ INCH
PER EACH 15 MINUTES OF CURVE IN EXCESS OF 6 DEGREES.

2. INCREASED DISTANCES BETWEEN TRACK CENTERS SHALL BE APPLIED IN ½ INCH INCREMENTS.
DEGREES OF CURVATURE THAT ARE NOT EXACT 15 MINUTE INCREMENTS SHALL BE ROUNDED UP
TO THE NEXT GREATER 15 MINUTE INCREMENT. FOR EXAMPLE, IF TWO CURVED TRACKS ARE TO BE
PARALLEL AND THE INNER TRACK IS D-8°15' 10", THEY SHALL BE SEPARATED BASED ON THE
ASSUMPTION THAT ITS CURVATURE IS D-8°30'

ASSUMPTION THAT ITS CURVATURE IS D=8°30'.

3. WHERE ADJACENT TRACK IS ON THE OUTSIDE OF A CURVE AND ITS SUPERELEVATION IS MORE THAN ON THE INSIDE TRACK, DISTANCE BETWEEN THE TRACKS SHALL BE INCREASED THREE INCHES FOR EACH INCH DIFFERENCE IN SUPERELEVATION. THE INCREASE SHALL BE ADDED TO THE AMOUNT SHOWN IN TABLE AT LEFT. WHERE SUCH TRACK HAS THE SAME OR LESS AMOUNT OF SUPERELEVATION, USE SPACING AS SHOWN IN THE TABLE.

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ENGINEERING	STANDARDS	STANDARD
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TRACK CENTER SPACING

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#### MAXIMUM SPEEDS THROUGH TURNOUTS, SPRING SWITCHES AND SLIP SWITCHES

SUBJECT TO SPEED RESTRICTIONS IMPOSED BY LOCAL CONDITIONS, OTHER THAN THE NUMBER OF THE TURNOUT OR TYPE OF SWITCH, THE FOLLOWING WILL GOVERN THE MAXIMUM SPEEDS PERMITTED THROUGH TURNOUTS AND OVER SPRING SWITCHES AND SLIP SWITCHES:

			FREIGHT					
TURNOUT NO	TANGE	NTIAL	STAN	DARD	EQUILATERAL (MPH)	DOUBLE SLIP (MPH)		
-	SWITCH LENGTH (POINTS)	MPH	SWITCH LENGTH (POINTS)	MPH	-	-		
8	-	-	16'-6"	10	N/A	10		
9	-	-	16'-6"	10	N/A	N/A		
10	21'-6"	15	19'-6"	15	N/A	10		
11	-	-	19'-6"	15	N/A	N/A		
14	29'-0"	25	26'-0"	20	N/A	N/A		
15	-	-	26'-0"	20	N/A	N/A		
20	47'-0"	4 0	39'-0"	35	50	N/A		
2 4	61'-6" 50		39'-0"	40	60	N/A		
30	82'-0"	60	-	N/A	80	N/A		
	- 8 9 10 11 14 15 20 24	- SWITCH LENGTH (POINTS)  8 - 9 - 10 21'-6"  11 - 14 29'-0"  15 - 20 47'-0"  24 61'-6"	- SWITCH LENGTH MPH  8 9 10 21'-6" 15  11 14 29'-0" 25  15 20 47'-0" 40  24 61'-6" 50	TURNOUT NO         TANGENTIAL         STAN           -         SWITCH LENGTH (POINTS)         MPH         SWITCH LENGTH (POINTS)           8         -         -         16'-6"           9         -         -         16'-6"           10         21'-6"         15         19'-6"           11         -         -         19'-6"           14         29'-0"         25         26'-0"           15         -         26'-0"           20         47'-0"         40         39'-0"           24         61'-6"         50         39'-0"	TURNOUT NO         TANGENTIAL         STANDARD           -         SWITCH LENGTH (POINTS)         MPH           8         -         -           9         -         -           10         21'-6"         15           11         -         -           14         29'-0"         25           20         47'-0"         40           24         61'-6"         50           39'-0"         40	TURNOUT NO         TANGENTIAL         STANDARD         EQUILATERAL (MPH)           -         SWITCH LENGTH (POINTS)         MPH         -           8         -         -         16'-6"         10         N/A           9         -         -         16'-6"         10         N/A           10         21'-6"         15         19'-6"         15         N/A           11         -         -         19'-6"         15         N/A           14         29'-0"         25         26'-0"         20         N/A           15         -         -         26'-0"         20         N/A           20         47'-0"         40         39'-0"         35         50           24         61'-6"         50         39'-0"         40         60		

PASSENGER														
TURNOUT NO	TANGE	NTIAL	STAN	DARD	EQUILATERAL (MPH)	DOUBLE SLIP (MPH)								
-	SWITCH LENGTH (POINTS)	MPH	SWITCH LENGTH (POINTS)	MPH	-	-								
8	-	-	16'-6"	12	N/A	12								
9	-	-	16'-6"	12	N/A	N/A								
10	21'-6"	25	16'-6"	20	N/A	15								
11	-	-	19'-6"	20	N/A	N/A								
14	29'-0"	35	26'-0"	30	N/A	N/A								
15	-	-	26'-0"	30	N/A	N/A								
20	47'-0"	50	39'-0"	45	70	N/A								
24	61'-6"	60	39'-0"	55	85	N/A								
30	82'-0"	75	-	N/A	110	N/A								

#### NOTE:

1. MAXIMUM SPEEDS WERE CALCULATED BASED ON TURNOUT GEOMETRY WITH Ea = 0" AND ASSUMED Eu = 3.5" FOR PASSENGER TRAINS AND Eu = 2.0" FOR FREIGHT

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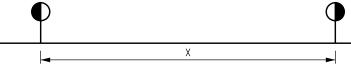
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CH O IN	SOUTHERN	CALIFORNIA REGIONAL RAIL AUTHORITY

ENGINEERING STANDARDS	STANDARD 2208
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SPEED THROUGH TURNOUTS	REVISION SHEET  - 1 OF 1
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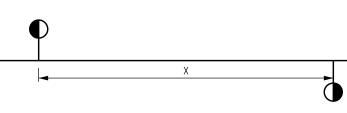
Varet D. Poll ASSISTANT DIRECTOR: STANDARDS & DESIGN Million Davae
DIRECTOR OF ENGINEERING AND CONSTRUCTION

#### FACING TURNOUTS OF OPPOSITE HAND



FROG NO	DESIRABLE X (FT)	MINIMUM X (FT)					
8, 10	82	46					
14	122	86					
20	N/A	118					
24	N/A	150					

#### FACING TURNOUTS OF LIKE HAND



FROG NO	DESIRABLE X (FT)	MINIMUM X (FT)				
8, 10	82	52				
14	125	90				
20	N/A	122				
2 4	N/A	150				

#### NOTES:

- DESIGN SPEED, SIGNAL SPACING AND CIRCUITS WILL GOVERN AT LOCATIONS WHERE INSULATED JOINTS ARE REQUIRED.
   ANY DISTANCE BETWEEN FACING POINTS OF SWITCH LESS THAN THE MINIMUMS GIVEN SHALL REQUIRE THE APPROVAL OF THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.

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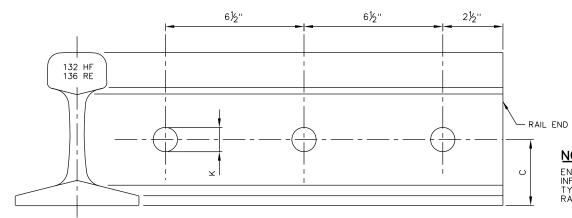
FACING POINT TURNOUT ARRANGEMENT AND SPACING

ENGINEERING STANDARDS

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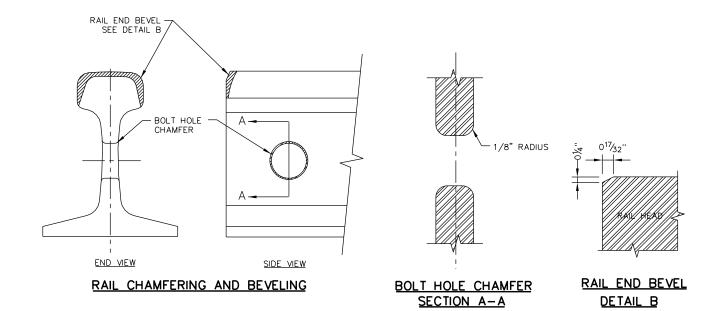
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0 -	RAIL SECTION	OF RAIL	OF BASE	OF HEAD	THICK-	AT CENTER	AT CORNER	FISHING DEPTH	OF BASE	THICKN AT ED OF BA	HEAD ANGLE	BASE ANGLE	OF HEAD	HEAD RADIUS	OF HEAD RADIUS	CORI	NER IUS	& WEB FILLET	0. DACE	WEB RADIUS	© OF WEB RADIUS	NEUT. AXIS	€ OF BOLT HOLES	BOLT HOLES
S Z		INCHES H	INCHES B	INCHES HD	INCHES W	INCHES D	INCHES D'	INCHES F	INCHES E	INCHES T	INCHES A'	INCHES A <sup>2</sup>	INCHES S	INCHES R"	INCHES CH	INCH R	HES R1	INCHES R²	INCHES R <sup>3</sup>	INCHES R*	INCHES L	INCHES N	INCHES C	INCHES K
≥	132-LB. HEAD FREE	7 <b>¾</b> 6	6	2 31/32	21/32	1 <sup>15</sup> <b>/</b> 6	25/32	4 ¾6	1 ¾6	7/6	60 <b>%</b> .	1: 4	1: 40	14	1 <sup>13</sup> / <sub>6</sub>	1	¾6	K	3/4	10-TOP 23-BOT.	4 1/4	3.30	3 ¾ <sub>32</sub>	1 5/16
	136-LB. RE	7 5/16	6	2 <sup>15</sup> / <sub>16</sub>	11/16	1 15/16	1 %	4 3/6	1 3/6	7/6	1: 4	1:4	1: 40	14	1 13/32	1 1/4	%6	5/ <sub>6</sub> & 3/ <sub>4</sub>	3/4	8-TOP 20-BOT	3 7/8	3.35	3 <b>¾</b> 32	1 5/16

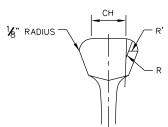
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П		POUNDS	TRACK		CO IN	%	%	%	INCHES*	INCHES <sup>3</sup>	INCHES <sup>3</sup>	용로인	있 유 다	
1 0		POUNDS			SQ. IN.	76	76	76	INCHES	INCHES	INCHES			
2	132-LB. HEAD FREE	132.40	233.02	4.29	12.98	35.8	26.7	37.5	93.80	23.38	28.42	7.23	1.80	
۵	136-LB. RE	136.17 239.66 4.17 13.35 36.4 27.1		36.5	94.90	23.93	28.35	7.11	1.79					



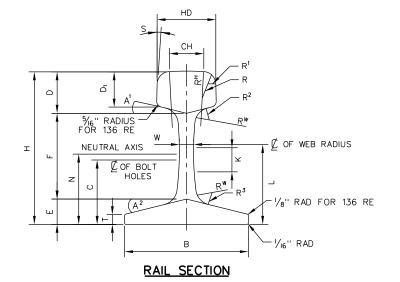
END OF RAIL DRILLING SHOWN FOR INFORMATION ONLY. END HOLE IS NOT TYPICALLY DRILLED TO FACILITATE

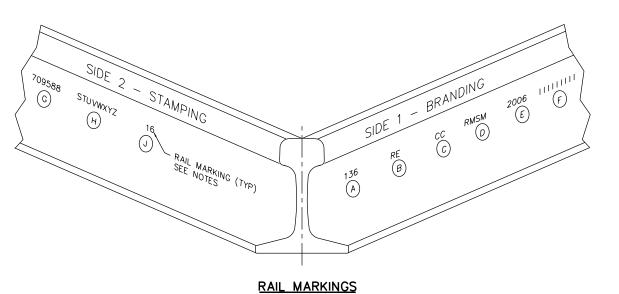
#### RAIL DRILLING FOR JOINTS











#### RAIL MARKING NOTES

BRANDING SHALL BE ROLLED IN RAISED CHARACTERS ON THE SIDE OF THE WEB OF EACH RAIL IN ACCORDANCE TO

- A WEIGHT OF RAIL
  B SECTION
  C HYDROGEN REDUCTION METHOD (CC CONTROL COOLED, VT VACUUM TREATED)
  D MANUFACTURER (EG, RMSM ROCKY MOUNTAIN STEEL MILLS)
  E YEAR ROLLED
  F MONTH ROLLED

- SIDE 2: THE WEB OF OPPOSITE SIDE OF THE RAIL SHALL BE HOT STAMPED IN ACCORDANCE TO AREMA.

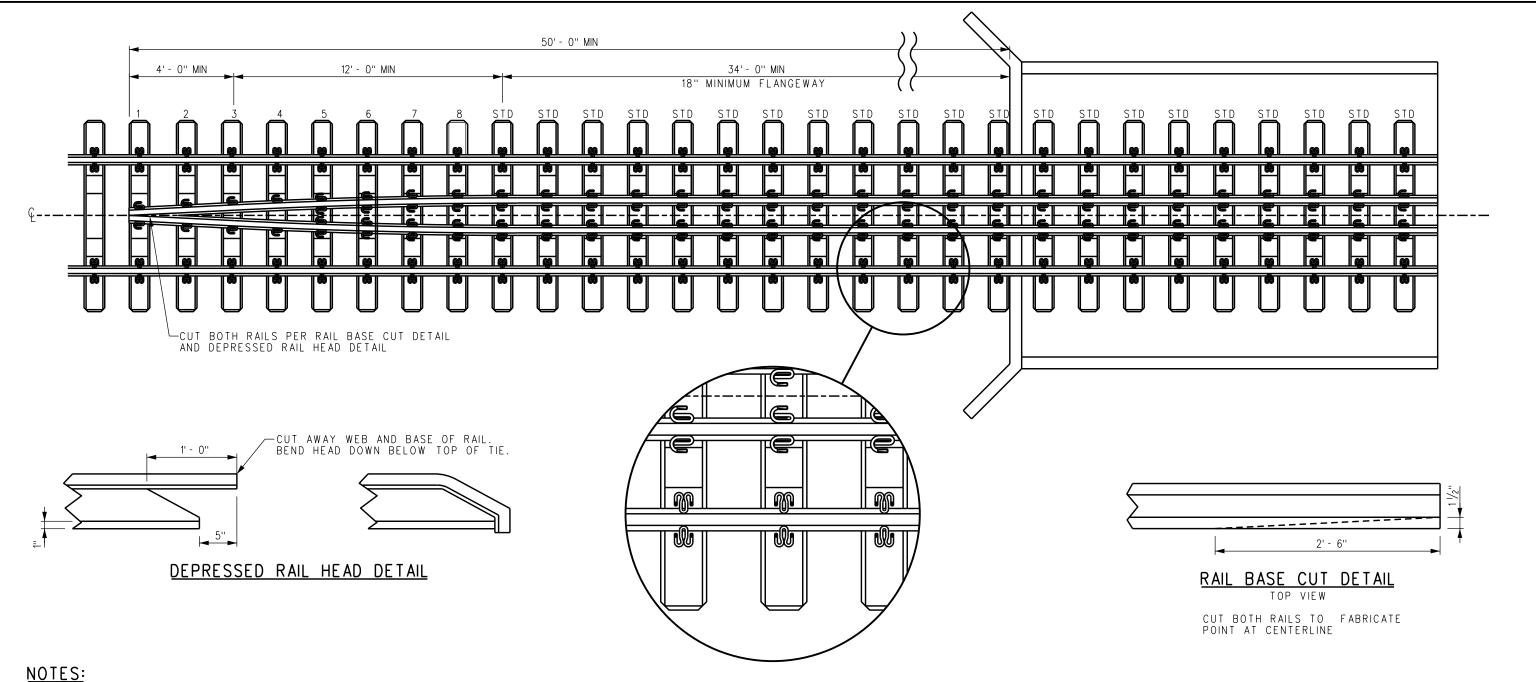
  - G HEAT NUMBER H RAIL LETTER J INGOT NUMBER OR STRAND AND BLOOM NUMBER (IF APPLICABLE)

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SOUTHERN CAI	LIFORNIA RE	GIONAL RAI	L AUTHORITY
ONE GATEWAY	PLAZA, 12TH	FLOOR, L.	A., CA. 90012

ENGINEERING STANDARDS	standard 2301
DATA FOR STANDARD RAIL SECTIONS	SCALE:  NTS  REVISION SHEET  1 OF 1  CADD FILE:
	FS2301



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- 1. INNER GUARD RAILS ON BRIDGES SHALL BE REQUIRED FOR ALL SPANS WHERE EXPOSED STRUCTURAL STEEL IS PRESENT ABOVE T/R AND IS SUBJECTED TO STRUCTURAL DAMAGE BY DERAILED EQUIPMENT. INNER GUARD RAILS SHALL BE INSTALLED ON BRIDGES WHERE INDIVIDUAL SPANS ARE OVER 100 FEET IN LENGTH OR WHERE THE ENTIRE STRUCTURE IS OVER 800 FEET IN LENGTH AND AT LEAST ONE SPAN CROSSES OVER A WATERWAY THAT NORMALLY CONTAINS WATER AT LEAST 15 FEET DEEP. INNER GUARD RAILS SHALL EXTEND 50 FEET BEYOND THE SPAN OR SPANS TO BE PROTECTED.
- 2. INNER GUARD RAILS SHALL BE INSTALLED ON ANY OTHER BRIDGE AS DIRECTED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- 3. INSIDE GUARDRAILS ARE NOT REQUIRED ON BRIDGES UNTIL BRIDGE OR BRIDGE DECK IS REPLACED OR RUNNING RAIL IS REPLACED ACROSS BRIDGE UNLESS DIRECTED BY SCRRA DIRECTOR OF ENGINEERING.

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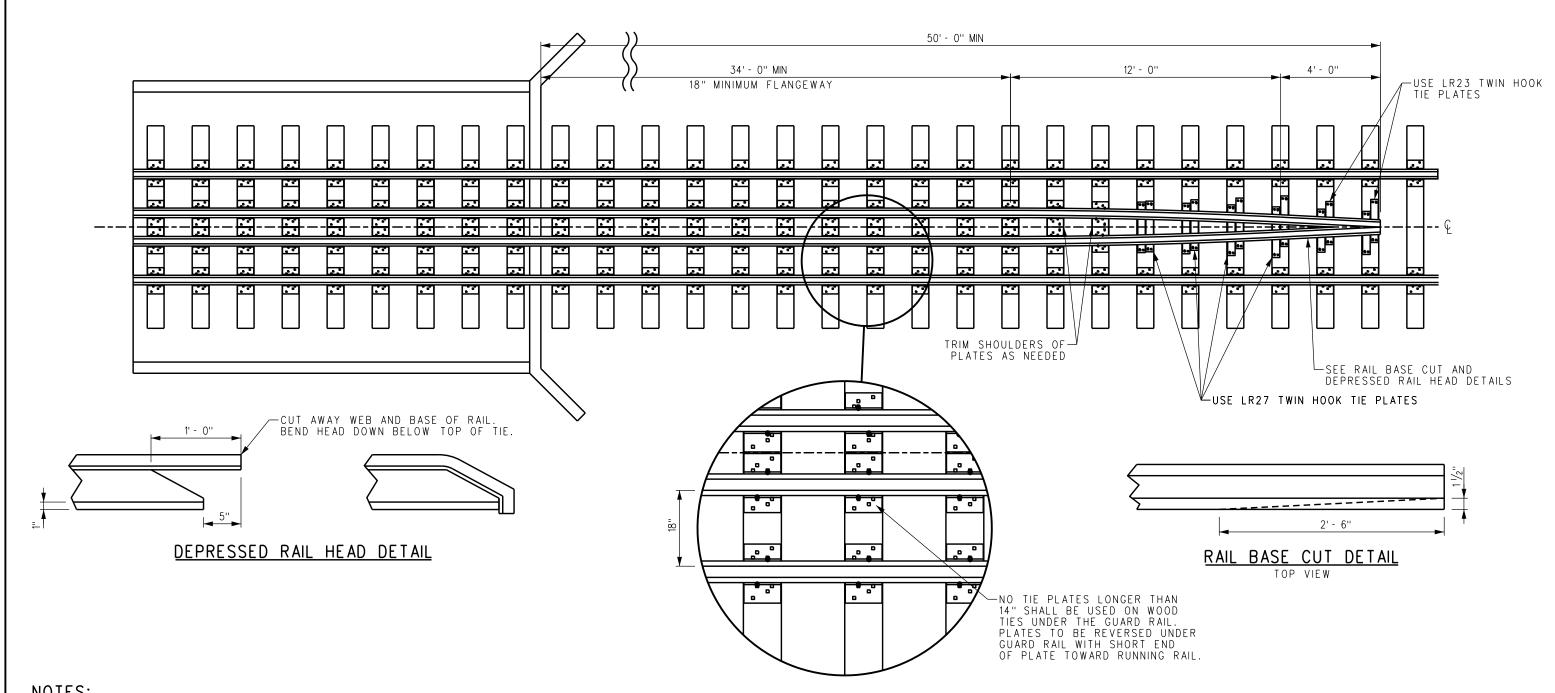
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- 4. INSIDE GUARD RAILS MAY BE CONSTRUCTED USING SECOND HAND RAIL NOT LESS THAN 23 LBS LIGHTER OR NO LARGER THAN RUNNING RAILS. IF GUARD RAIL HAS 5½" BASE, USE MODIFIED PLATES FOR 5½" BASE SCRRA ES2371.

  5. ON CONCRETE TIES, GUARD RAILS SHALL BE FASTENED TO
- EACH TIE.
- GUARD RAIL JOINTS, IF PRESENT, SHALL BE FULLY BOLTED USING SECOND-HAND JOINT BARS.
  THE QUANTITY OF STD PLATES ON CONCRETE TIES WILL
- VARY DEPENDING ON THE NUMBER OF TIES. THEY ARE TO BE ORDERED AS NEEDED. PLATES 1 THROUGH 8 COME AS TWO SETS AND ARE TO BE ROTATED 180° ON OPPOSITE

REFERENCE DRAWINGS: FOR PLATES SEE SCRRA ES2371
FOR CONCRETE TIE SEE SCRRA ES2406 OR ES2407
FOR SCREW AND WASHER SEE SCRRA ES2356

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- 1. INNER GUARD RAILS ON BRIDGES SHALL BE REQUIRED FOR ALL SPANS WHERE EXPOSED STRUCTURAL STEEL IS PRESENT ABOVE T/R AND IS SUBJECTED TO STRUCTURAL DAMAGE BY DERAILED EQUIPMENT. INNER GUARD RAILS SHALL BE INSTALLED ON BRIDGES WHERE INDIVIDUAL SPANS ARE OVER 100 FEET IN LENGTH OR WHERE THE ENTIRE STRUCTURE IS OVER 800 FEET IN LENGTH AND AT LEAST ONE SPAN CROSSES OVER A WATERWAY THAT NORMALLY CONTAINS WATER AT LEAST 15 FEET DEEP. INNER GUARD RAILS SHALL EXTEND 50 FEET BEYOND THE SPAN OR SPANS TO BE PROTECTED.
- INNER GUARD RAILS SHALL BE INSTALLED ON ANY OTHER BRIDGE AS DIRECTED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- 3. INSIDE GUARDRAILS ARE NOT REQUIRED ON BRIDGES UNTIL BRIDGE OR BRIDGE DECK IS REPLACED OR RUNNING RAIL IS REPLACED ACROSS BRIDGE

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- 4. INSIDE GUARD RAILS MAY BE CONSTRUCTED USING SECOND HAND RAIL NOT LESS THAN 23 LBS LIGHTER OR NO LARGER THAN RUNNING RAILS. IF GUARD RAIL HAS 5½" BASE, USE MODIFIED PLATES FOR 5½" BASE SCRRA ES2371.

  5. ON WOOD TIES, GUARD RAILS SHALL BE FULLY PLATED
- AND SPIKED.
- GUARD RAIL JOINTS, IF PRESENTS, SHALL BE FULLY BOLTED USING SECOND-HAND JOINT BARS.

  ON TANGENT TRACK, SPIKE THE INSIDE GUARD RAIL WITH
- TWO SPIKES PER PLATE ON EACH RAIL OF THE TANGENT PORTION AND THREE SPIKES ON EACH RAIL OF THE CURVED PORTION. ON CURVED TRACK, SPIKE THE ENTIRE GUARD
- RAIL WITH THREE SPIKES PER PLATE ON EACH RAIL.
  ON WOOD TIES, BOX ANCHOR TWO TIES NEAR THE CENTER OF BRIDGE TO RESTRICT LONGITUDINAL MOVEMENT OF GUARD RAIL.

REFERENCE DRAWINGS: FOR PLATES SEE SCRRA ES2371 FOR SCREW SPIKE SEE SCRRA ES2355

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#### ANCHOR NOTES:

1.	MATERIAL
2.	MATERIAL
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RAIL BASE SIZE

6"

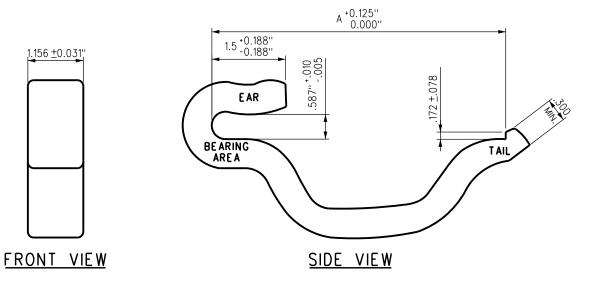
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5.625"

6.125"

- FOR RAIL ANCHOR TO BE HIGH CARBON STEEL.
  FOR RAIL ANCHOR TO BE HEAT TREATED TO RC 34-47,
- TARGET RANGE RC 39-44.
- 3. ALL DIMENSIONS ARE MINIMUM UNLESS OTHERWISE SPECIFIED. 4. TYPICAL CHEMISTRY, CARBON .58-.90, MANGANESE .7-1.1, SILICON .5 MAXIMUM.
- 5. RAIL ANCHORS SHALL CONFORM TO AREMA MANUAL, CHAPTER 5, PART 7, SECTION 7.1.



RAIL ANCHOR

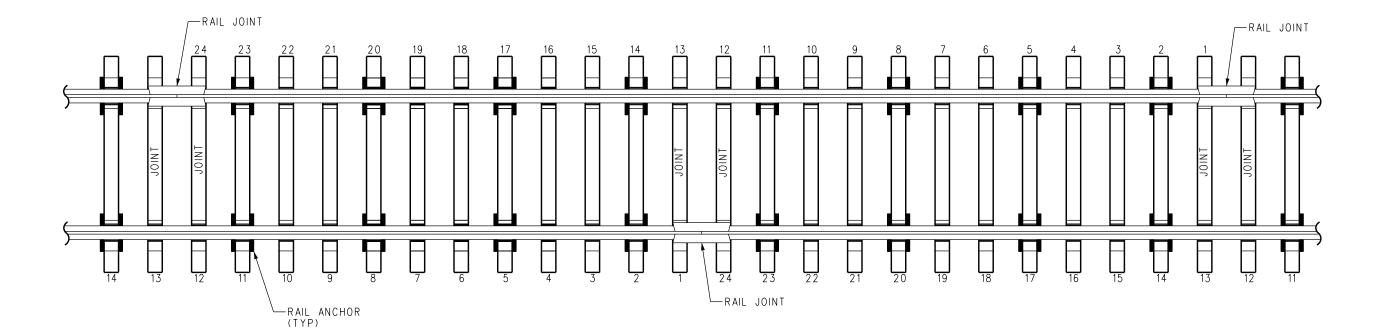
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#### NOTES:

- RAIL ANCHORS SHALL NOT BE PLACED AGAINST JOINT TIES, INCLUDING INSULATED JOINTS.
   WHILE THE NUMBER OF ANCHORS REQUIRED MAY VARY WITH LOCAL CONDITIONS, STANDARD IS 16 ANCHORS PER RAIL LENGTH OF 39 FT OR 24 TIES.

- 3. AT LOCATIONS WHERE ADDITIONAL ANCHORS ARE REQUIRED, SCRRA ENGINEER
  WILL DETERMINE THE NUMBER OF ANCHORS REQUIRED.

  4. RAIL ANCHOR SHALL BE DRIVEN ON BASE OF RAIL UNTIL LOCKING NOTCH ENGAGES
  EDGE OF OPPOSITE FLANGE. ANCHORS MUST NOT BE DRIVEN ALONG THE RAIL. IF
  ADJUSTMENTS ARE NECESSARY, REMOVE AND RE-APPLY.
- 5. FOR CONTINUOUS WELDED RAIL, APPLICATION OF ANCHORS SHALL BE IN ACCORDANCE WITH SCRRA ES2351-02.
- TURNOUTS THAT ARE NOT FASTENED WITH ELASTIC CLIPS ARE TO BE FULLY BOX ANCHORED EXCEPT AT JOINTS OR LOCATIONS WHERE ANCHOR WILL INTERFERE WITH SWITCH OPERATION.
- 7. ELASTIC FASTENERS WILL SATISFY RAIL ANCHORAGE NEEDS. USE OF ANCHORS IN COMBINATION WITH ELASTIC FASTENERS SHALL BE DONE ONLY AS DIRECTED BY SCRRA ENGINEER.
- 8. FOR JOINTED RAIL IN LENGTHS IN EXCESS OF 39 FT, CONTINUE THE PATTERN OF BOX ANCHORS APPLIED TO EACH RAIL ON EVERY THIRD TIE, SKIPPING AND ADJUSTING FOR JOINT TIES.
- 9. ANCHOR PATTERN IS EVERY THIRD TIE TO BE BOX ANCHORED SKIPPING TIES WHERE JOINT BAR IS PRESENT. TIES NUMBER 2, 11, 14, AND 23 MAY BE IMPACTED BY JOINT BAR. IF THIS OCCURS, ANCHOR ADJACENT TIE INSTEAD TO MAINTAIN 8 BOX ANCHORED TIES
- 10. EPOXY BONDED INSULATED JOINTS ARE TO BE CONSIDERED AS CONTINUOUS LENGTHS OF RAIL AND NOT AS "JOINTS" FOR THE PURPOSES OF SELECTING ANCHOR PATTERNS.



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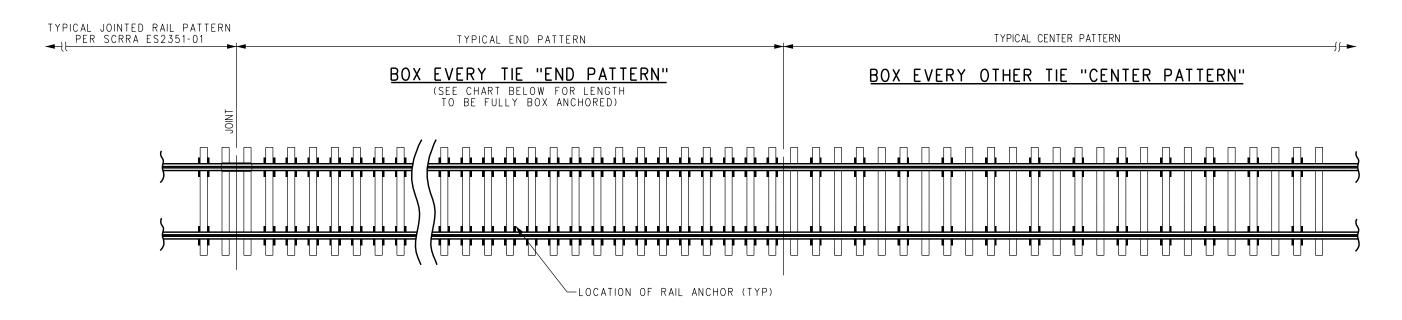
16 ANCHORS PER 39 FT RAIL

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- 1. END PATTERN IS TO BE APPLIED TO BOTH RAILS WHEN JOINT IS ON ONLY ONE RAIL.
- 2. FOR JOINTED RAIL, APPLICATION OF ANCHORS SHALL BE IN ACCORDANCE WITH DRAWING ES2351-01.
- BOX ANCHOR EVERY TIE FOR A DISTANCE OF 200 FT AHEAD OF AND BEHIND TURNOUTS ON MAIN TRACK AND TO THE CLEARANCE POINT ON SIDE TRACK OF TURNOUT FOR ALL SWITCHES IN CWR TERRITORY. ALSO BOX ANCHOR EVERY TIE AS ABOVE FOR RAILROAD DIAMOND CROSSINGS
- 4. FULLY BOX ANCHOR HOT BOX OR DRAGGING EQUIPMENT DETECTORS FOR 200 FT IN EACH DIRECTION.
- 5. EPOXY BONDED INSULATED JOINTS DO NOT REQUIRE END PATTERNS.
  6. RAIL ANCHORS MUST NOT BE PLACED AGAINST JOINT TIES, INCLUDING INSULATED JOINTS.
  7. AT LOCATIONS WHERE ADDITIONAL ANCHORS ARE REQUIRED, SCRRA ENGINEER WILL DETERMINE THE NUMBER OF ANCHORS REQUIRED.
- RAIL ANCHOR SHALL BE DRIVEN ON BASE OF RAIL UNTIL LOCKING NOTCH ENGAGES EDGE OF OPPOSITE FLANGE, ANCHORS MUST NOT BE DRIVEN ALONG THE RAIL. IF
- ADJUSTMENTS ARE NECESSARY, REMOVE AND RE-APPLY.

  ELASTIC FASTENERS WILL SATISFY RAIL ANCHORAGE NEEDS. USE OF ANCHORS IN COMBINATION WITH ELASTIC FASTENERS SHALL BE DONE ONLY AS DIRECTED BY SCRRA ENGINEER.
- 10. IF FIELD WELD INTERFERES WITH TYPICAL END PATTERN, ANCHOR MAY BE OMMITTED.

  DO NOT APPLY ANCHOR TO SAME SIDE OF TIE ON OPPOSITE RAIL, AS ANCHOR
  PATTERN MUST BE A MIRROR PATTERN TO AVOID SKEWING TIES.
- 11. APPLIES TO ALL TRACKS-ML, SIDING, AND YARD WITH CONTINUOUS WELDED RAIL.



	END PATTERN ANCHORING	REQUIRED AT EACH END OF CWR					
LENGTH OF CWR	MIN DISTANCE OF END PATTERN (FT)	EQUIVALENT NUMBER OF TIES					
1000' OR MORE	200	120					
800' TO 1000'	150	96					
550' TO 800'	120	72					
400' TO 800'	80	48					
200' TO 400'	4 0	2 4					

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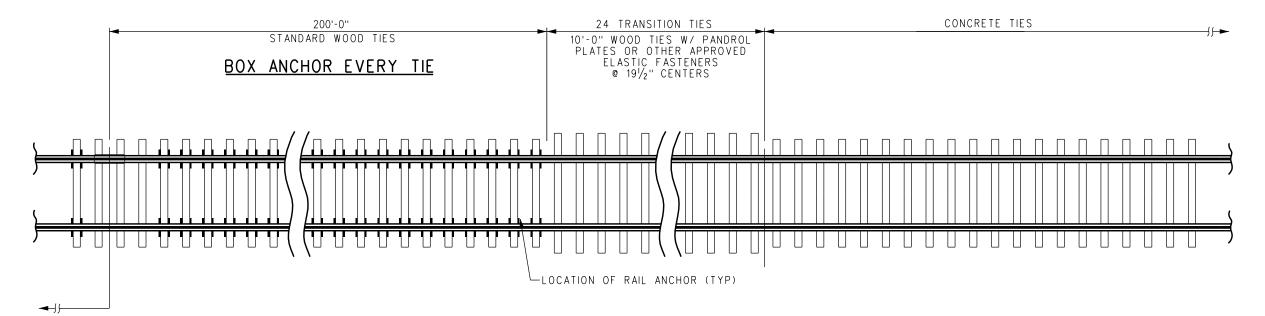
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RAIL ANCHOR APPLICATIONS FOR CONTINUOUS WELDED RAIL WITH WOOD CROSS TIES

NTS 2 OF 4 ES2351-02

- 1. TRANSITION TIES TO CONSIST OF 24, 10'-0" LONG, WOOD TIES WITH PANDROL TYPE, GALVANIZED CLIPS OR EQUAL.
  2. BOX ANCHORS ARE REQUIRED FOR 200 FT IN THE WOOD TIES AFTER
- 2. BOX ANCHORS ARE REQUIRED FOR 200 FT IN THE WOOD TIES AFTER TRANSITION TIES. ANCHORS ON BOTH RAILS MAY BE OMITTED IF RAIL JOINTS ARE PRESENT.
- 3. RAIL ANCHORS MUST NOT BE PLACED AGAINST JOINT TIES, INCLUDING INSULATED JOINTS.
  GLUE LAMINATED INSULATED JOINTS ARE NOT CONSIDERED AS JOINTS AND WILL BE FULLY ANCHORED.
- 4. AT LOCATIONS WHERE ADDITIONAL ANCHORS ARE REQUIRED, SCRRA ENGINEER WILL DETERMINE THE NUMBER OF ANCHORS REQUIRED.
- RAIL ANCHOR SHALL BE DRIVEN ON BASE OF RAIL UNTIL LOCKING NOTCH ENGAGES EDGE OF OPPOSITE FLANGE. ANCHORS MUST NOT BE DRIVEN ALONG THE RAIL. IF ADJUSTMENTS ARE NECESSARY, REMOVE AND RE-APPLY.
- 6. ELASTIC FASTENERS IN WOOD TIE ZONE WILL SATISFY RAIL ANCHORAGE NEEDS. USE OF ANCHORS IN COMBINATION WITH ELASTIC FASTENERS SHALL BE DONE ONLY AS DIRECTED BY SCRRA ENGINEER.
- 7. APPLIES TO ALL CONCRETE TIE TRACKS-ML, SIDING, AND YARD.



STANDARD RAIL ANCHOR PATTERN PER SCRRA ES2351-02 IF CWR, AND SCRRA ES2351-01 IF JOINTED RAIL

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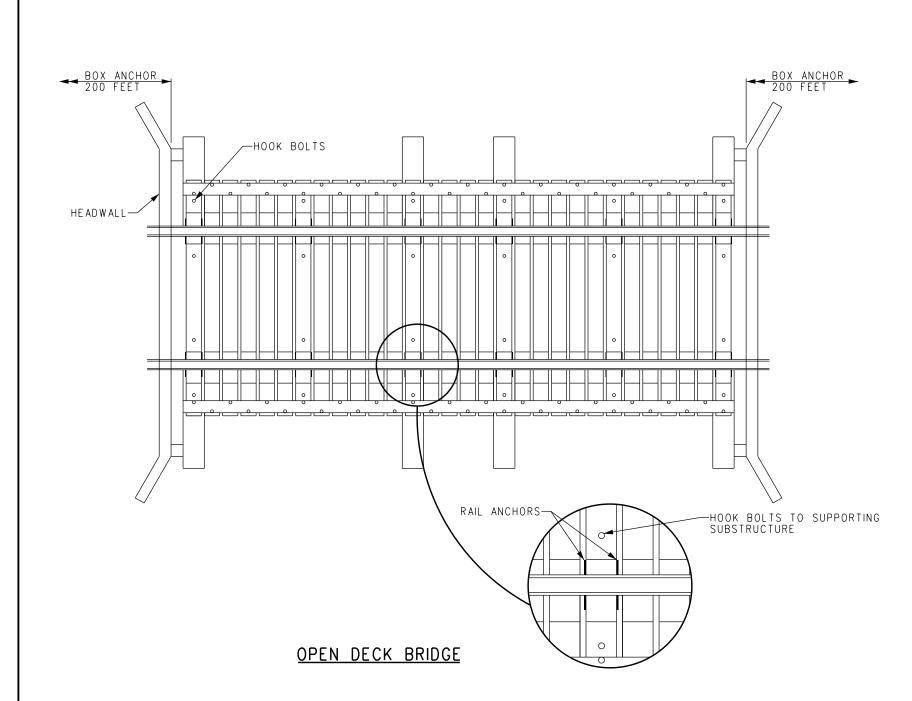
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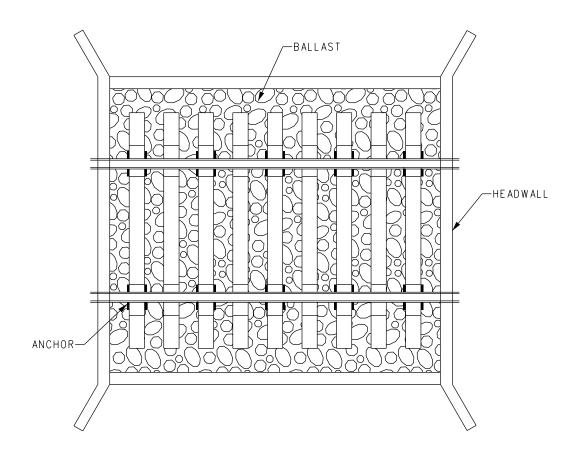


SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

RAIL ANCHOR APPLICATIONS
FOR CONTINUOUS WELDED RAIL – TRANSITION
FROM WOOD TO CONCRETE CROSS TIES

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# BALLAST DECK BRIDGE

FOR ANCHORING RAIL ON BALLAST DECK BRIDGES, BRIDGE HAS NO IMPACT ON PATTERN, USE PATTERN REVISIONS IN ES2351-02.

# OPEN DECK BRIDGES:

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- 1. BOX ANCHOR EVERY TIE FOR 200 FEET AWAY FROM HEADWALL ON ALL OPEN DECK BRIDGE APPROACHES. USE ANCHOR PATTERN ON SCRRA ES2351-02.
- 2. ALL TIES ACROSS OPEN DECK BRIDGES WHICH ARE ANCHORED TO SUBSTRUCTURE WILL BE BOX ANCHORED.
- 3. ONLY APPLIES TO BRIDGE 200' OR LONGER.

#### NOTES:

- 1. EXISTING ANCHOR PATTERNS MAY REMAIN UNTIL RAIL RELAY IS COMPLETE.
  2. SECOND HAND ANCHORS MAY BE USED ON ALL INDUSTRY AND YARD TRACKS.
  3. AS A GENERAL RULE, TRACK WITH ELASTIC FASTENERS DOES NOT REQUIRE ANCHORING. HOWEVER, IF THE SCRRA ENGINEER DEEMS IT NECESSARY TO PROPERLY RESTRAIN THE RAIL FROM MOVING LONGITUDINALLY, RAIL ANCHORS SHALL BE INSTALLED.

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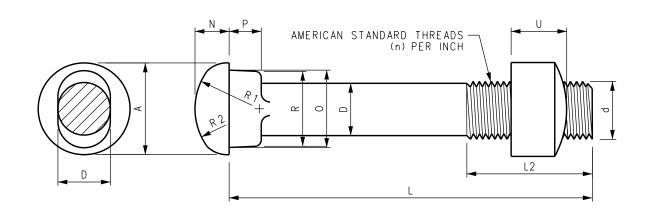


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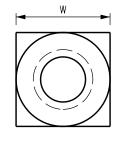
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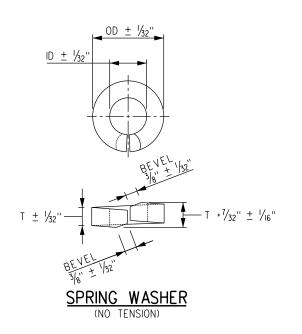
- 1. BOLTS AND NUTS TO BE MADE OF CLASS B STEEL. 2. NOMINAL SIZE OF BOLT IS THE THREAD DIAMETER (d). 3. WASHERS TO CONFORM TO AREMA SPECIFICATIONS.

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WEIGHT AND SECTION OF RAIL	OUTSIDE DIAMETER	LENGTH	NUMBER PER INCH	SHANK DIAMETER	LENGTH UNDER HEAD	DIAMETER	THICKNESS	L ONG R ADIUS	SHORT RADIUS	MAXIMUM WIDTH	MINIMUM WIDTH	ОЕРТН	THICKNESS	WIDTH	WEIGHT EACH (BOLT AND N	NUMBER OF BOLTS PER 2 LB KEG	INSIDE DIAMETER	OUTSIDE DIAMETER	THICKNESS
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80 LB ASCE	13/16	11/2	10	3/4	4 3/8	1 1/16	%6			11/16	11/32	7/16	3/4	13/8	1.09	184	7∕8	13/4	7/16
75 LB CS & CS REV	15/16	1 1/8	_	7/8	4 3/4	19/	117			17/	13/16	17	11/	11/	1 5 6	10.0	11/	21/	%6
80 LB ASCE	7 '916	1 78	9	/8	4 74	1%	11/16			17/32	1 7/16	1/2	11/8	11/2	1.56	128	11/16	21/16	716
90 LB AREA	11	- 11	11	7/8	51/8	11	11			11	11	11	11	- 11	1.62	123	11/16	21/16	%
110 LB RE	11/16	21/8	8	1	5 %	111/16	3/4			111/32	15//6	5/8	11/4	15/8	2.22	90	11/8	21/8	%
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112 LB, 115 LB, 131 LB RE																			
113 LB HF, 132 LB HF	11//8	21/2	7	11/16	61/2	1 <sup>57</sup> ⁄ <sub>64</sub>	45/64	155/64	43/64	117/32	11/2	5/8	11/8	111/16	2.62	76	13/16	2 7/32	%6
119 LB CF&I, 136 LB RE 141 LB RE																			



TRACK BOLT AND NUT





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					/ Varen D. Pall
			T		ASSISIANT DIRECTOR: STANDARDS & DESIGN
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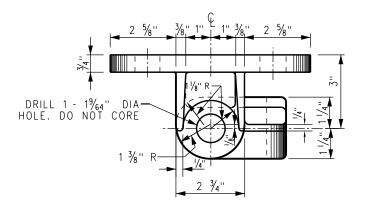
ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

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	SOUTHERN	CALIFORNIA	REGIONAL	RAIL	AUTHORITY

TRACK	BOLTS,	NUTS	AND	WASHER

ENGINEERING STANDARDS

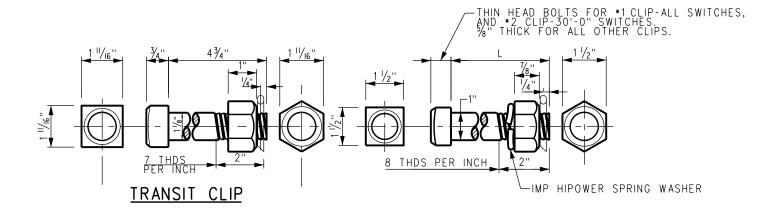
NTS 1 OF 1 ES2352

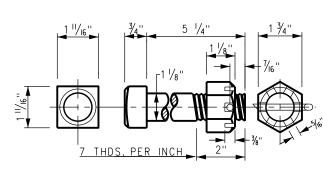


"X"	HEIGHT ABOVE	BASE
WEIGHT OF RAIL	REINFORCING	11/4" REINFORCING
110 LB TO 119 LB	2	2"/16"
130 LB TO 136 LB	2 <sup>15</sup> / <sub>16</sub> ''	2 3/4"

# **SPECIFICATIONS:**

CAST STEEL CLIPS IGHT AND LEFT HAND RIGHT HAND SHOWN



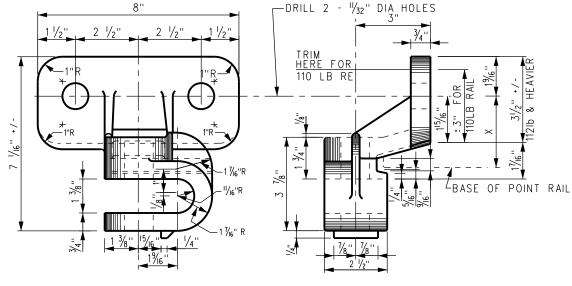


SWITCH LENGTH	ROD NO:	T Y P E CLIP	LENGTH OF BOLTS "L					
LENGIH	NO.	CLIP	SPLIT SWT	SPRING SWT				
	1	TRANSIT	4 ''					
16'-6"	'	OSJ	*4"	4"				
10 - 0	2	TRANSIT	4 ''	4 1/2 "				
	3	OSJ		+ 4 3/4"				
	,	TRANSIT	4 1/2"					
24'-0"	'	OSJ	*4"	4 3/4"				
AND 30'-0"	2-5IN	TRANSIT	4 1/2 "	51/4"				
30 -0	4	OSJ		+51/2"				
*F0R +F0R			ATED SWITC LOCK OPER					

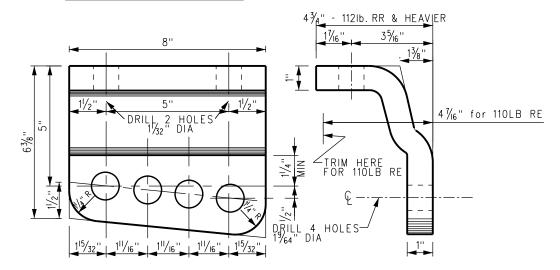
# OPEN SIDE JAW CLIP

BOLTS FOR SWITCH RODS AND CLIPS BOLTS FOR SWITCH POINTS AND CLIPS

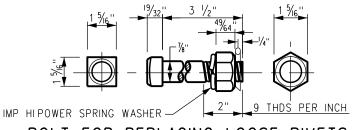
OPEN SIDE JAW & TRANSIT CLIP



#### OPEN SIDE JAW CLIP



RIGHT AND LEFT HAND REQUIRED RIGHT HAND SHOWN



# **BOLT FOR REPLACING LOOSE RIVETS**

# NOTE:

THIS BOLT TO BE USED FOR REPLACING LOOSE RIVETS ON SWITCHES FORMERLY FURNISHED WITH TRANSIT CLIPS RIVETED TO SWITCH POINTS.

NOTES:

# FOR MAINTENANCE ONLY

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					ASSISIANT DIRECTOR: STANDARDS & DESIGN	A
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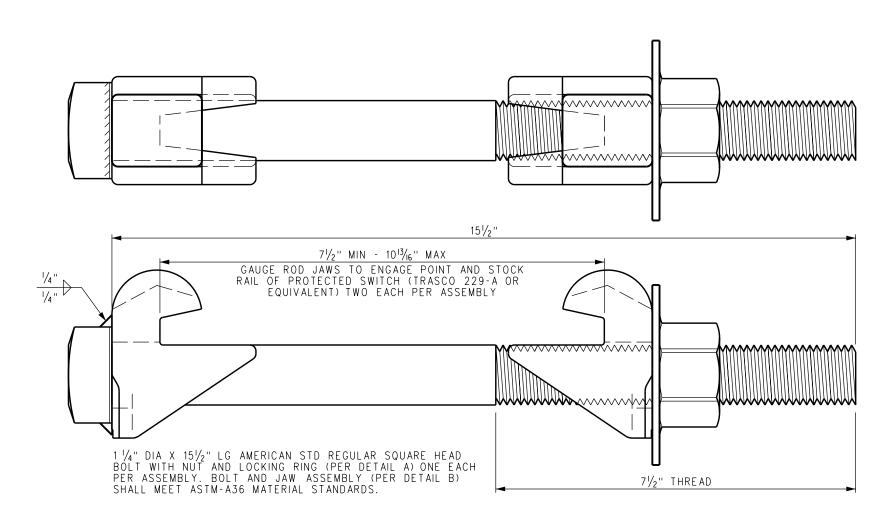
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# METROLINK

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ENGINEERING STANDARDS	STANDARD 2353
	scale: NTS
SWITCH ROD CLIPS AND BOLTS	REVISION SHEET  AC 1 OF 1
	CADD FILE: ES2353

1. ALL BOLTS TO BE TURNED BOLTS WITH CUT THREADS. 2. DRILL  $\%_{32}$ " DIA HOLE FOR  $\frac{1}{4}$ " SPRING COTTERS AS SHOWN. 3. SLOTTED NUT SHOWN TO BE AMERICAN STANDARD HEAVY

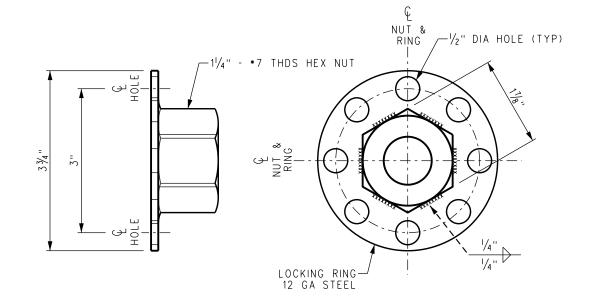


# SWITCH POINT CLAMP

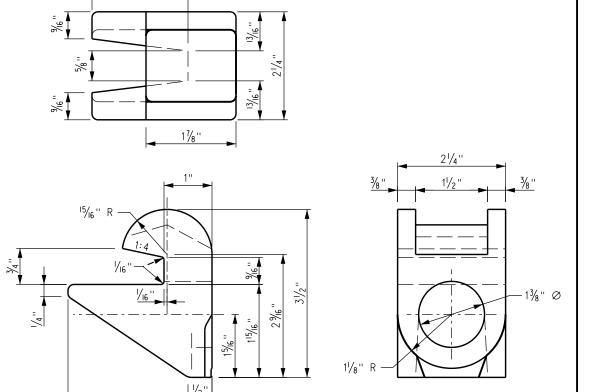
# NOTES:

- 1. TO BE INSTALLED ALONG SWITCH POINT
  BETWEEN POINT OF SWITCH AND NO 2 ROD.
  2. PAINT ASSEMBLY DARK BLUE-EXCEPT THREADS.
  USE SCRRA MAINTENANCE PADLOCK.

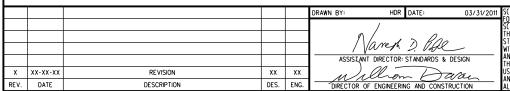
  WITH A PROPERTY OF THE PROPERTY OF THE PARTY OF
- 3. WHEN CLAMP IS APPLIED ON SWITCH WITH HAND THROW SWITCH STAND, STANDARD SWITCH LOCK WILL BE REPLACED WITH SCRRA MAINTENANCE PADLOCK, AND TAGGED "OUT OF SERVICE".



# DETAIL A - NUT AND LOCKING RING



DETAIL B - CLAMP CASTING



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ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS NTS SWITCH POINT CLAMP 1 OF ES2354

# R 21/8 11/4" 6 <sup>11</sup>/<sub>16</sub> " 6" - UNDER HEAD 1<sup>1</sup>/<sub>4</sub>''

TRACK SPIKE

#### NOTES:

- 1. TRACK SPIKES MUST CONFORM TO AREMA SPECIFICATIONS.
- 2. WEIGHT = 0.85 LBS.
- 3. PACKAGE IN 50\* BOX OR 100\* KG.

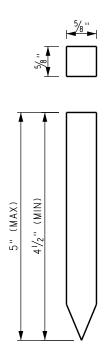
#### TIGHT SPIKE

OR APPROVED EQUAL, SPIKE HOLE FINISHING COMPOUND

#### NOTES:

- 1. CONSISTS OF A 2-PART FILLER MATERIAL-PART A
- (ISOCYANATE) AND PART B (POLYOL).
  2. OPERATORS MUST BE PROPERLY TRAINED AND USE APPROPRIATE EQUIPMENT FOR INSTALLATION OF MATERIAL.

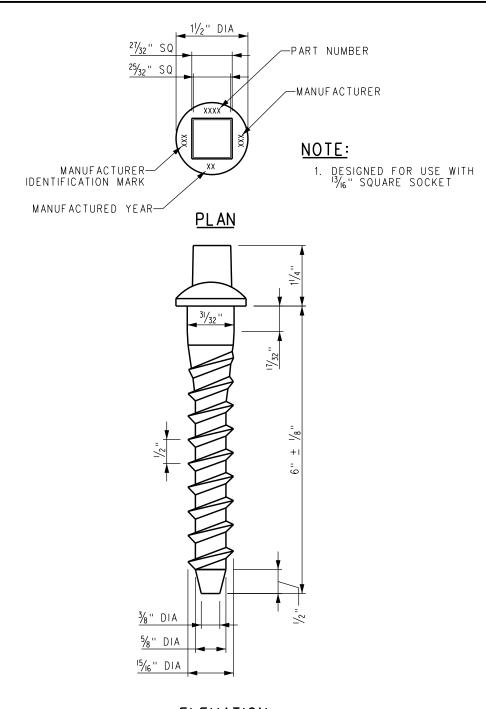
  3. MATERIAL IS SUPPLIED IN SEVERAL SIZES INCLUDING
- SMALL CAULK GUN TUBES FOR MINOR INSTALLATION REQUIREMENTS.
- 4. MUST ADHERE TO REQUIREMENTS OF MATERIAL SAFETY DATA SHEET WHEN HANDLING MATERIAL.



TIE PLUG

# NOTES:

- 1. TIE PLUG TO CONFORM TO AREMA STANDARDS.
- 2. TIE PLUG TO BE FABRICATED FROM HARDWOOD TREATED WITH CREOSOTE, CCA, OR BORATE.
- MAY BE BUNDLED OR BAGGED.
- PLUG MUST BE FULLY INSERTED INTO EMPTY SPIKE HOLE AND TAMPED INTO PLACE. REMOVE EXCESS PLUG WITH ADZE.



**ELEVATION** SCREW SPIKE

## MATERIAL SPECIFICATIONS:

- ALL SCREW SPIKES TO BE HOT FORGED. SCREW SPIKES TO BE MADE FROM MEDIUM CARBON STEEL TO MEET ASTM A-66 SPECIFICATIONS.
- SCREW SPIKES TO BE COATED TO RESIST CORROSION.
- APPROXIMATE SHIPPING WEIGHT OF EACH SCREW
- 5. SCREW SPIKES TO BE PACKED 100 TO A BAG.

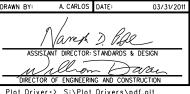
#### INSTALLATION INSTRUCTIONS:

- PRE-DRILL WOOD TIES WITH 1/16" DIA DRILL BIT
- TO DEPTH OF 5 1/2".

  2. PRE-DRILLED HOLES MUST BE PERPENDICULAR
- WITH BASE PLATE.

  3. USING A ¾" SQUARE DRIVE SOCKET AND AN IMPACT WRENCH, SCREW IN UNTIL SNUG.

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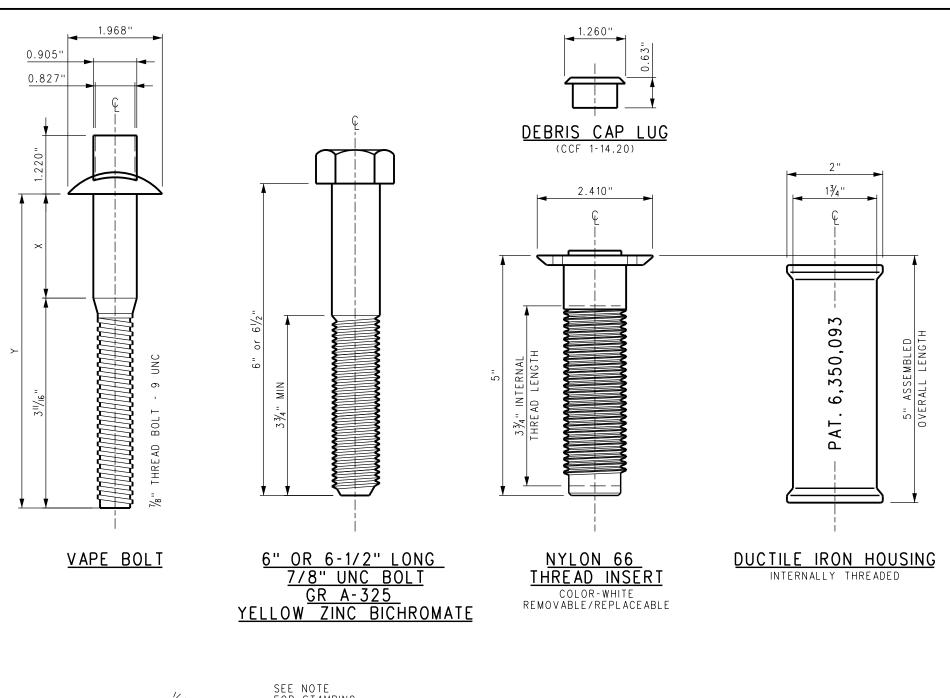


SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA. 12TH FLOOR. L. A., CA. 90012

6" TRACK SPIKES, 15/46" SCREW FASTENER, TIE PLUGS AND TIGHT SPIKE FILLER

ENGINEERING STANDARDS

NTS 1 OF FS2355

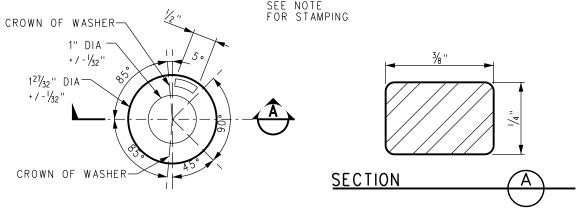


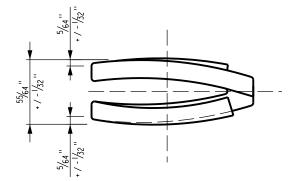
VAPE BOLT DIMENSIONS

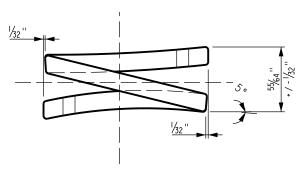
X GRIP LENGTH	Y SHANK LENGTH	FOR USE THROUGH
2 3/16 ''	5 1/8"	¾" PLATING
25%"	63/8"	1" PLATING

# NOTES:

- 1. 6" OR 6-1/2" LONG 7/8" UNC BOLT
  GR A-325 YELLOW ZINC BICHROMATE.
  2. TO AVOID DAMAGE TO THE TIE, ENSURE THAT PROPER SCREW SIZE
  IS USED FOR VARIOUS PLATE THICKNESSES. (SEE TABLE)
- 3. VAPE SCREW TO BE TORQUED TO 150 FT-LBS. THIS TORQUE CORRESPONDS TO A 1mm CLEARANCE BETWEEN COILS ON THE SPRING WASHER.
- 4. FOR CONCRETE GUARD RAIL TIE SEE SCRRA ES2406 OR ES2407.
  FOR CONCRETE SWITCH TIE SEE CORRESPONDING TIE PLAN.
  5. SPRING WASHERS SHALL CONFORM TO UIC CODE 864-3. DIMENSIONS AS DELIVERED (UNLOADED). STAMPING IS TO BE DONE IN AREA INDICATED ON CURRENT YEAR AND SUPPLIER'S LOGO.







HELICAL SPRING WASHER

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**METROLINK** 

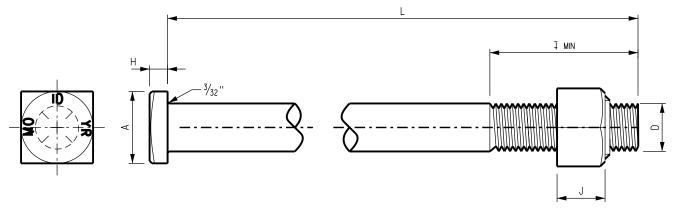
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PIM 532 SCREW, INSERT AND HELICAL WASHER FOR CONCRETE TIES

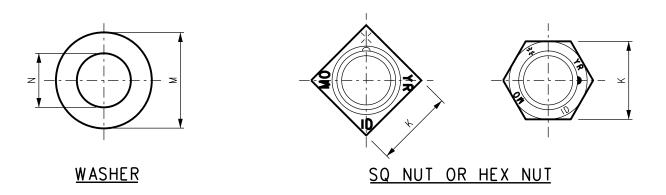
ENGINEERING STANDARDS

NTS 1 OF

				LTS, NUTS, AND				
			TA	BLE OF DIMENS	IONS			
D	L	A	Н	I	J	K	M	N
1"	3 1/2"	1 1/2"	3/8''	2"	-	-	-	-
П	4 ''	П	н	п	-	-	-	-
П	4 1/2"	П	н	п	-	-	-	-
п	5"	п	П	2 1/2"	-	-	-	-
П	5 ½"	п	П	п	-	-	-	-
п	6"	п	П	п	-	-	-	-
п	6 ½"	11	ш	"	-	-	-	-
п	7"	11	II	"	-	-	-	-
1"	3 1/2"	1 1/2"	21/32 ''	"	-	-	-	-
11	4 ''	11	П	"	-	-	-	-
11	4 1/2"	11	н	11	-	-	-	-
11	5"	11	11	"	-	-	-	-
п	5 ½"	П	н	п	-	-	-	-
п	6"	п	П	п	-	-	-	-
11	6 1/2"	п	П	0	-	-	-	-
п	7"	п	П	п	-	-	-	-
WASHER							2"	1 1/8"
SQ NUT					1"	1 5/8"		, ,
HEX NUT					1"	1 5/8"		
1 3/8"	4''	2 1/16"	29/32''	2 3/4"	-	-	-	-
11	4 1/2"	11	11	- / -	-	-	-	_
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	23"	"	"		-	-	-	-
"	24"	п	П	п	-	-	- 3/	- 17
WASHER					7 /	. 7,	2 3/4"	1 1/2"
SQ NUT					1 3/8" 1 3/8"	2 <sup>3</sup> / <sub>16</sub> " 2 <sup>3</sup> / <sub>16</sub> "		
HEX NUT					■ 1 ½"	1 2 1/6"		



# SQUARE HEAD FROG BOLTS



#### NOTES:

- 1. BOLT SHALL CONFORM TO THE CURRENT VERSION OF SAE J429 GRADE 8. HEAT TREATED TO 150,000 PSI TENSILE STRENGTH, 130,000 PSI YIELD, OIL QUENCHED FROM 4140 STEEL. HEAD MARKINGS SHALL INCLUDE GRADE 8 GRADE MARKINGS, MANUFACTURER ID, MONTH AND YEAR OF MANUFACTURE. THREADS TO BE ROLLED AND CONFORM TO ANSI/ASME B1.1 UNC-2A THREAD FORM.

  2. NUTS SHALL CONFORM TO HARDNESS AND MATERIAL REQUIREMENTS OF SAE J995 GRADE 8 OR ASTM A-563 GRADE 'DH', AND DIMENSIONAL REQUIREMENTS OF ANSI/ASME B18.2.2 HEAVY HEX OR SQUARE NUTS, PLAIN FINISH. HEAVY HEX OR SQUARE OCK NUT THREADS SHALL CONFORM TO ANSI/ASME B1.1 LINC-2B THREADS
- SQUARE LOCK NUT THREADS SHALL CONFORM TO ANSI/ASME B1.1 UNC-2B THREAD
- 3. FLAT WASHERS SHALL CONFORM TO HARDNESS REQUIREMENTS OF THE CURRENT VERSION OF ASTM F-436 AND BE \( \frac{1}{32} \)" THICK.

  4. WORKMANSHIP: BOLTS, NUTS, AND WASHERS SHALL BE FREE FROM BURRS,
- SEAMS, LAPS, AND SCALE.
- 5. BOLT TIGHTENING SEQUENCE SHALL START WITH THE BOLT NEAREST THE CENTER OF FROG. WORK IN A CIRCULAR PATTERN IN A CLOCKWISE DIRECTION, PROGRESSING OUTWARD TO THE NEXT NEAREST FROG BOLT UNTIL ALL BOLTS ARE TIGHTENED TO PROPER TORQUE, OR AS DIRECTED BY MANUFACTURER. ALL BOLTS AND NUTS WILL BE SUPPLIED WITH SELF-CENTERING WASHERS OR EQUIVALENT FOR PROPER LOAD DISTRIBUTION. SEE SCRRA ES2359.

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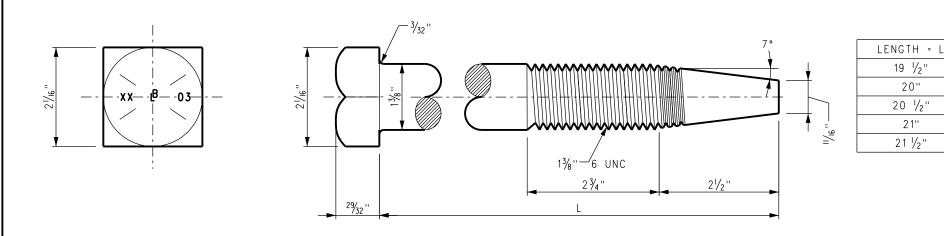


SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS

SQUARE HEAD FROG BOLTS, SQUARE AND HEX NUTS AND HARDENED FLAT WASHERS

NTS 1 OF ES2357

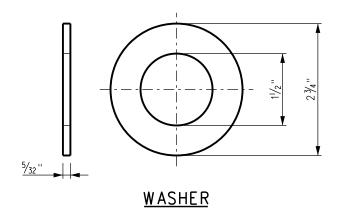


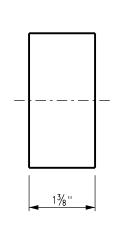
TAPERED FROG BOLTS

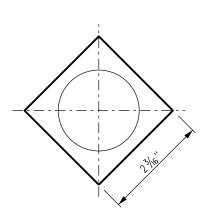
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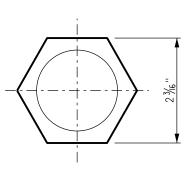
- 1. WHEN USING/ORDERING TAPERED BOLTS, ADD 2½" TO OLD BOLT LENGTH.
  2. BOLT SHALL CONFORM TO THE CURRENT VERSION OF SAE J429 GRADE 8. HEAT TREATED TO 150,000 PSI TENSILE STRENGTH, 130,000 PSI YIELD, OIL QUENCHED FROM 4140 STEEL. HEAD MARKINGS SHALL INCLUDE GRADE 8 GRADE MARKINGS, MANUFACTURER ID, MONTH AND YEAR OF MANUFACTURE. THREADS TO BE ROLLED AND CONFORM TO ANSI/ASME B1.1 UNC-2A THREAD FORM.
- NUTS SHALL CONFORM TO HARDNESS AND MATERIAL REQUIREMENTS OF SAE J995 GRADE 8 OR ASTM A-563 GRADE 'DH' AND DIMENSIONAL REQUIREMENTS OF ANSI/ASME B18.2.2 HEAVY HEX OR SQUARE NUTS, PLAIN FINISH. HEAVY HEX OR SQUARE LOCK NUT THREADS SHALL CONFORM TO ANSI/ASME B1.1 UNC-2B THREAD FORM, FREE FIT.
- 4. FLAT WASHERS SHALL CONFORM TO HARDNESS REQUIREMENTS OF THE CURRENT VERSION OF ASTM F-436 AND BE 5/32" THICK.

  5. WORKMANSHIP: BOLTS, NUTS, AND WASHERS SHALL BE FREE FROM BURRS, SEAMS, LAPS, AND SCALE.









SQ NUT OR HEX NUT

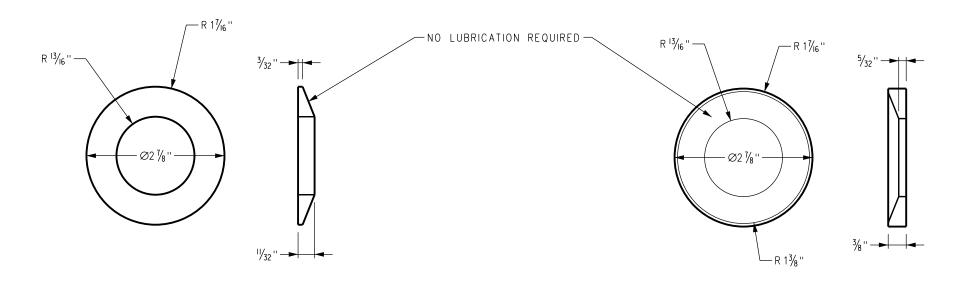
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ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

**ENGINEERING STANDARDS** NTS TAPERED FROG BOLT ASSEMBLY 1 OF ES2358



FROG BOLT OR NUT TOP WASHER -BOTTOM WASHER

SECTION VIEW

PLAN VIEW SECTION VIEW SECTION VIEW

TOP WASHER INSTALL UNDER BOLT HEAD OR NUT

PLAN VIEW

**BOTTOM WASHER** INSTALL AGAINST ITEM BEING BOLTED BELOW TOP WASHER ASSEMBLED WASHER SET

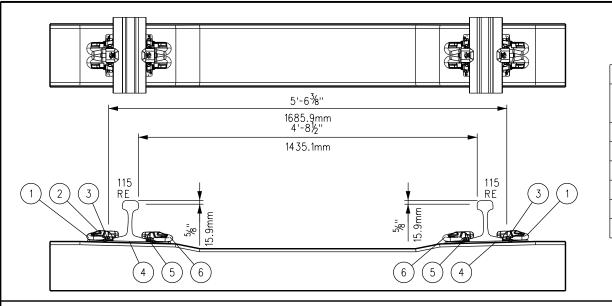
# **INSTALLATION NOTES:**

- INSTALL TOP WASHER UNDER BOLT HEAD OR NUT.
   INSTALL BOTTOM WASHER BELOW TOP WASHER AND AGAINST FROG OR OBJECT BEING CLAMPED.
   USE OF D-WASHER OR BEVELED WASHERS ARE REQUIRED WHEN INSTALLING SPHERICAL (SELF-ALIGNING) WASHERS ON RAIL AND ON FROGS, WHERE APPLICABLE.
   BOLT HEAD LOCKS WILL NOT WORK WITH SPHERICAL WASHERS, AND NEED TO BE REPLACED WITH D-WASHERS, BEVELED WASHERS, OR REMOVED BY GRINDING.

# MANUFACTURING SPECIFICATION:

- 1. SURFACE FINISH COEFFICIENT OF FRICTION SHALL BE 0.05-0.10.
  2. FINISHED SURFACES MUST PROVIDE LONG-TERM (NOT TEMPORAL)
  LUBRICATION EFFECTS UNDER EXTREME PRESSURE:
  150,000 PSI (10,500 KG/CM SQUARED).
  3. FINISHED WASHERS MUST WITHSTAND 1000 HOURS OF ASTM B-117
  SALT FOG TEST, WITH LESS THAN 15% RED RUST.
  4. FINISHED WASHERS SHALL HAVE THE MINIMUM HARDNESS
  APPROPRIATE AND SUFFICIENT FOR USE WITH HIGH STRENGTH FASTENERS.
  5. MINIMUM COMPENSATING ANGLE SHALL BE 10 DEGREES IN ALL DIRECTIONS.
  6. PARTS SHALL BE MARKED WITH MANUFACTURERS IDENTIFYING CHARACTERISTICS.

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L				ASSISTANT DIRECTOR: STANDARDS & DESIGN	THIS INFORMATION AGREES THAT IT ASSUMES ALL LIABILITY ARISING FROM SUCH	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY	SPHERICAL (SELF - CENTERING) WASHER SET	-   1 OF '
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	115 RE RAIL AND 136 LB RAIL CONCRETE TIE						
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY			
1	10218	RAIL CLIP ASSY - FC1603 CLIP / 7695 TOE INSULATOR	BLUE	2			
2	9086	FASTCLIP TWIN-STEM SHOULDER		4			
3	11458	SIDE POST INSULATOR - 0.726" THICK POST	BLUE	2			
4	115 4 9	RAIL PAD		2			
5	11459	SIDE POST INSULATOR - 0.430" THICK POST	GREEN	2			
6	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NEUTRAL	2			

- 1. FOR RAIL PAD DETAILS, SEE SCRRA ES2364. 2. FOR SIDE POST INSULATOR DETAILS,
- SEE SCRRA ES2365.
- SEE SCRRA ES2365.

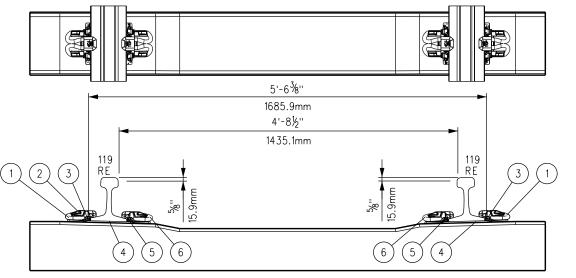
  3. FOR RAIL CLIP DETAILS, SEE SCRRA ES2366.

  4. FOR TOE INSULATORS DETAILS, SEE SCRRA ES2367.

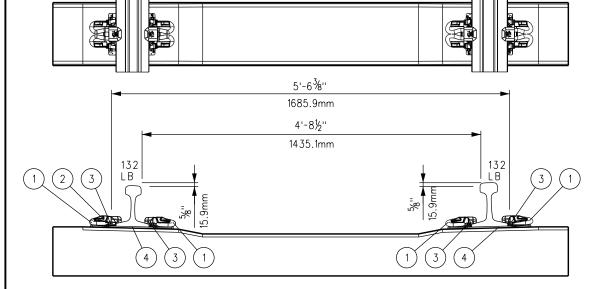
  5. ALL COMPONENTS FOR TIE ASSEMBLIES TO BE PANDROL TYPE OR EQUIVALENT AS APPROVED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- CONSTRUCTION.

  6. ALL PART NUMBERS LISTED ON THIS DRAWING CORRESPOND TO PANDROL BRAND COMPONENTS AND ARE SUBJECT TO CHANGE.

  7. FOR CONCRETE TIE DETAILS AND FRICTION
- PATTERN, SEE SCRRA ES2402.



		119 RE RAIL AND 136 LB RAIL CONCRETE TIE		
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY
1	10218	RAIL CLIP ASSY - FC1603 CLIP / 7695 TOE INSULATOR	BLUE	2
2	9086	FASTCLIP TWIN-STEM SHOULDER		4
3	11458	SIDE POST INSULATOR - 0.726" THICK POST	BLUE	2
4	115 4 9	RAIL PAD		2
5	11459	SIDE POST INSULATOR - 0.430" THICK POST	GREEN	2
6	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NEUTRAL	2



		132 LB RAIL AND 136 LB RAIL CONCRETE TIE		
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY
1	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NEUTRAL	4
2	9086	FASTCLIP TWIN-STEM SHOULDER		4
3	7692	STANDARD SIDE POST INSULATOR - 0.326" THICK POST	NUETRAL	4
4	7083	RAIL PAD ASSEMBLY		2

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					Vareh D. Voll	Ņ
					ASSISTANT DIRECTOR: STANDARDS & DESIGN	A
Α	05-16-16	REVISED RAIL PAD ASSEMBLY NUMBER	AC	NDP	Cate Inteller	Ú
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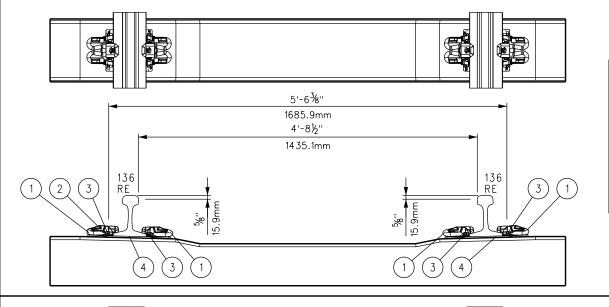
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SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

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PANDROL FASTCLIP CONCRETE TIE ASSEMBLIES
EOD MADIOUS DAIL COMPINATIONS
FOR VARIOUS RAIL COMBINATIONS



	136 RE RAIL AND 136 LB RAIL CONCRETE TIE							
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY				
1	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NUETRAL	4				
2	9086	FASTCLIP TWIN-STEM SHOULDER		4				
3	7692	STANDARD SIDE POST INSULATOR FOR TWIN-STEM SHOULDER	NUETRAL	4				
4	7083	RAIL PAD ASSEMBLY		2				

- FOR RAIL PAD DETAILS, SEE SCRRA ES2364.
   FOR SIDE POST INSULATOR DETAILS, SEE SCRRA ES2365.

- SEE SCRRA ES2365.

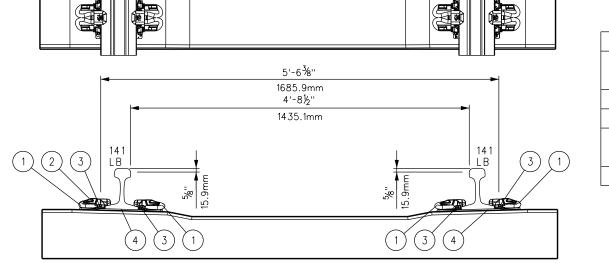
  3. FOR RAIL CLIP DETAILS, SEE SCRRA ES2366.

  4. FOR TOE INSULATORS DETAILS,
  SEE SCRRA ES2367.

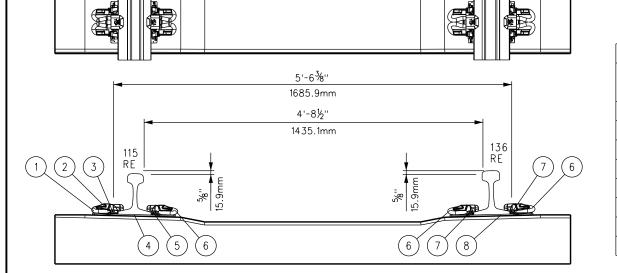
  5. ALL COMPONENTS FOR TIE ASSEMBLIES TO BE
  PANDROL TYPE OR EQUIVALENT AS APPROVED
  BY THE SCRRA DIRECTOR OF ENGINEERING AND
- CONSTRUCTION.

  6. ALL PART NUMBERS LISTED ON THIS DRAWING CORRESPOND TO PANDROL BRAND COMPONENTS AND ARE SUBJECT TO CHANGE.

  7. FOR CONCRETE TIE DETAILS AND FRICTION
- PATTERN, SEE SCRRA ES2402.



		141 LB RAIL AND 136 LB RAIL CONCRETE TIE		
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY
1	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NUETRAL	4
2	9086	FASTCLIP TWIN-STEM SHOULDER		4
3	7692	STANDARD SIDE POST INSULATOR FOR TWIN-STEM SHOULDER	NUETRAL	4
4	7083	RAIL PAD ASSEMBLY		2



TITEM NO DESCRIPTION COLOR COLOR COLOR 1 10218 RAIL CLIP ASSY - FC1603 CLIP / 7695 TOE INSULATOR BLUE 2 9086 FASTCLIP TWIN-STEM SHOULDER 3 11458 SIDE POST INSULATOR - 0.726" THICK POST BLUE	COMBINATION 115 RE RAIL AND 136 RE RAIL								
2 9086 FASTCLIP TWIN-STEM SHOULDER	ГҮ								
3 11458 SIDE POST INSULATOR - 0.726" THICK POST BLUE	1								
4 11549 RAIL PAD									
5 11459 SIDE POST INSULATOR - 0.430" THICK POST GREEN									
6 10216 RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR NUETRAL	3								
7 7692 STANDARD SIDE POST INSULATOR NUETRAL	2								
8 7083 RAIL PAD ASSEMBLY									

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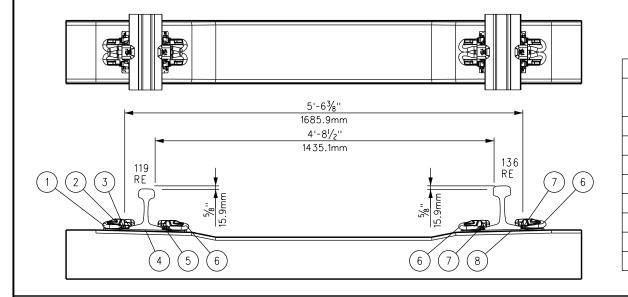
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DANIDDOL FACTOLID CONCDETE TIE ACCEMBLIEC
PANDROL FASTCLIP CONCRETE TIE ASSEMBLIES
FOR VARIOUS RAIL COMPINATIONS
FOR VARIOUS RAIL COMBINATIONS

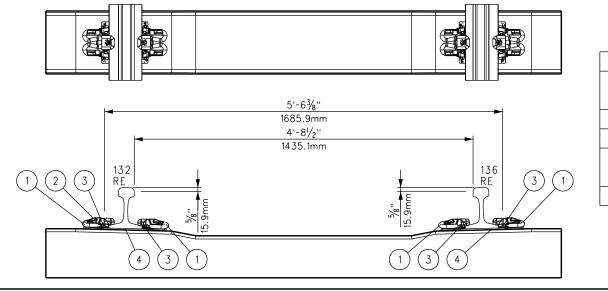


	COMBINATION 119 RE RAIL AND 136 LB RAIL							
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY				
1	10218	RAIL CLIP ASSY - FC1603 CLIP / 7695 TOE INSULATOR	BLUE	1				
2	9086	FASTCLIP TWIN-STEM SHOULDER		4				
3	11458	SIDE POST INSULATOR - 0.726" THICK POST	BLUE	1				
4	11549	RAIL PAD		1				
5	11459	SIDE POST INSULATOR - 0.430" THICK POST	GREEN	1				
6	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NUETRAL	3				
7	7692	STANDARD SIDE POST INSULATOR	NUETRAL	2				
8	7083	RAIL PAD ASSEMBLY		1				

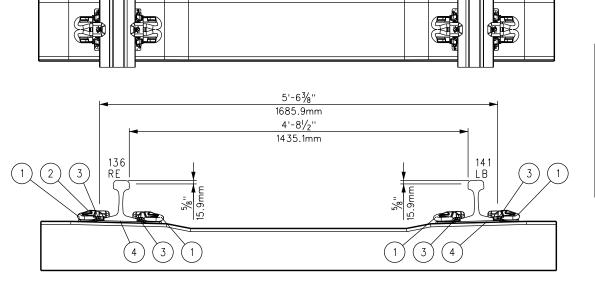
- 1. FOR RAIL PAD DETAILS, SEE SCRRA ES2364. 2. FOR SIDE POST INSULATOR DETAILS,
- SEE SCRRA ES2365. 3. FOR RAIL CLIP DETAILS, SEE SCRRA ES2366.
- 3. FOR RAIL CLIP DETAILS, SEE SCRRA ES2366.
  4. FOR TOE INSULATORS DETAILS, SEE SCRRA ES2367.
  5. ALL COMPONENTS FOR TIE ASSEMBLIES TO BE PANDROL TYPE OR EQUIVALENT AS APPROVED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
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  6. ALL PART NUMBERS LISTED ON THIS DRAWING CORRESPOND TO PANDROL BRAND COMPONENTS AND ARE SUBJECT TO CHANGE.

  7. FOR CONCRETE TIE DETAILS AND FRICTION
- PATTERN, SEE SCRRA ES2402.



136 RE RAIL AND 136 LB RAIL CONCRETE TIE							
PART NO	DESCRIPTION	COLOR	QTY				
10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NUETRAL	4				
9086	FASTCLIP TWIN-STEM SHOULDER		4				
7692	STANDARD SIDE POST INSULATOR FOR TWIN-STEM SHOULDER	NUETRAL	4				
7083	RAIL PAD ASSEMBLY		2				
	10216 9086 7692	PART NO  DESCRIPTION  10216  RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR  9086  FASTCLIP TWIN-STEM SHOULDER  7692  STANDARD SIDE POST INSULATOR FOR TWIN-STEM SHOULDER	PART NO DESCRIPTION COLOR  10216 RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR NUETRAL  9086 FASTCLIP TWIN-STEM SHOULDER  7692 STANDARD SIDE POST INSULATOR FOR TWIN-STEM SHOULDER				



	141 LB RAIL AND 136 LB RAIL CONCRETE TIE								
ITEM NO	PART NO	DESCRIPTION	COLOR	QTY					
1	10216	RAIL CLIP ASSY - FC1601 CLIP / 7695 TOE INSULATOR	NUETRAL	4					
2	9086	FASTCLIP TWIN-STEM SHOULDER		4					
3	7692	STANDARD SIDE POST INSULATOR	NUETRAL	4					
4	7083	RAIL PAD ASSEMBLY		2					

					DRAWN BY: A. CARLOS DATE: 04/12/02
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					ASSISTANT DIRECTOR: STANDARDS & DESIGN
Α	05-16-16	REVISED RAIL PAD ASSEMBLY NUMBER	AC	NDP	1
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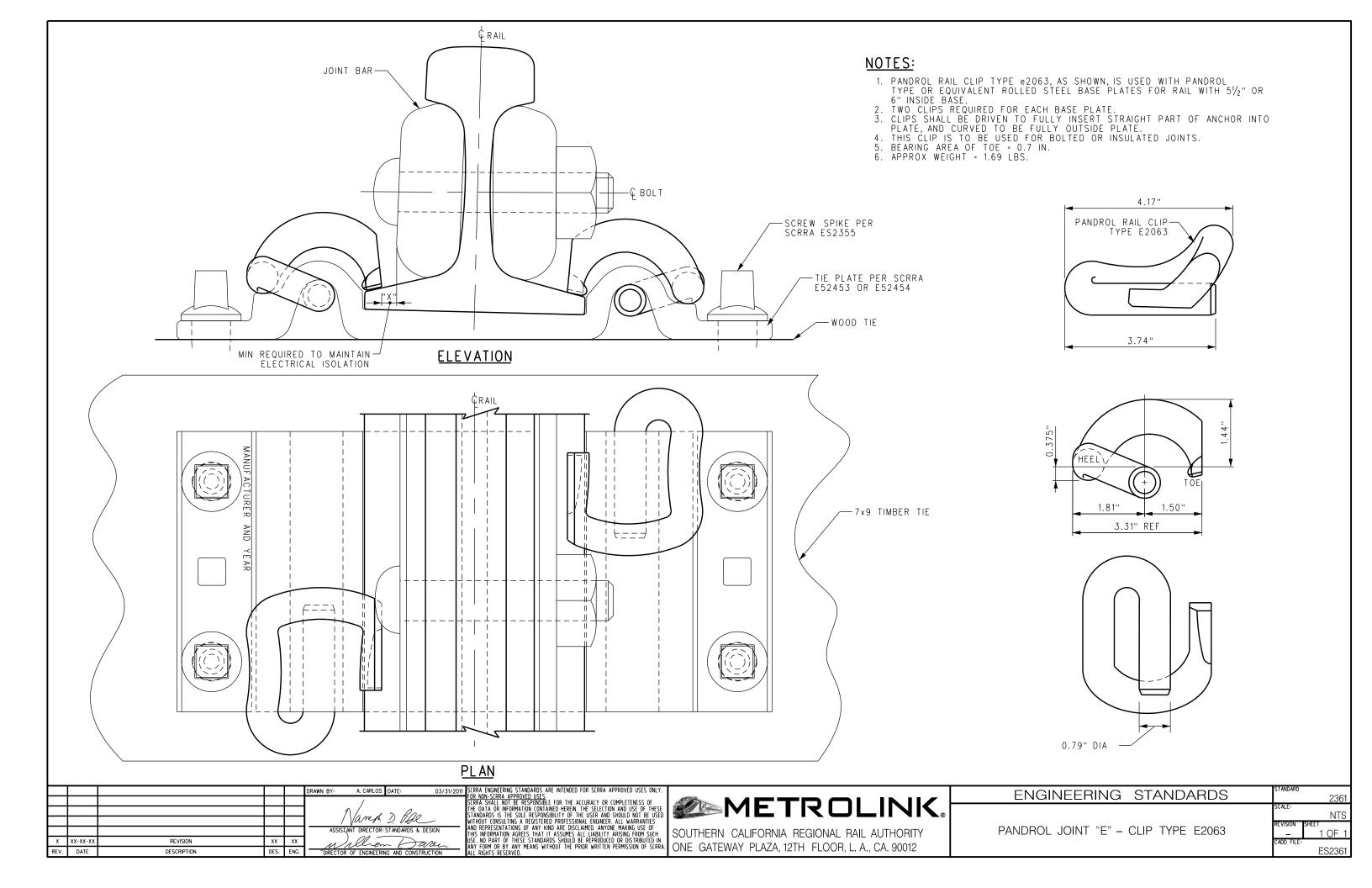
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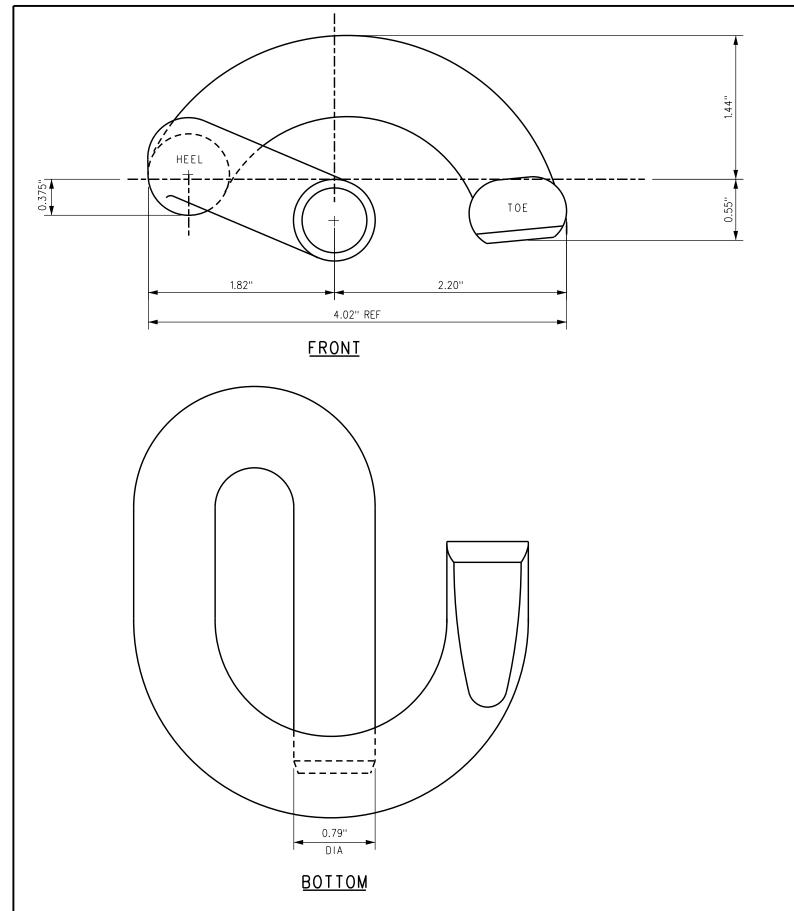
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PANDROL FASTCLIP CONCRETE TIE ASSEMBLIES
PANDROL PASTOLIP CONCRETE HE ASSEMBLIES
FOR VARIOUS RAIL COMBINATIONS
FOR VARIOUS RAIL COMBINATIONS





4	4.17"
-	3.74"
•	SIDE

(RH CLIP SHOWN, LH OPPOSITE)

# NOTES:

- 1. PANDROL PART NO 2055 2. CLIP IS STANDARD TYPE. GALVANIZED CLIP IS NOT REQUIRED TO COMPLY WITH THIS STANDARD.

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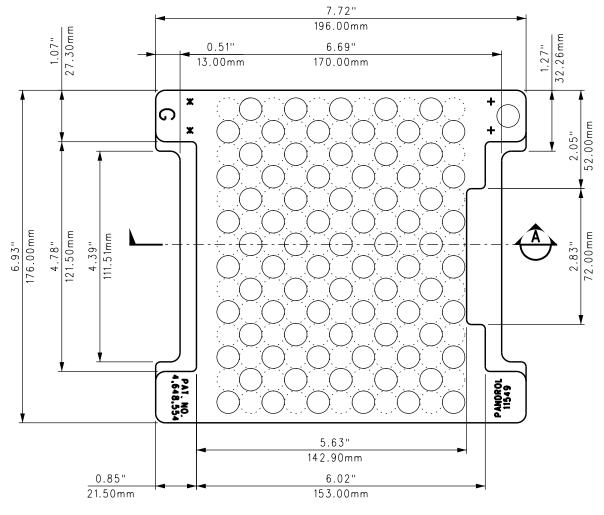
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**METROLINK**®

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

ENGINEERING STANDARDS NTS PANDROL BRAND RAIL CLIP 1 OF 1 "E" CLIP ES2362



0.85"
21.50mm

FASTCLIP TIE PAD
FOR 6" RAIL
PANDROL RAIL PAD ASSEMBLY
(PART #7083)

0.51"

13.00mm

FASTCLIP TIE PAD
FOR 5/2" RAIL
USING SCRRA STANDARD
6" BASE CONCRETE TIE
(PART \*11549)

SECTION A

<b>†</b>		
0.256" 6.5mm	SECTION	B

7.72"

196.00mm

6.69"

170.00mm

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SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012 ENGINEERING STANDARDS

PANDROL CONCRETE TIE PADS FOR 5½" & 6" RAIL BASE

2364

SCALE: NTS

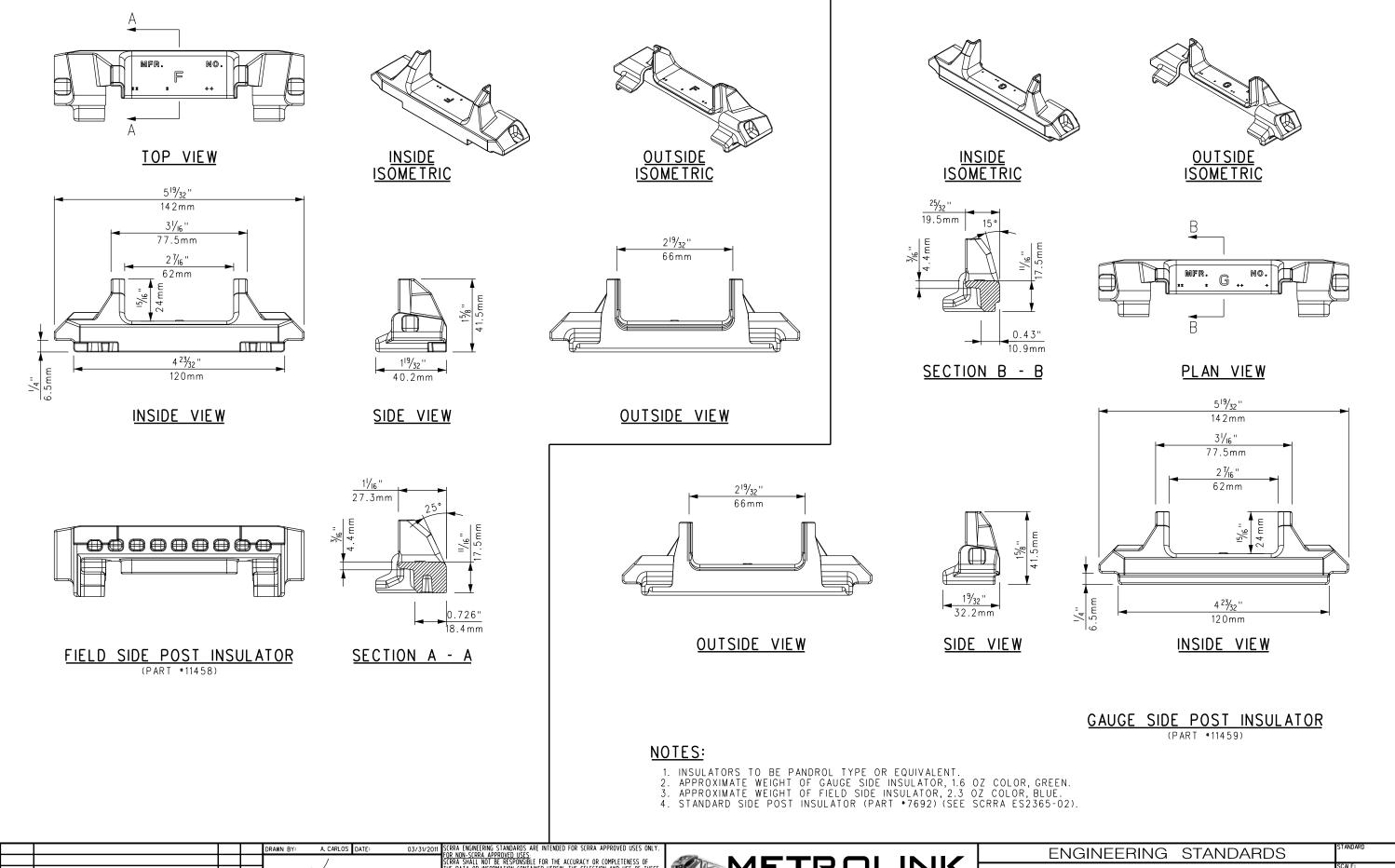
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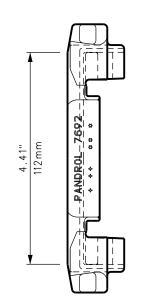
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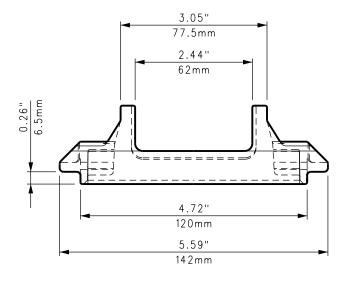
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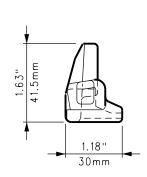
4.39" 111.51mm

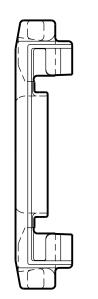


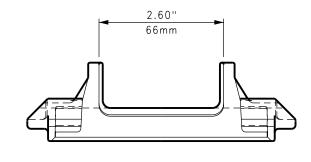
				/	FOR NON-SCRRA APPROVED USES: SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF		ENGINEERING STANDARDS	2365
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				Ware 2. Vall	WITHOUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER. ALL WARRANTIES			REVISION SHEET
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Х	XX-XX-XX	REVISION	XX XX	11 Illiam Daras		DNE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012		CADD FILE:
REV.	DATE	DESCRIPTION	DES. ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	ALL RIGHTS RESERVED.	JNE GATEWAY PLAZA, IZTH FLOOK, L. A., CA. 90012		ES2365-01

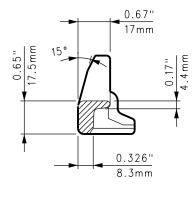












1. COLOR: NATURAL (OFF-WHITE) OR AS SPECIFIED ON PURCHASE ORDER.

STANDARD SIDE POST INSULATOR (PART \*7692)

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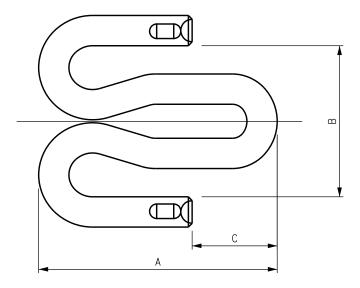
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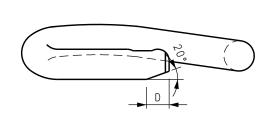
ENGINEERING STANDARDS	STANDARD 2365
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INSULATOR SIDE POST FOR FC1600 SERIES PANDROL FASTCLIP	REVISION SHEET  - 2 OF 2
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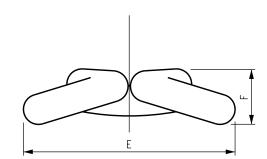
ES2365-02

- 1. PANDROL RAIL CLIP TYPE FC1601 AND FC1603 AS SHOWN ARE USED WITH PANDROL TYPE OR EQUIVALENT FASTCLIP CONCRETE TIE ASSEMBLIES FOR  $5 \frac{1}{2}$ " BASE AND 6" BASE RAIL RESPECTIVELY.
- 2. TWO CLIPS ARE REQUIRED PER ASSEMBLY. SEE SCRRA ES2360-01 THROUGH ES2360-03.



	DIMENSION TABLE					
		FAST C IPS (OR	LIP TYPE EQUAL)			
DIM	INCHES	mm	INCHES	mm		
А	4 31/32 ''	126	5 <sup>5</sup> / <sub>32</sub> ''	131		
В	3 <sup>5</sup> / <sub>32</sub> ''	80	3 <sup>5</sup> / <sub>32</sub> ''	80		
С	1 <sup>25</sup> / <sub>32</sub> ''	45	2 3/32 ''	53		
D	15/32 ''	12	15/32 ''	12		
E	4 13/32 ''	112	4 13/32 ''	112		
F	11/8"	29	11/16"	27		
NO	FC1601 (13	6 LB)	FC1603 (115	-119 LB)		





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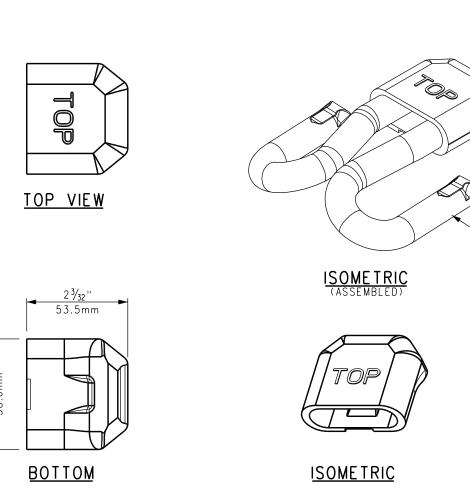
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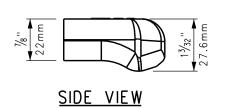
ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

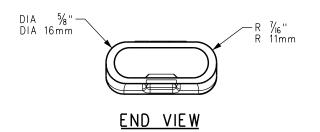
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PANDROL TYP	PE FASTCLIP
136LB. FC1601 AND	115-119LB. FC1603

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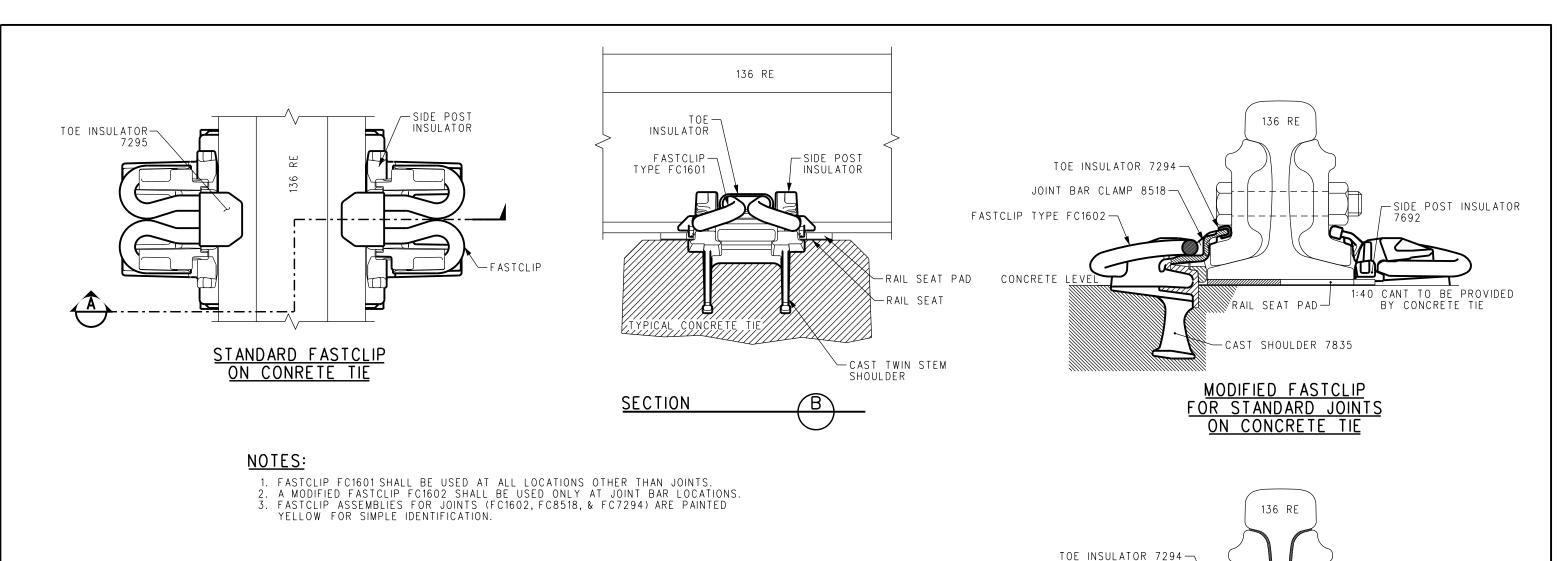


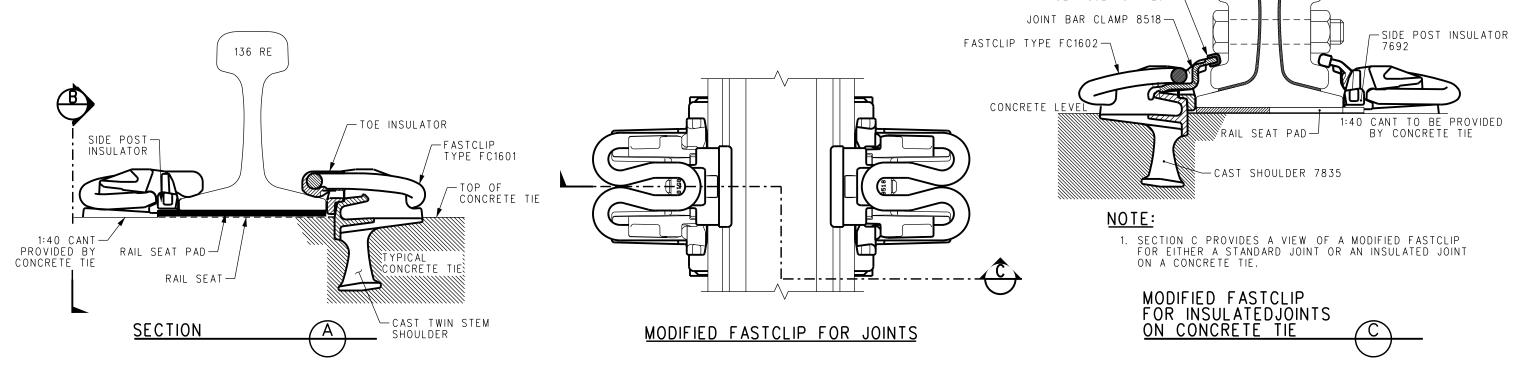
-FASTCLIP CLOSED TOE INSULATOR TYPE 7695 COLOR: BLUE

-FASTCLIP TYPE FC1601 OR FC1603

- TOE INSULATOR TO BE PANDROL TYPE 7695 OR EQUIVALENT. INSULATOR COLOR: BLUE
   FOR USE WITH PANDROL FASTCLIP TYPE FC1601, FC1603, OR EQUIVALENT.
   TYPE 7695 TOE INSULATOR IS A HEAVY DUTY PART NUMBER.

			DRAWN BY: A. CARLOS DATE: 03/31/2011	SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY. FOR NON-SCRRA APPROVED USES. SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF	ENGINEERING STANDARDS	STANDARD 2367
			Narch D. Rge	THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE STANDARDS IS THE SOLE RESPONSIBILITY OF THE USER AND SHOULD NOT BE USED	PANDROL TYPE TOE INSULATOR	SCALE: NTS
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X REV.	XX-XX-XX DATE	REVISION         XX         XX           DESCRIPTION         DES.         ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	USE NO PART OF THESE STANDARDS SHOULD BE REPRODUCED OR DISTRIBUTED IN ANY FORM OR BY ANY MEANS WITHOUT THE PRIOR WRITTEN PERMISSION OF SCRRA.  ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012  ALL RIGHTS RESERVED.	1600 SERIES RAIL CLIPS	CADD FILE: ES2367





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# **METROLINK**

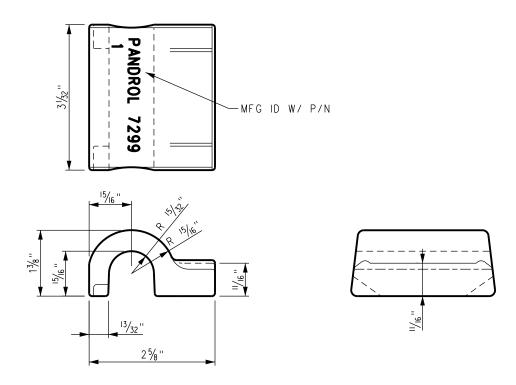
SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

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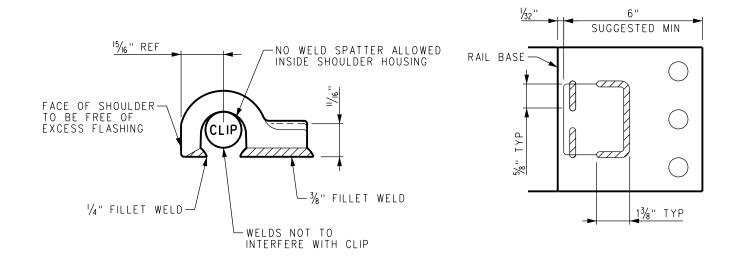
PANDROL TYPE TOE INSULATOR TO SUIT PANDROL FASTCLIP 1600 SERIES RAIL CLIPS FOR STANDARD RAIL & JOINT APPLICATIONS

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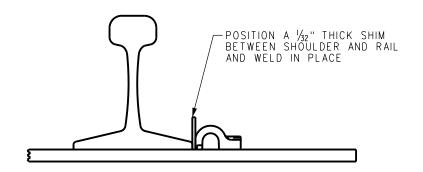
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WELD-ON SHOULDER PANDROL P/N 7299-1



# WELDING DETAIL OF 7299-1 SHOULDER

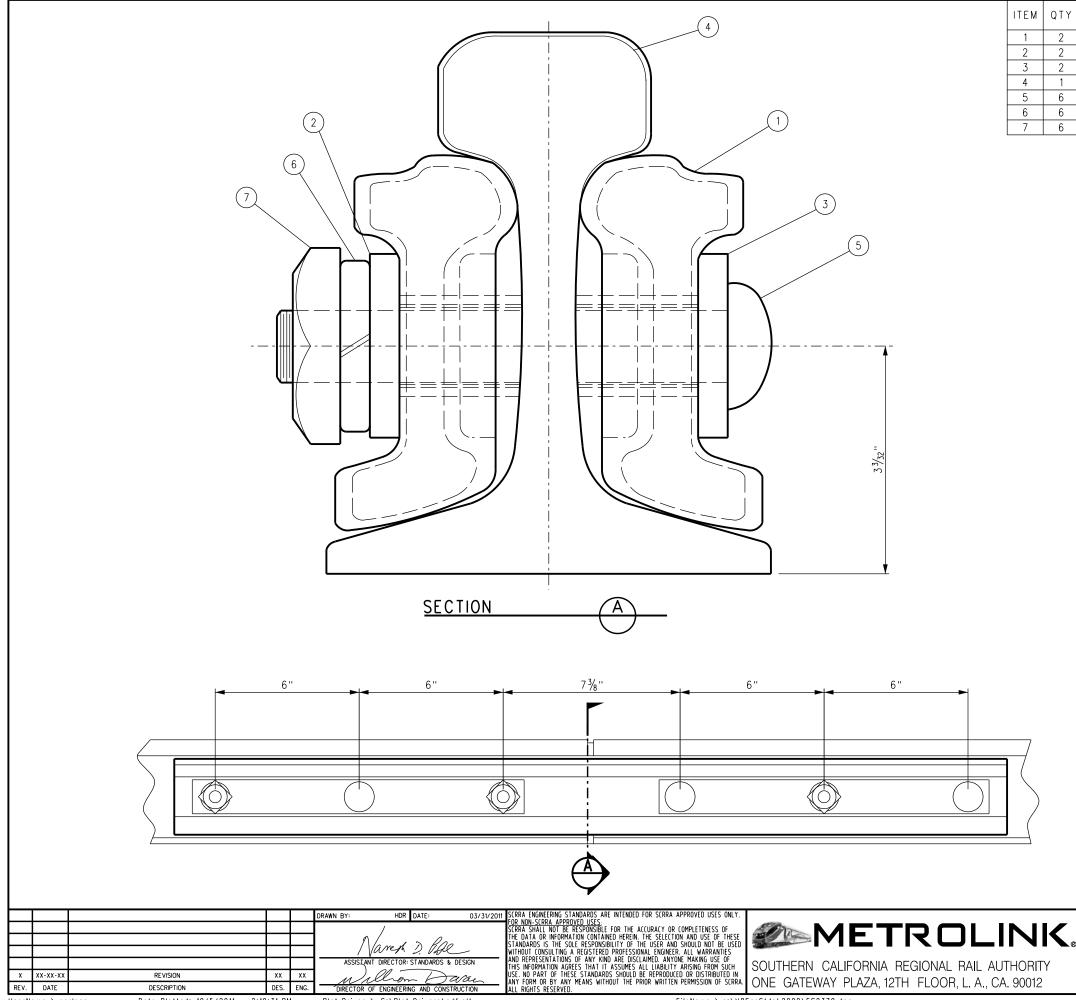


# POSITIONING SHOULDER

## NOTES:

- ALL WELDS TO BE IN ACCORDANCE WITH AREMA WELDING SPECIFICATIONS, V<sub>8</sub>" 7018 WELDING ROD, 3-PASSES.
   ALL WELDS TO HAVE FULL PENETRATION TO BOTH PLATE AND SHOULDER.
   WELD MUST NOT INTERFERE WITH EITHER THE RAIL OR THE CLIP.
   RAIL SEATS AND INSIDE SHOULDER HOUSING TO BE FREE OF EXCESS WELD, SLACE AND SPATER
- SLAG, AND SPATTER.
- 5. SHOULDERS TO BE TACKED (OR CLAMPED) DOWN PRIOR TO FINAL WELDING, TO PREVENT THE CLIP FROM RISING DURING THE FINAL WELDS.
  6. SHOULDER TO BE GENERALLY CENTERED ON THE PLATE, IF POSSIBLE.

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	ITEM	QTY	DESCRIPTION
	1	2	POLY INSULATED JOINT BAR 361/2" LONG
	2	2	STEEL BACKUP PLATE ½" THICK X 15¼" LONG
	3	2	STEEL BACKUP PLATE ½" THICK X 15¼" LONG
	4	1	SCOTCHPLY END POST 🔏 "THICK
	5	6	OVAL NECK BOLT 1" X 7½" LONG
	6	6	1" LOCKWASHER
	7	6	1" SQUARE NUT

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

POLY - INSULATED JOINT 141-136-132 LB. RE RAIL

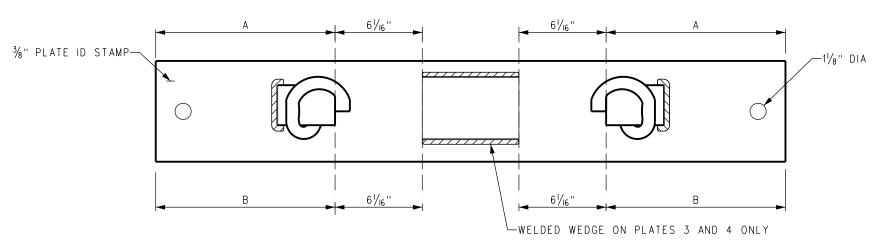
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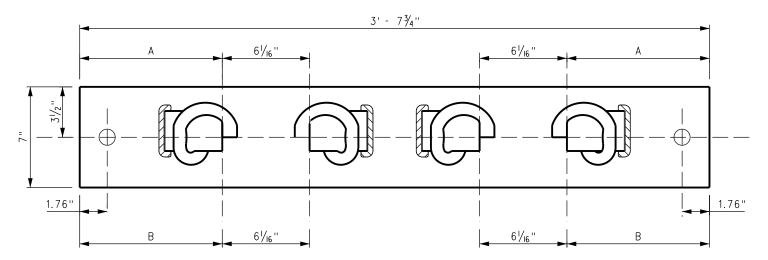
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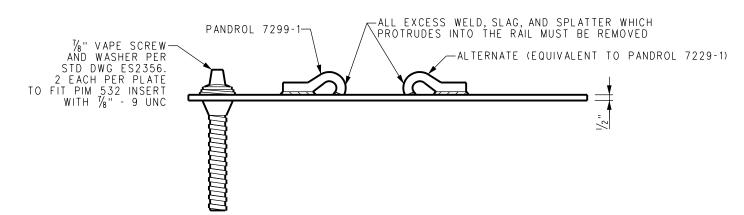
DESCRIPTION



# PLATES 1 - 4



# PLATES 5-8 AND STANDARD



# DOUBLE INSIDE GUARD RAIL PLATE DETAILS

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# **METROLINK**

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

# 1/2" X 4" A36 STEEL BLOCK (WEDGE) FOR PLATES 3 & 4

PLATE ID	C (IN)	D (IN)
3	2.65	3.14
4	5.34	5.74

## ENTRY PLATES

PLATE ID	A (IN)	B (IN)
1	17.12	16.75
2	15.85	15.69
3	14.59	14.15
4	13.21	12.87
5	12.11	11.83
6	11.24	11.03
7	10.60	10.47
8	10.21	10.14

# STANDARD PLATE

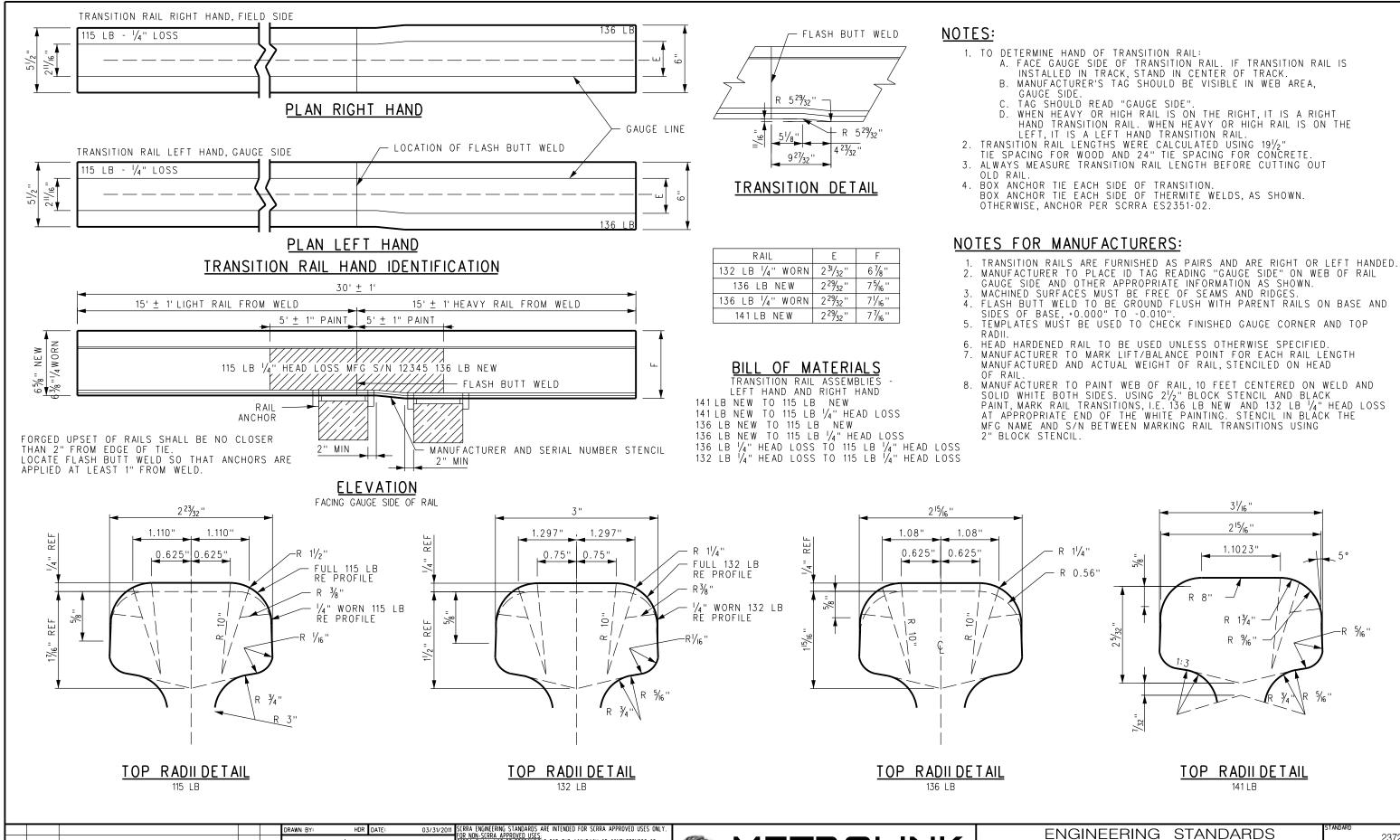
PLATE I	A (IN)	B (IN)
STD	10.05	10.05

DOUBLE INSIDE GUARD RAIL ITEM NUMBERS SET INCLUDES ALL ENTRY PLATES (2 OF EACH) STD PLATE (EACH) ENTRY PLATES 1-8 (SET)

# NOTES:

- 1. NO INSIDE CLIPS FOR PLATES 1, 2, 3, & 4. REQUIRES STEEL WEDGE ON PLATES 3 & 4.
  2. IF 5½" BASE RAIL WILL BE USED FOR GUARD RAIL, THEN DIMENSIONS A & B ARE TO BE INCREASED BY 0.50 (IN), AND THE RAIL SEAT DIMENSION WILL CHANGE FROM 61/16" TO 51/16"

**ENGINEERING STANDARDS** NTS INSIDE GUARD RAIL PLATES FOR CONCRETE TIES 1 OF -ES2371



SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA, 90012

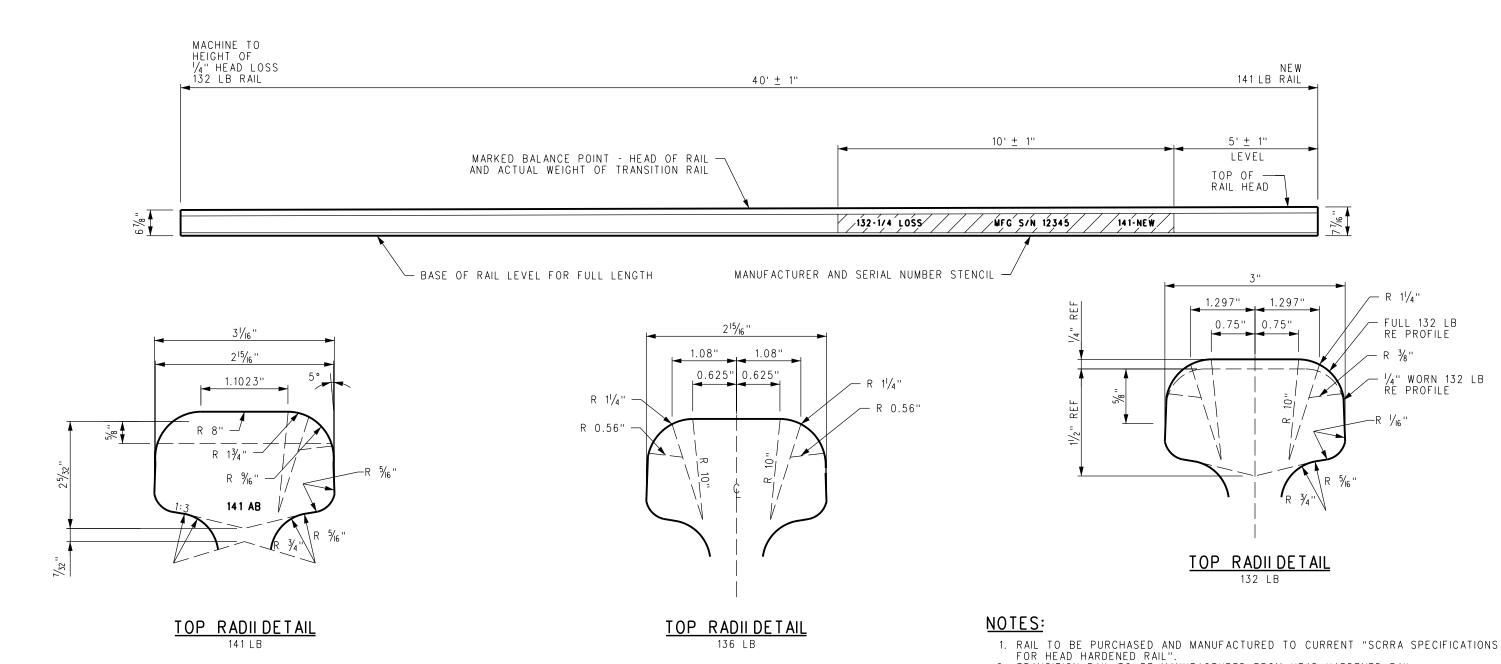
FORGED TRANSITION RAILS FOR NEW 141 / 136LB. TO 115LB. NEW AND 1/4" HEAD LOSS RAIL

NTS 1 OF ES2372

REV. DATE

DESCRIPTION

OF ENGINEERING AND CONSTRUCTION



RAIL HEIGHT						
SECTION	NEW	¼" HEAD LOSS				
141 LB	7 ½ <sub>6</sub> ''	-				
136 LB	7 5/ <sub>16</sub> ''	-				
132 LB	-	6 7/8"				

## BILL OF MATERIALS

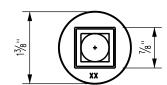
QUANTITY TRANSITION RAIL 1 EA TRANSITION RAIL, 141 LB NEW TO 132 LB 1/4" HEAD LOSS 1 EA TRANSITION RAIL, 136 LB NEW TO 132 LB 1/4" HEAD LOSS

- 2. TRANSITION RAIL TO BE MANUFACTURED FROM HEAD HARDENED RAIL.

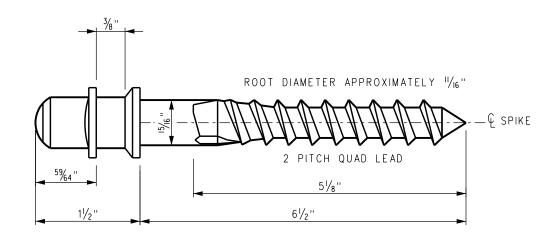
- 3. MACHINED SURFACES MUST BE FREE OF SEAMS AND RIDGES.
  4. TEMPLATES MUST BE USED TO CHECK FINISHED GAUGE CORNER AND TOP RADII.
  5. MANUFACTURER TO MARK LIFT/BALANCE POINT FOR EACH RAIL LENGTH AND ACTUAL WEIGHT OF RAIL, STENCILED ON HEAD OF RAIL.
- MANUFACTURER SHALL PAINT WEB OF RAIL, 10 FEET AS SHOWN ON RAIL, SOLID WHITE, BOTH SIDES. USING 2½" BLOCK STENCIL AND BLACK PAINT, MARK RAIL TRANSITIONS, I.E. 141-NEW AND 132 ¼" LOSS AT EACH END OF THE WHITE PAINTING. STENCIL IN BLACK THE MFG NAME AND S/N BETWEEN MARKING RAIL TRANSITIONS
- USING 2" BLOCK STENCIL 7. THE 141 LB TRANSITION RAIL CAN BE USED WITH 136 LB AND 132 LB RAIL SECTIONS
- NEW TO 1/4" HEAD LOSS.

  8. TRANSITION RAIL IS UNIVERSAL AND CAN BE USED AS RIGHT HAND OR LEFT HAND RAIL.

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X REV.	XX-XX-XX DATE	REVISION         XX         XX           DESCRIPTION         DES.         ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	THE NO DADE OF THESE STANDADES SHOULD BE DEDDOODIED ON DISTURBILLED IN	ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012	TO 132LB. 1/4" HEAD LOSS	CADD FILE: ES2373



<u>PL AN</u>



EVERGRIP SCREW SPIKE 15/16" X 61/2" PART NUMBER SSEP2094-0650P

U.S. PATENT 6.471.140

# **ELEVATION**

# NOTES:

- 1. EASILY DRIVEN WITH CONVERTED AUTOMATIC SPIKE DRIVER, PORTABLE SPIKER OR MAUL.
  2. FINS LOCK THE SPIKE INTO THE WOOD PREVENTING BACKWARD ROTATION.
  3. CANNOT BE OVERDRIVEN.

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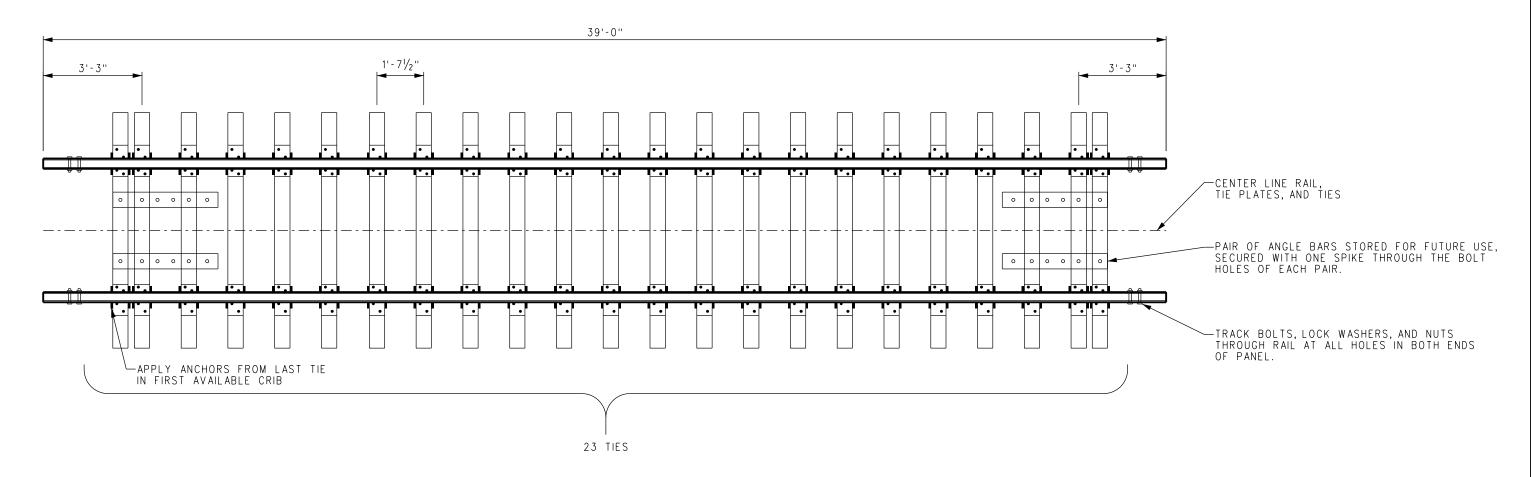
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ENGINEERING STANDARDS NTS EVERGRIP DOUBLE HEAD SPIKE 1 OF 1

ES2375



#### EMERGENCY TRACK PANEL

# PANEL WEIGHTS

PANEL WEIG	HTS IN LBS							
39' PANEL								
115 LB	136 LB							
10,300±	10,800±							

#### ALLOWABLE SECOND HAND RAIL WEAR

R AIL WEIGHT		VABLE AR
=	TOP	GAUGE
136 LB	3/8''	1/4"
115 LB	1/8"	1/4"
	3/8"	1/4"

### NOTES:

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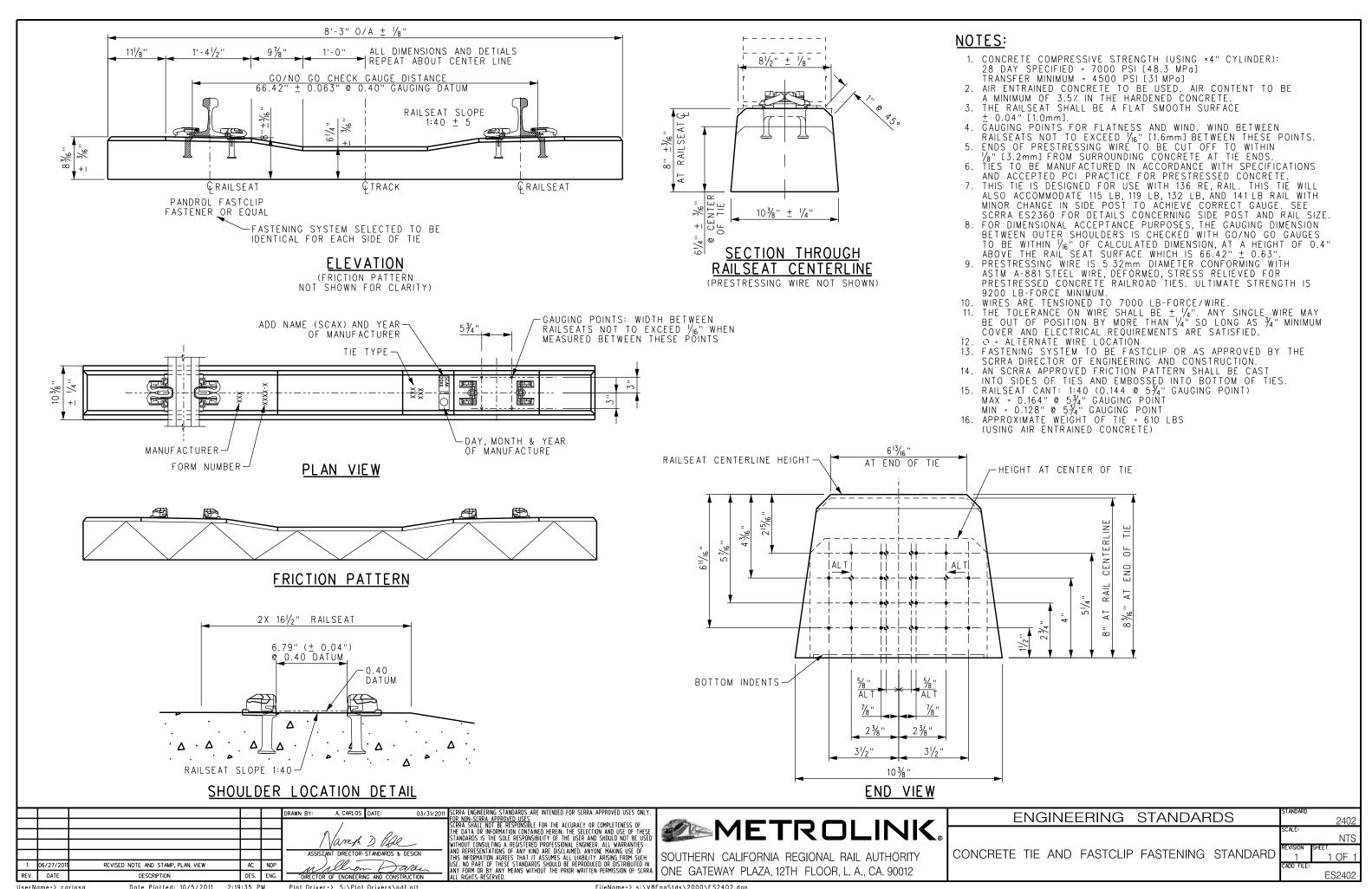
- 1. TRACK PANELS TO BE CONSTRUCTED AS ABOVE.
  2. NEW OR SECOND HAND RAIL. NEW RAIL TO BE DRILLED SECOND AND THIRD HOLES ONLY (HOLE NEAREST END NOT DRILLED). RAIL TO COMPLY TO CURRENT SPECIFICATIONS. RAIL TRANSPOSED SO THAT WORN RAIL IS ON FIELD SIDE OF PANELS. THE HEAD LOSS AND GAUGE FACE LOSS BETWEEN ANY TWO PANELS ON A SHIPMENT, MEASURED ON BOTH ENDS, SHOULD NOT BE MORE THAN ±1/8". GAUGE IS TO BE WITHIN ±1/8" TO PROPER GAUGE OF 56½" AT %" BELOW TOP OF BALL OF RAIL.
  3. NEW OR SECOND HAND TIE PLATES TO MATCH NEW OR SECOND HAND RAIL.
- 4. USE NEW ANCHORS.
- 5. DERAILMENT PANELS TO BE BOX ANCHORED EVERY TIE.
  6. ANCHOR PATTERN OF MAIN LINE PANELS ADJUSTED IN THE FIELD DURING INSTALLATION TO COMPLY WITH SCRRA ES2351-02.
- 7. USE NEW 7" X 9" X 8'-6" HARDWOOD TREATED TIES SPIKING PATTERN TO COMPLY
- WITH SCRRA ES2460-01, FIGURE A.

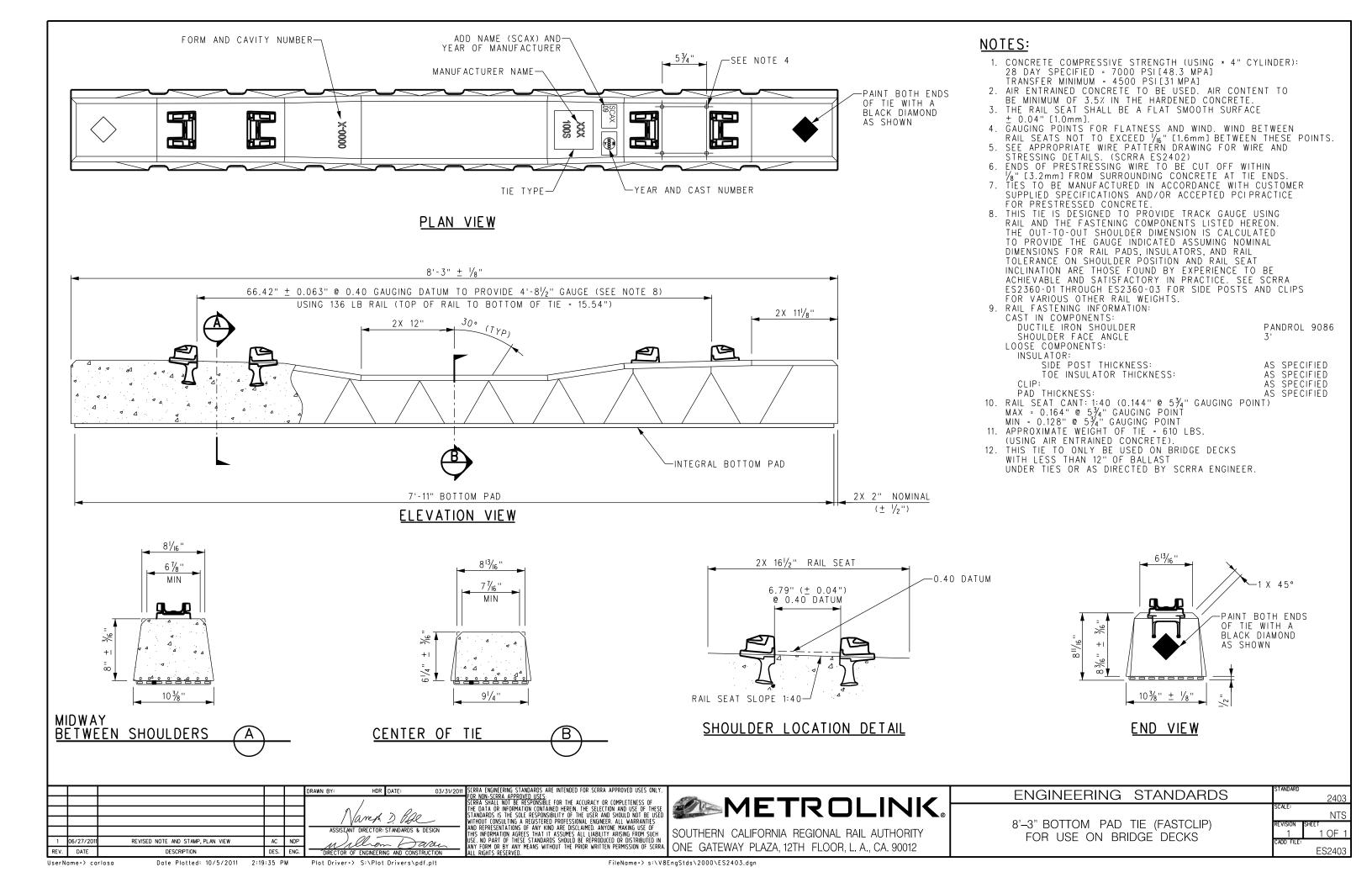
  APPROPRIATE NUMBER OF TRACK BOLTS, LOCK WASHERS AND NUTS FASTENED IN ALL BOLT HOLES IN RAILS. WRAP BOLT ENDS WITH DUCT TAPE TO PREVENT BOLT LOOSENING.
- ANGLE BARS ARE TO BE PLACED ON THE END TIES AS SHOWN ABOVE AND SPIKED TO THE TIE THROUGH THE BOLT HOLE WITH ONE SPIKE PER PAIR OF BARS. TIES ARE PERPENDICULAR TO RAIL AND SPACING IS ±1". RAIL ENDS TO MATCH
- WITHIN 1/4".

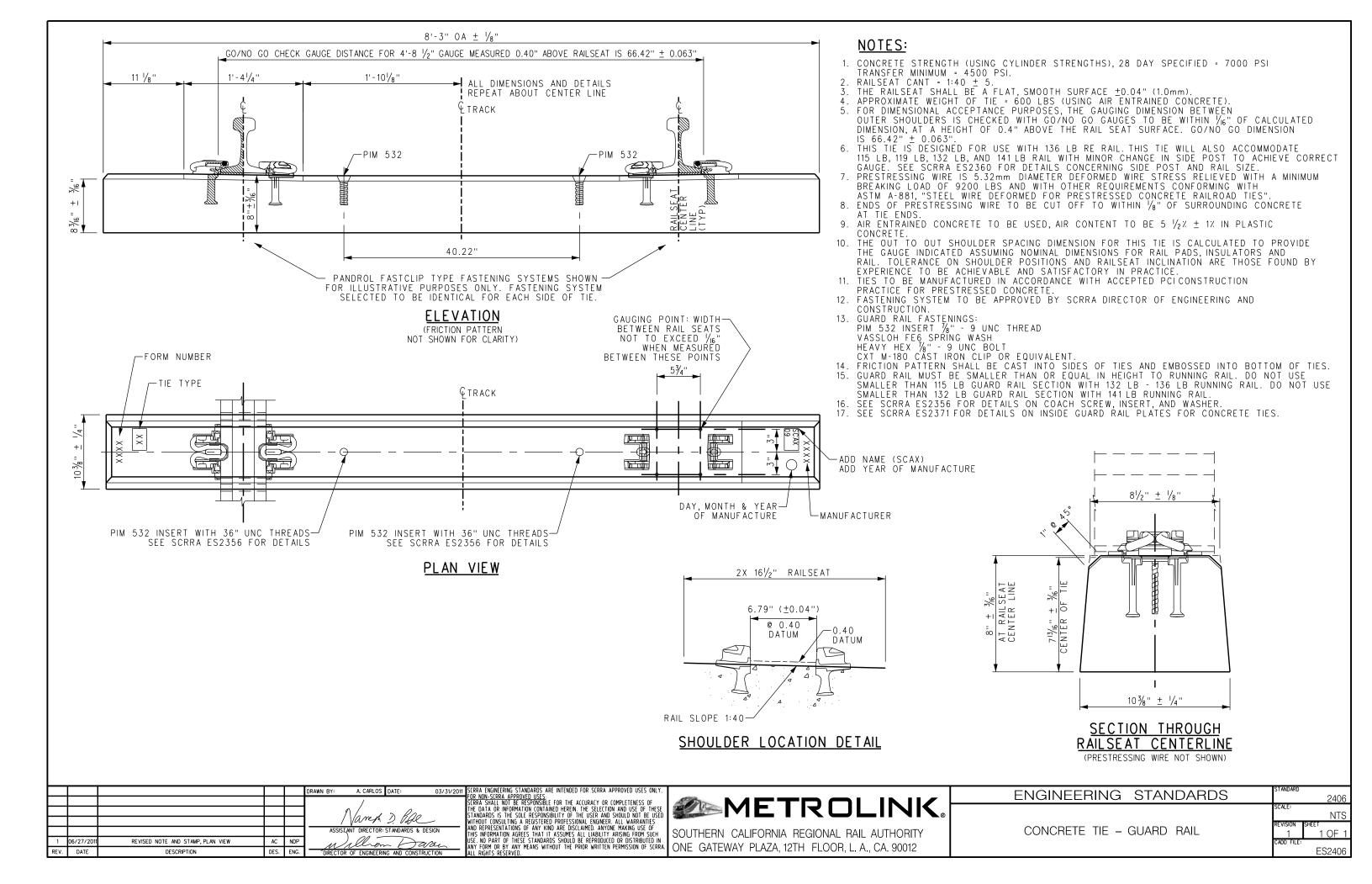
  11. PANELS TO BE MARKED TO INDICATE WEIGHT WITH INDELIBLE MARKER ON TOP OF RAIL AT CENTER OF PANEL.
- PANEL TO BE MARKED TO INDICATE NEW OR SECOND HAND RAIL WITH INDELIBLE MARKER ON SIDE OF RAIL NEAR CENTER OF PANEL.

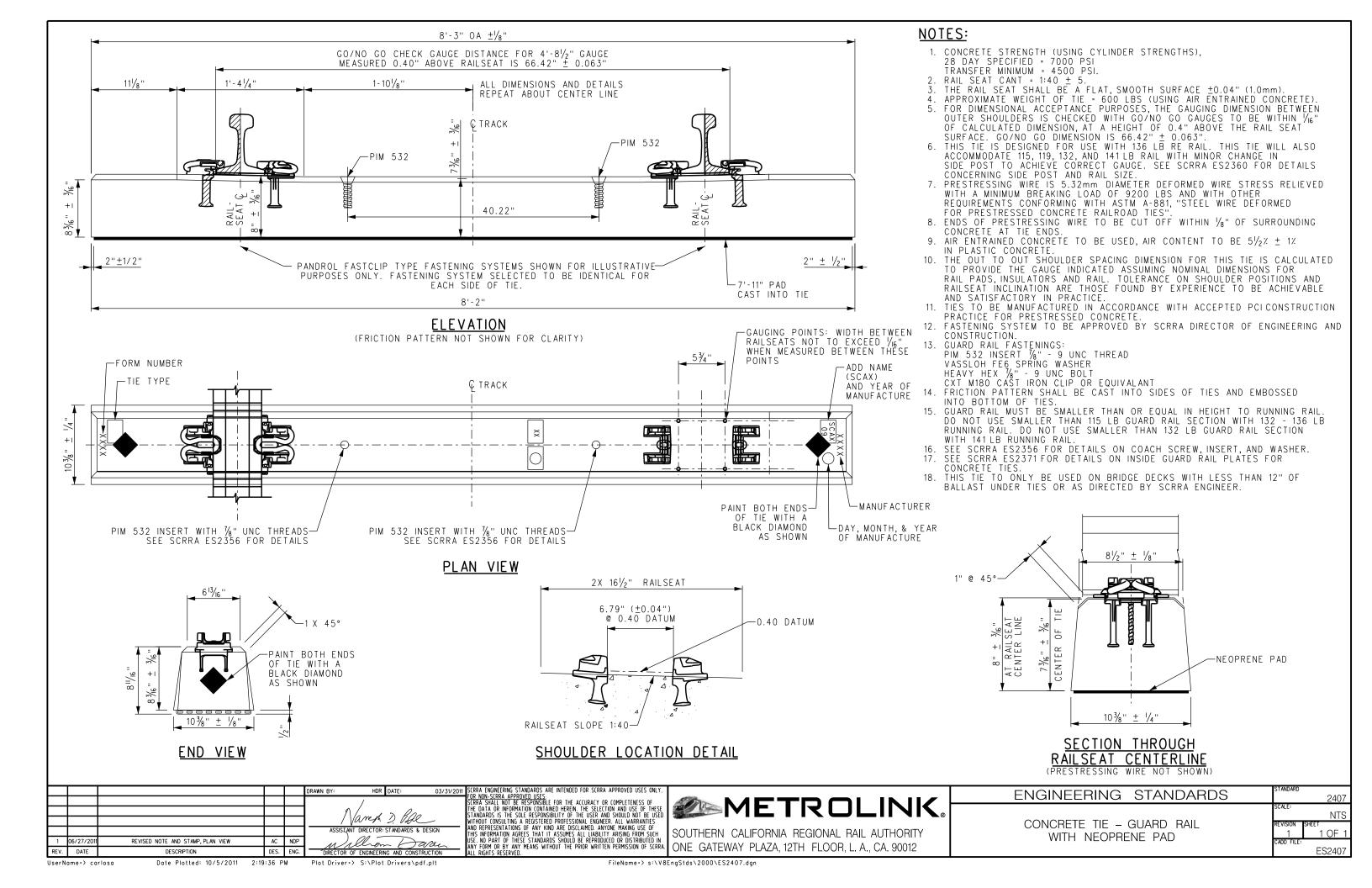
				DRAWN BY: HDR DATE: 03/31/2011	SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY.		ENGINEERING STANDARDS	STANDARD
				/	SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF	MFTROI INK	ENGINEERING STANDARDS	2380
				1/2 / 2 /22	THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE STANDARDS IS THE SOLE RESPONSIBILITY OF THE LISER AND SHOULD NOT BE USED.	MEIKULINK		SCALE:
				Wareh D. Vige	WITHOUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER. ALL WARRANTIES		TYPICAL TRACK PANELS	REVISION SHEET
				ASSISTANT DIRECTOR: STANDARDS & DESIGN	AND REPRESENTATIONS OF ANY KIND ARE DISCLAIMED. ANYONE MAKING USE OF THIS INFORMATION AGREES THAT IT ASSUMES ALL LIABILITY ARISING FROM SUCH	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY		- 1 OF 1
	XX-XX-XX	REVISION XX	XX	11) Illiam Darai-	NIICE NO DADT OF THECE CTANDADOC CHOILI O DE DEDDODUCED OD DICTDIRLITED IN		115 LB. AND 136 LB. TIMBER TIES	CADD FILE:
RE	. DATE	DESCRIPTION DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	ALL RIGHTS RESERVED.	ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012		ES2380

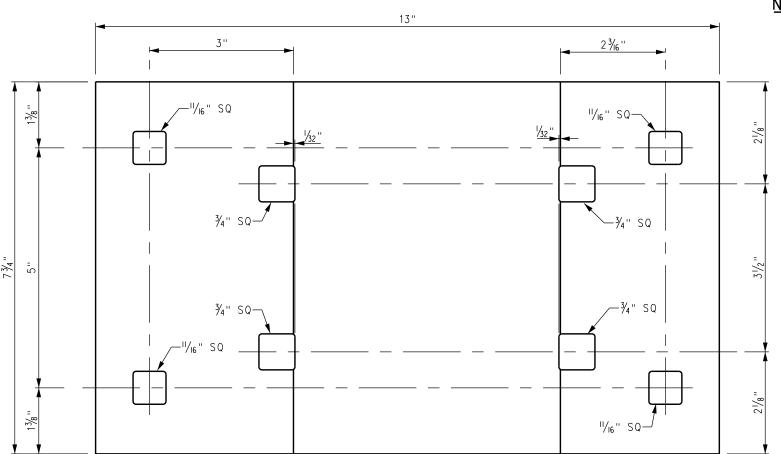
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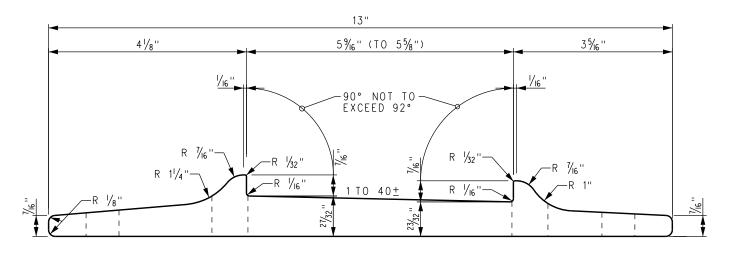






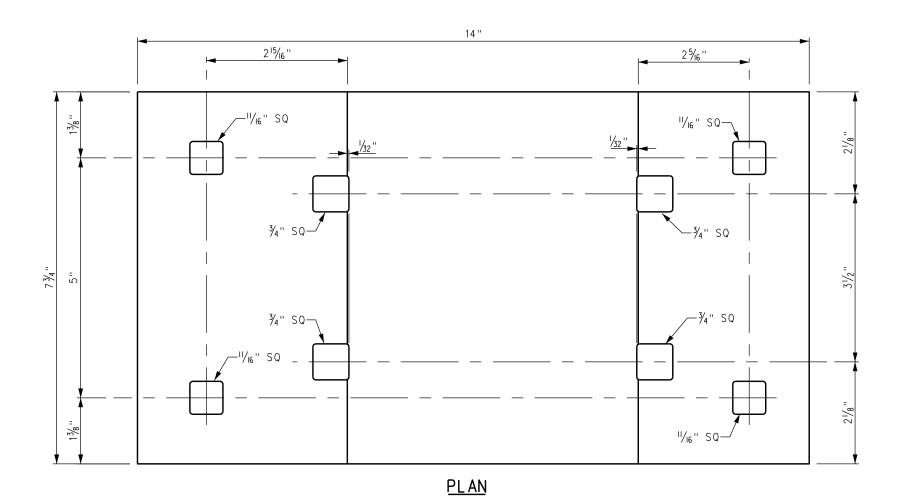
- 1. ALL SQUARE SPIKE HOLES SHALL HAVE 1/6" FILLETS IN CORNERS. 2. ESTIMATED WEIGHT: 19.60 LBS EACH. 3. MUST MEET AREMA SPECIFICATIONS.



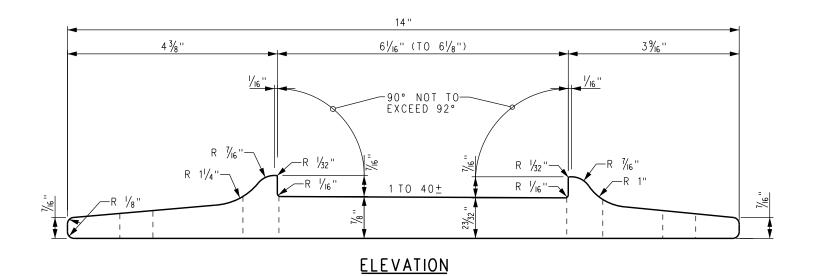


# **ELEVATION**

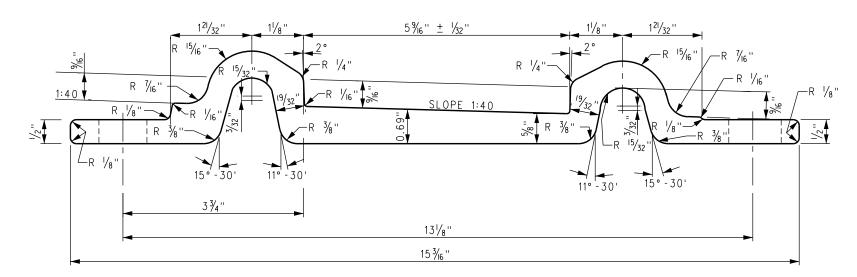
	_				SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY. FOR NON-SCRRA APPROVED USES:		ENGINEERING STANDARDS	STANDARD	2451
				$\Lambda$ / $\Lambda$ $\Lambda$	ISCRAC SHÂLL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE ISTANDARDS IS THE SOLE RESPONSIBILITY OF THE USER AND SHOULD NOT BE USED	<b>METROLINK</b>		SCALE:	NTS
$\vdash$	+			ACCICTANT DIDECTOR: CTANDADDC & DECICN	WITHOUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER. ALL WARRANTIES AND REPRESENTATIONS OF ANY KIND ARE DISCLAIMED. ANYONE MAKING USE OF	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY	1	REVISION SHE	EET 1 OF 1
×	xx-xx-xx	REVISION XX	xx		THISE NO DADE OF THESE STANDADDS SHOULD BE DEDDOODIGED OF DISTRICTED IN	ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012	FOR 5½" BASE RAIL	CADD FILE:	
RE	. DATE	DESCRIPTION DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION	ALL RIGHTS RESERVED.	ONE GATEWAT FLAZA, IZTIT FLOON, L. A., CA. 90012			ES2451



- 1. ALL SQUARE SPIKE HOLES SHALL HAVE 1/16" FILLETS IN CORNERS.
  2. ESTIMATED WEIGHT: 21.47 LBS EACH.
  3. MUST MEET AREMA SPECIFICATIONS.

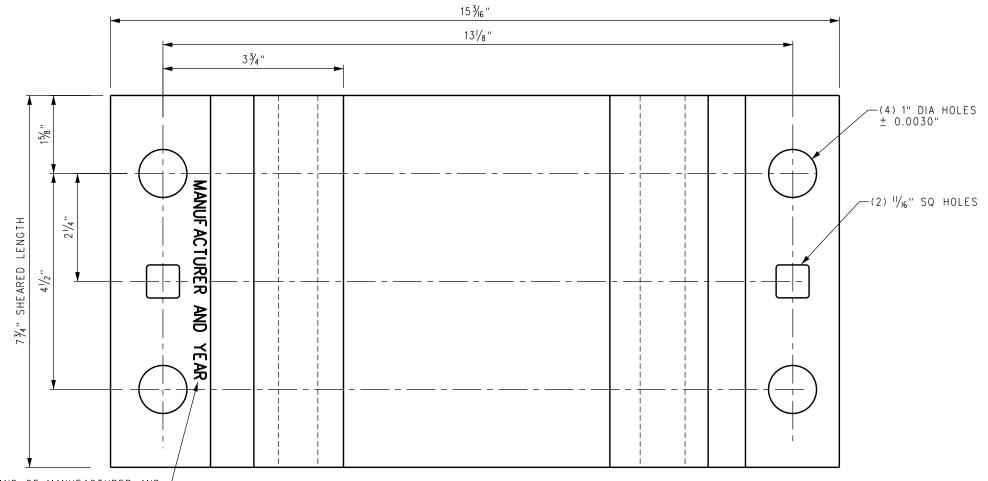


			DRAWN BY: A. CARLOS DATE: 03/31/2	SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY.		ENGINEERING STANDARDS	STANDARD
			/	SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF		ENGINEERING STANDARDS	2452
			$\Box$ $\wedge$ $\wedge$ $\wedge$ $\wedge$ $\wedge$ $\wedge$ $\wedge$	THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE STANDARDS IS THE SOLE RESPONSIBILITY OF THE USER AND SHOULD NOT REJUSED.	METROLINK.		SCALE:
			- Vareh D. Ville	WITHOUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER. ALL WARRANTIES			REVISION SHEET
			ASSISIANT DIRECTOR: STANDARDS & DESIGN	<ul> <li>AND REPRESENTATIONS OF ANY KIND ARE DISCLAIMED. ANYONE MAKING USE OF THIS INFORMATION AGREES THAT IT ASSUMES ALL LIABILITY ARISING FROM SUCH</li> </ul>	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY	14" TIE PLATE FOR 6" BASE RAIL	- 1 OF 1
х	xx-xx-xx	REVISION XX	XX 11 Illiam Darae	MILE NO DADT OF THESE STANDADDS SHOULD BE DEDDODIED OD DISTDIRITED IN			CADD FILE:
REV	. DATE	DESCRIPTION DES. E	ENG. DIRECTOR OF ENGINEERING AND CONSTRUCTION	ALL RIGHTS RESERVED.	ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012		ES2452



- 1. ALL HOLE MEASUREMENTS TO BE TAKEN FROM BOTTOM OF PLATE.
  2. PLATE TO BE STANDARD PANDROL TYPE OR APPROVED
  EQUAL TIE PLATE MODIFIED FOR 1" DIA HOLES.
  3. PLATE TO BE INSTALLED WITH 2 EACH PANDROL RAIL FASTENING "e" CLIP PER SCRRA ES2362.
- 4. PLATE TO BE INSTALLED WITH 4 EACH SCREW SPIKES PER PLATE PER SCRRA ES2355.

# **SECTION**



NAME OR BRAND OF MANUFACTURER AND-LAST TWO DIGITS OF YEAR MANUFACTURED
TO BE ROLLED IN RAISED LETTERS

**PLAN** 

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					None of a Rose
					/ Vareth D. Vasic
					ASSISIANT DIRECTOR: STANDARDS & DESIGN
Х	XX-XX-XX	REVISION	XX	XX	William Daran
REV.	DATE	DESCRIPTION	DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION

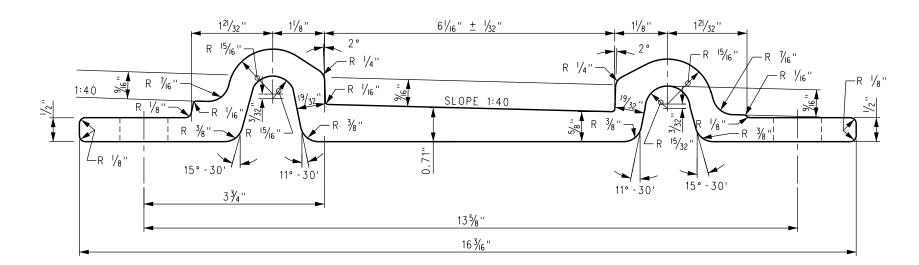
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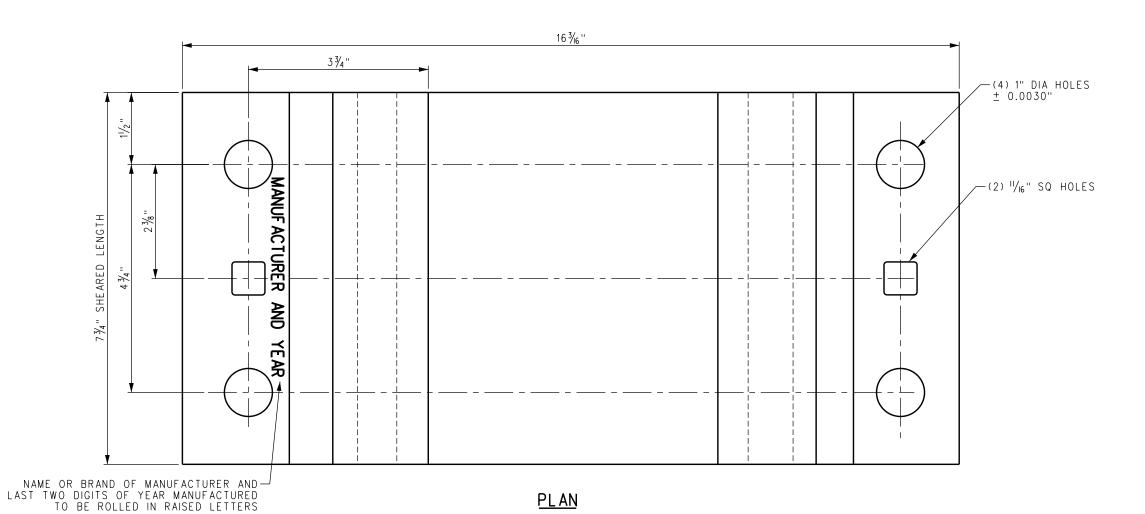
SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING	STANDARDS

ROLLED STEEL TIE PLATE TO SUIT 51/2" BASE AREMA RAIL AND PANDROL RAIL CLIPS E2055



# **SECTION**



A. CARLOS DATE: 03/31/2011 Varet D. Page ASSISIANT DIRECTOR: STANDARDS & DESIGN Illian Davae x xx-xx-x REVISION REV. DATE DES. ENG. DESCRIPTION

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**METROLINK** 

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS

NOTES:

1. ALL HOLE MEASUREMENTS TO BE TAKEN FROM BOTTOM OF

PLATE.

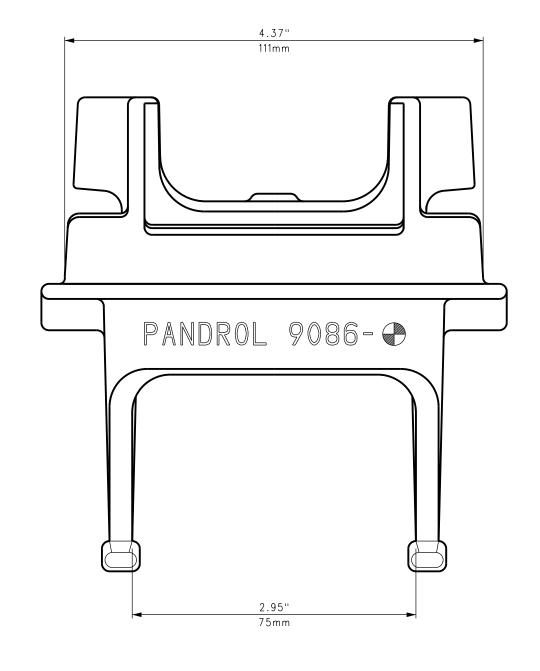
2. PLATE TO BE STANDARD PANDROL TYPE OR APPROVED EQUAL TIE PLATE MODIFIED FOR 1" DIA HOLES.

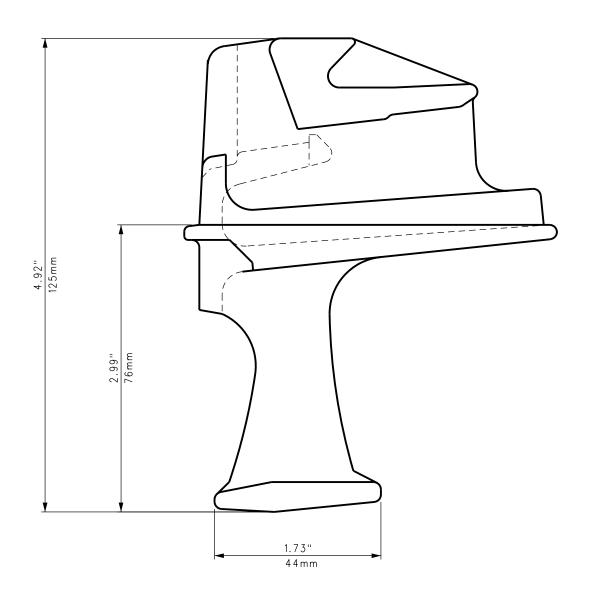
3. PLATE TO BE INSTALLED WITH 2 EACH PANDROL RAIL FASTENING "e" CLIP PER SCRRA ES2362.

4. PLATE TO BE INSTALLED WITH 4 EACH SCREW SPIKES PER SCRRA ES2355.

ROLLED STEEL TIE PLATE TO SUIT 132 LB. RE - 141 LB. RE RAIL AND PANDROL RAIL CLIPS E2055

2454 NTS 1 OF -ES2454





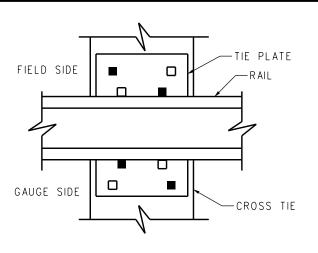
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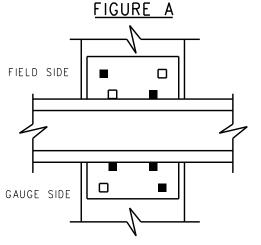
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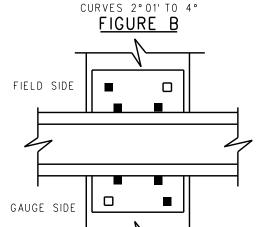
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#### TANGENT AND CURVES 2° AND LESS



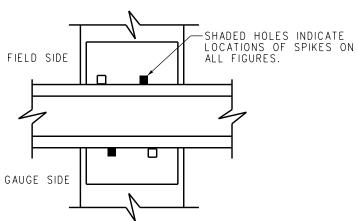


CURVES > 4° FIGURE C

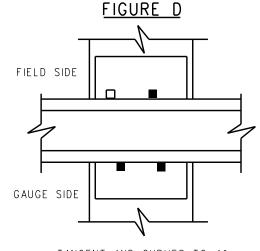
#### TIE PLATE WITH HOLD-DOWN SPIKE HOLES

"NEW CONSTRUCTION"

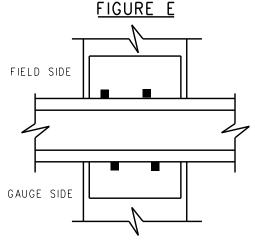
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TANGENT TRACK WHERE SPEED IS 25 MPH FREIGHT OR 30 MPH PASSENGER OR LESS YARD AND INDUSTRY TRACK < 6° CURVES



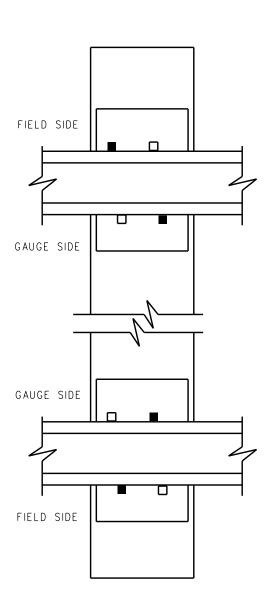
TANGENT AND CURVES TO 4° YARD AND INDUSTRY TRACK > 6° CURVES



CURVES > 4° FIGURE F

#### TIE PLATE WITHOUT HOLD-DOWN SPIKE HOLES

"MAINTENANCE ONLY"

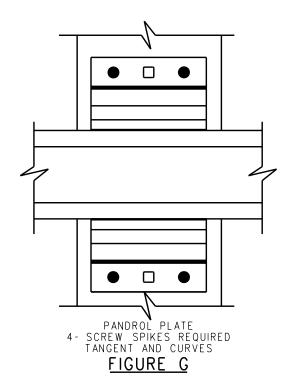


TANGENT AND CURVES TO 4° YARD AND INDUSTRY TRACK > 6° CURVES FIGURE H

#### NOTES:

- 1. TIE PLATE SPIKING FOR PLATES WITH HOLD-DOWN SPIKE HOLES
  - FIGURE A TANGENT AND CURVES TO 2°00' 4 SPIKES REQUIRED, 2 LINE AND
- FIGURE B CURVES 2°01' TO 4°00' INCLUSIVE 5 SPIKES REQUIRED, 3 LINE AND 2 HOLD-DOWN.
- FIGURE C CURVES OVER 4°00' 6 SPIKES REQUIRED, 4 LINE AND 2 HOLD-DOWN.
- 2. TIE PLATE SPIKING FOR PLATES WITHOUT HOLD-DOWN SPIKE HOLES.

   FIGURE D TANGENT TRACK WHERE THE MAXIMUM OPERATING SPEED DOES NOT EXCEED 25 MPH FOR FREIGHT AND 30 MPH FOR PASSENGER TRAINS, 2 LINE SPIKES REQUIRED.
- FIGURE E TANGENT AND CURVES TO 4°00' INCLUSIVE, 3 LINE SPIKES REQUIRED.
   FIGURE F CURVES OVER 4°00' 4 LINE SPIKES REQUIRED.
   TIE PLATE SPIKING FOR PANDROL TYPE FASTENING SYSTEMS FIGURE G,
- 4 SCREW SPIKES REQUIRED. 4. FIGURE H - THIS PATTERN TO BE USED ONLY ON EXISTING TRACK SO SPIKED.
- ANY VARIATIONS IN THE SPIKING PATTERNS ILLUSTRATED IN FIGURES A THRU F MUST BE APPROVED BY THE SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- 6. YARD AND INDUSTRY TRACK TO BE SPIKED WITH NOT LESS THAN TWO SPIKES TO EACH TIE PLATE.
- 7. REFER TO SCRRA ES2460-02 FOR "SP" PLATES.
  8. CUT SPIKES MAY BE USED ON PANDROL PLATE SQUARE HOLES FOR TEMPORARY ASSEMBLY OF TRACK. THEY WILL NOT BE REMOVED AFTER INSTALLATION OF SCREW



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					/
					$1 \qquad 1 \qquad 1 \qquad 2 \qquad $
					/ Vareh D. Vall
					ASSISIANT DIRECTOR: STANDARDS & DESIGN
Α	05/01/12	ADDED FIGURE H	AC	NDP	william Daran
REV.	DATE	DESCRIPTION	DES.	ENG.	DIRECTOR OF ENGINEERING AND CONSTRUCTION

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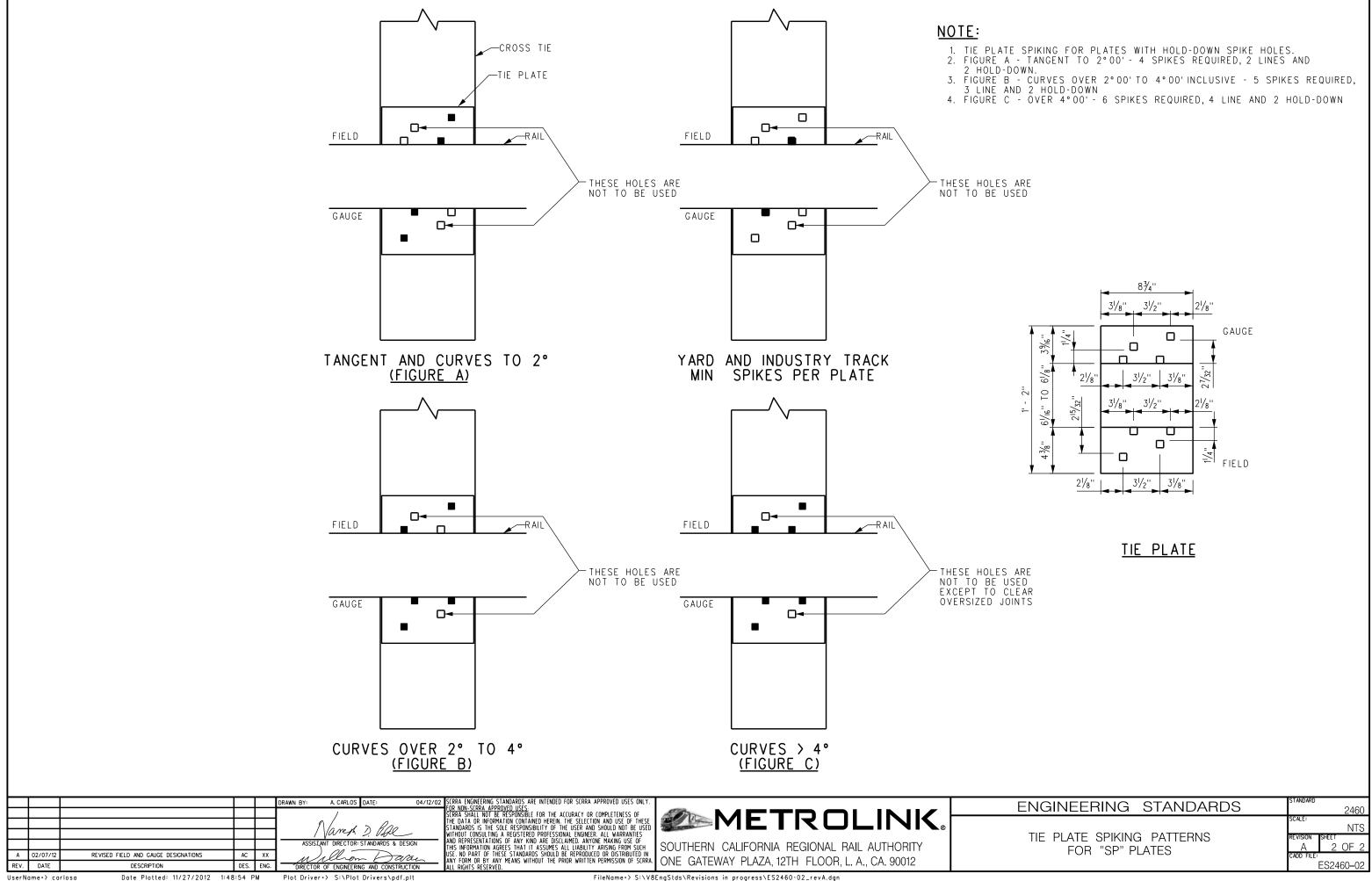
## **METROLINK**

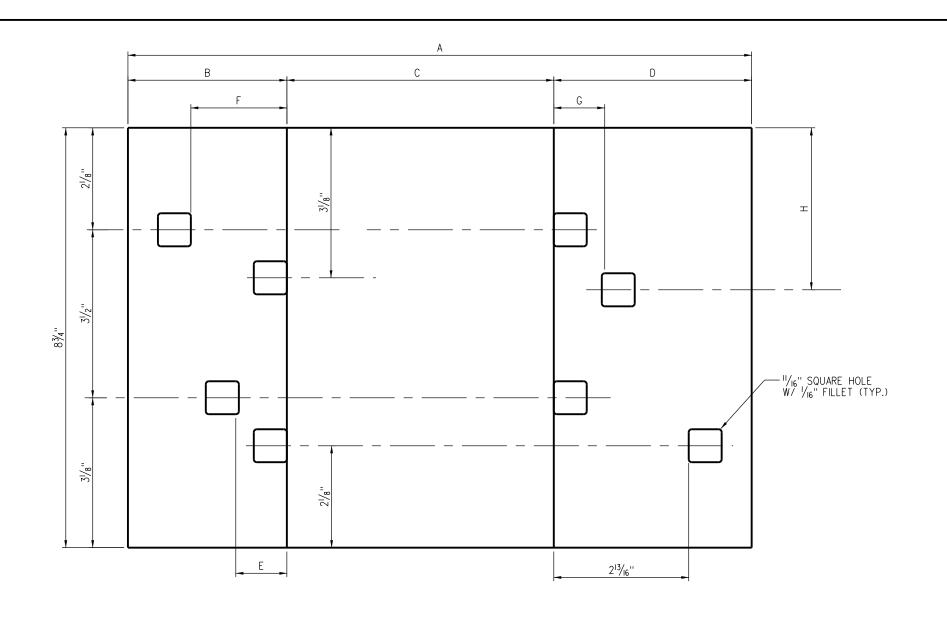
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ENGINEERING	STANDARDS

TIE PLATE SPIKING PATTERNS

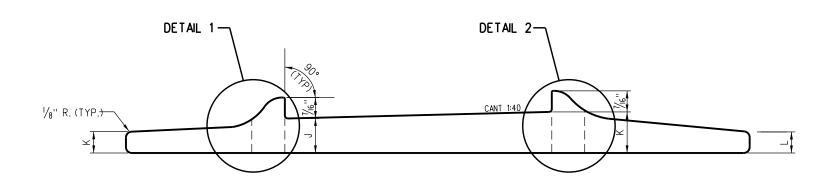
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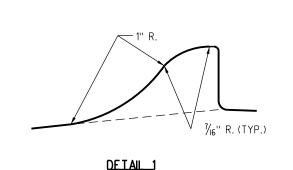


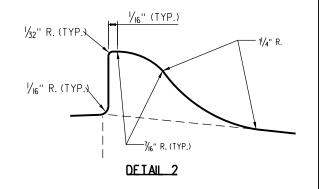


- 1. Manufacturer name, rail section, year and dimension base size to be rolled in raised letters and figures on the outside shoulder.
- Specifications and workmanship to be in accordance with current AREMA Manual requirements for hot-worked, high carbon steel tie plates.

	DIMENSIO	N TABLE	
RAIL BASE	51/2"	6"	6"
ITEM NO.	554-9010	554-9015	554-9020
Α	13"	14"	16"
В	35/16"	3%6"	3"/16"
С	5%"	6 <mark>1/16</mark> "	61/16"
D	41/8"	43/8"	6 <sup>1</sup> / <sub>4</sub> "
Ε	1"	11/4"	11/4"
F	2"	27/32"	27/32"
G	1"	11/4"	11/4"
Н	3¾"	31/8"	31/8"
J	23/32"	23/32"	<sup>3</sup> 1/ <sub>32</sub> ''
К	27/32"	7/8"	11/8''
L	<b>⅓</b> 6"	<b>⅓</b> 6"	3%"







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					/ Vareh D. Poll	FOR SCR THE STA WIT AND THIS
					ASSISIANT DIRECTOR: STANDARDS & DESIGN	ANL
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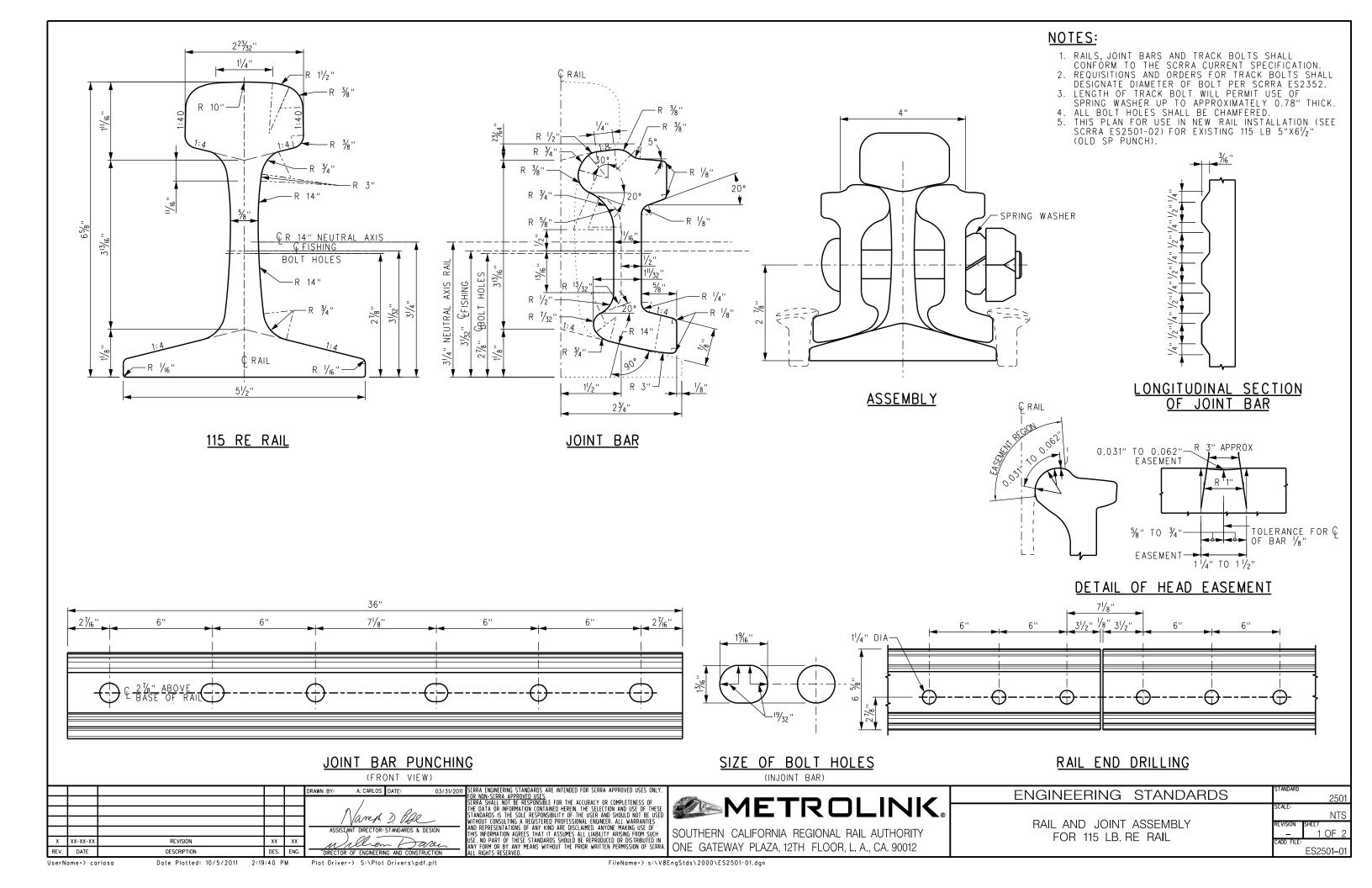
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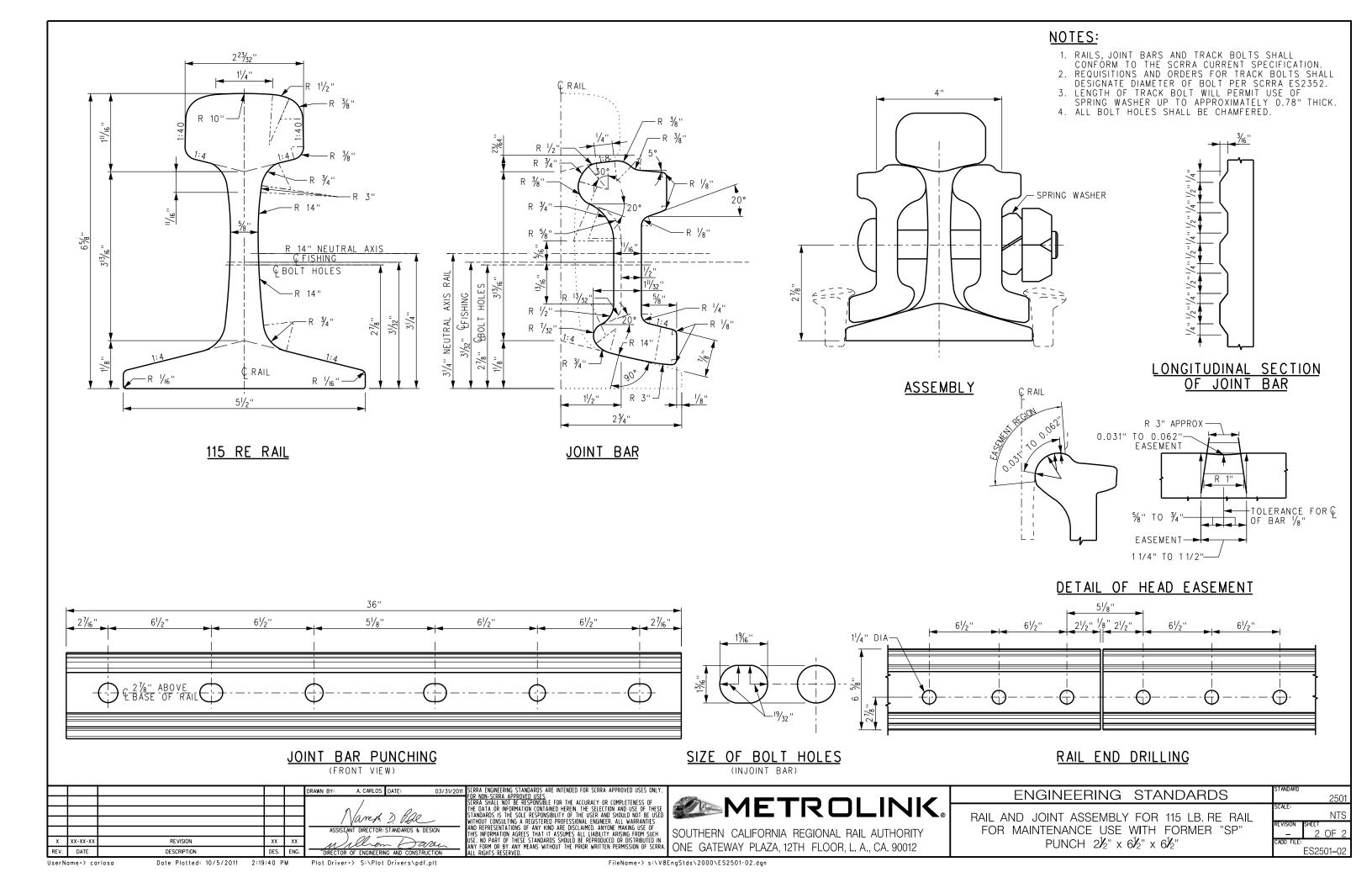
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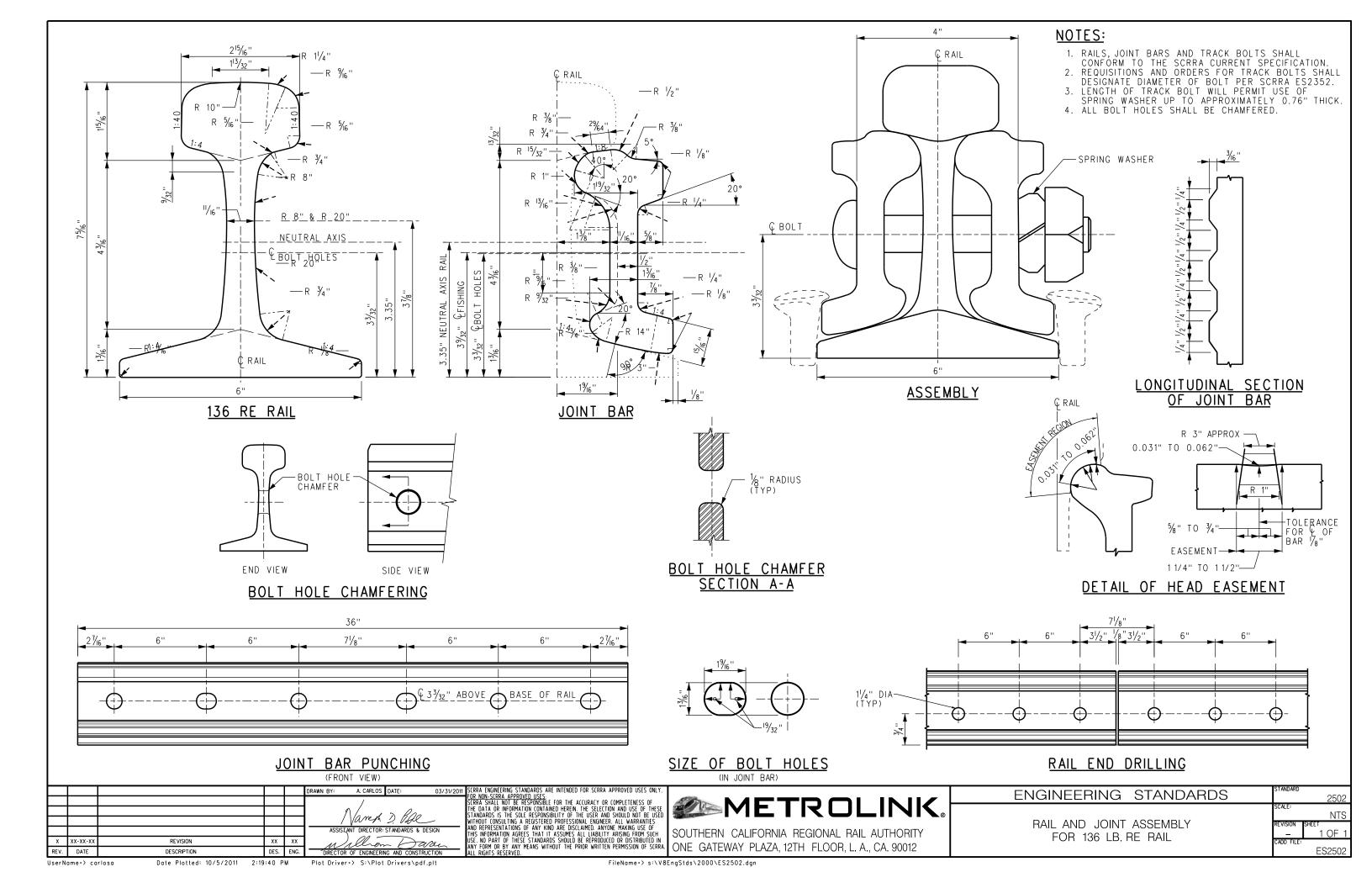
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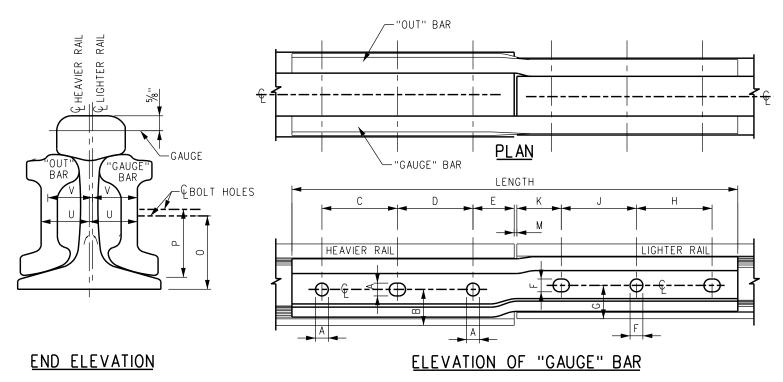
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DOUBLE SHO	JUL DER	TIF	PLATES	

DOUBLE SHOULDER TIE PLATES 51/2" AND 6" BASE RAIL









2. USE STANDARD JOINT BAR PER ES2501, MACHINED & LABELED TO INDICATE RAIL SIZE AND GAUGE AND FIELD SIDES. 3. USE STANDARD JOINT BAR PER ES2502, MACHINED & LABELED TO INDICATE RAIL SIZE AND GAUGE AND FIELD SIDES.

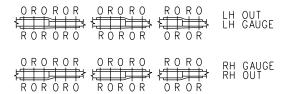
1. THIRD HOLE IN 110 LB RAIL TO BE DRILLED IN THE FIELD.

-HEAVIER RAIL (OUT) LIGHTER RAI (GAUGE) LEFT HAND JOINT (GAUGE) RIGHT HAND JOINT HEAVIER RAIL (OUT) LIGHTER RAIL-

IDENTIFICATION SKETCH

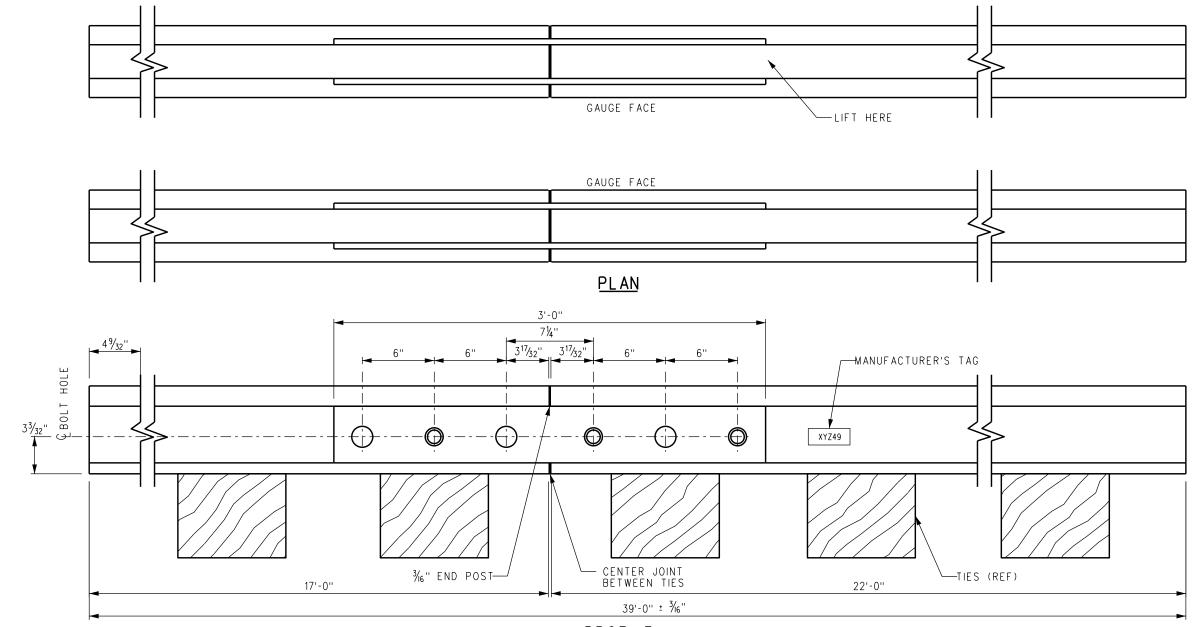
#### NOTES

- THIS PLAN SHOWS GENERAL INFORMATION FOR COMPROMISE JOINTS. SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION WILL FURNISH DETAIL PLANS FOR THE MANUFACTURER.
- 2. TO DETERMINE RIGHT HAND OR LEFT HAND JOINT: STAND BETWEEN
  RAILS IN THE TRACK, FACING RAILS TO BE JOINED. WHEN HEAVIER
  RAIL IS ON THE RIGHT HAND SIDE, IT IS A RIGHT HAND JOINT AND WHEN
  HEAVIER RAIL IS ON THE LEFT HAND SIDE, IT IS A LEFT HAND JOINT. ONE
  RIGHT HAND AND ONE LEFT HAND JOINT FORM A SET (FOUR BARS).
- 3. EACH BAR TO BE MARKED WITH THE FOLLOWING STAMPED IN DATA: SECTION OF RAIL. AT EACH END. "RH" OR "LH", FOR RIGHT HAND OR LEFT HAND,
  "GAUGE" OR "OUT", FOR GAUGE SIDE OR OUTSIDE,
- PATTERN NUMBER NAME OR TRADE MARK OF MANUFACTURER, YEAR MANUFACTURED.
- 4. ON ACCOUNT OF VARIOUS RAIL DRILLINGS FOR SECTIONS OTHER THAN SHOWN, REQUISITIONS AND ORDERS FOR COMPROMISE JOINTS FOR SUCH OTHER RAIL SHALL SHOW DIMENSIONS FOR B, F, J, K, AND O. FOR HEAVIER RAIL AND D, G, M AND P FOR THE LIGHTER RAIL
- BOLTS FOR COMPROMISE JOINTS ARE SAME AS FOR CORRESPONDING STANDARD JOINT BARS.
- 6. THE TYPE OF HOLES IN COMPROMISE BARS ARE AS SHOWN BELOW.
  "R" DENOTES ROUND HOLES AND "O" DENOTES OVAL HOLES.



COMPROMISE JOINT DIMENSIONS AND LENGTHS:																	
		HE A	VY.	А	В	С	D	E	LIG	SHT	F	G	Н	J	K		М
HEAVIER RAIL : LIGHTER RAIL	LENGTH	HEIGHT	BOLT DIA	OF HOLE IN BAR	RAIL DRILLING	TO CTD		RAIL END TO CTR 1st HOLE	R AIL HEIGHT	BOLT DIA	IN RAIL	BASE RAIL TO CL OF RAIL DRILLING	CTR 2nd TO CTR 3rd HOLE		RAIL END TO CTR 1st HOLE	AMOUNT OF WEAR	GAP BETWE RAIL EN
141 RE To 136 RE	36"	7 ½6''	11/8''	1 5/6"	3 <sup>3</sup> / <sub>32</sub> ''	6"	6''	31/2"	75/ <sub>16</sub> "	11/8''	1 5/6"	3¾32"	6"	6"	31/2"	-	/8"
141 RE To 132 RE	36"	7 ½6''	11/8''	1 5/6"	3 <sup>3</sup> / <sub>32</sub> ''	6''	6"	31/2"	71/8"	11/8''	1 5/6"	33/32"	6"	6"	31/2"	1/4"	1/8"
141 RE To 119 CFI	36"	7 ½6''	11/8''	1 5/16"	3 <sup>3</sup> / <sub>32</sub> ''	6''	6''	31/2"	6 <sup>13</sup> / <sub>16</sub> "	11/8''	1 5/6"	2 1/8"	6"	6"	31/2"	1/4"	1/8"
141 RE To 115 RE (MAINT ONLY)	36"	7 1/16"	11/8''	1 5/6"	33/32"	6''	6''	31/2"	65%''	11/8''	1 5/6"	2 1/8"	61/2"	61/2"	21/2"	1/4"	1/8"
141 RE To 115 RE	36"	7 1/16"	11/8''	1 5/6"	33/32"	6''	6''	31/2"	65/8"	11/8''	1 5/6"	2 1/8"	6"	6"	31/2"	1/4"	1/8"
136 RE To 132 RE	36"	75/6"	11/8''	1 5/6"	33/32"	6''	6''	31/2"	71/8"	11/8"	1 5/6"	33/32"	6"	6''	31/2"	1/4"	1/8"
136 RE To 119 CFI	36"	75/6"	11/8''	1 5/6"	33/32"	6''	6''	31/2"	6 <sup>13</sup> / <sub>16</sub> "	11/8"	1 5/6"	2 1/8"	6"	6''	31/2"	1/4"	1/8"
136 RE To 115 RE (MAINT ONLY)	36"	75/6"	11/8''	1 5/16"	33/32"	6"	6''	31/2"	65%''	11/8''	1 5/16"	2 1/8"	61/2"	61/2"	21/2"	1/4"	1/8"
136 RE To 115 RE	36"	75/16"	11/8''	1 5/6"	3 <sup>3</sup> / <sub>32</sub> ''	6''	6''	31/2"	6%"	11/8"	1 5/6"	2 1/8"	6"	6''	31/2"	1/4"	1/8"
132 RE To 119 CFI	36"	71/8"	11/8''	1 5/6"	33/32"	61/2"	61/2"	21/2"	6 <sup>13</sup> / <sub>16</sub> ''	11/8''	1 5/6"	2 1/8"	6''	6''	31/2"		1/8"
132 RE To 115 RE	36"	71/8"	11/8"	1 5/6"	33/32"	6"	6"	31/2"	65/8"	11/8"	1 5/6"	2 1/8"	6"	6"	31/2"		1/8"
132 RE To 115 RE (MAINT ONLY)	36"	71/8"	11/8"	1 5/6"	33/32"	61/2"	61/2"	21/2"	6%"	11/8"	1 5/6"	2 1/8"	61/2"	61/2"	21/2"		1/8"
119 RE To 115 RE	36"	6 <sup>13</sup> / <sub>16</sub> "	11/8''	1 5/6''	2 1/8"	6''	6''	31/2"	65/8"	11/8"	1 5/6"	2 1/8"	61/2"	61/2"	21/2"		1/8"
115 RE To 110 RE (MAINT ONLY)	30"	65/8"	11/8''	1 5/6"	2 1/8"	61/2"	61/2"	21/2"	61/4"	1"	1 3/6"	2%"		51/2"	211/16"		1/8"
115 RE To 110 RE	30"	65/8"	11/8"	1 5/16"	2 7/8"	6"	6"	31/2"	61/4"	1"	1 3/6"	245/64"		5"	215/32"		1/8"
115 RE To 100 RA	30"	65/8"	11/8"	1 5/6"	2 1/8"	6"	6"	31/2"	6''	1"	1 3/6"	25/8"		5"	215/32"		1/8"
115 RE To 100 RE	30"	65/8"	11/8''	1 5/6"	2 1/8"	6''	6''	31/2"	6''	1"	1 3/6"	211/16"		51/2"	211/16"		1/8"
110 RE To 90 RA	24"	61/4"	1"	1 3/6"	25/8"		51/2"	211/16"	5%"	1"	1 3/6"	237/64"		5"	213/32"		5/32''
110 RE To 85	24"	61/4"	1"	1 3/6"	25/8"		51/2"	211/16"	5 3/8"	1"	1 3/6"	229/64"		7"	2 1/16"		5/32'
90 RA To 85	24"	55/8''	1"	1 3/6"	2 <sup>13</sup> / <sub>16</sub> "		5"	213/32"	53/8"	1"	1 3/6"	229/64"		5"	215/32"		3/16"

				DRAWN BY: A. CARLOS DATE: 03/31/2	OTT SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY.  FOR NON-SCRRA APPROVED USES:  SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF	A AETROLINIA	ENGINEERING STANDARDS	STANDARD 2503
				Name 2 Rae	THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE STANDARDS IS THE SOLE RESPONSIBILITY OF THE USER AND SHOULD NOT BE USED	METROLINK		scale: NTS
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X XX	-XX-XX	REVISION DESCRIPTION	DES. ENG	william Daran	USE. NO PART OF THESE STANDARDS SHOULD BE REPRODUCED OR DISTRIBUTED IN  ANY FORM OR BY ANY MEANS WITHOUT THE PRIOR WRITTEN PERMISSION OF SCRRA.	ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012	VARIOUS WEIGHTS OF RAILS	CADD FILE: ES2503
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#### PROFILE

#### NOTES:

- 1. INSULATED JOINT PLUG SHALL MEET OR EXCEED CURRENT AREMA SPECIFICATION CHAPTER 4, PART 3. ONLY ALLEGHENY BONDED INSULATED JOINT OR APPROVED EQUAL WILL BE ACCEPTED.

  2. INSULATED JOINT PLUGS SHALL BE MANUFACTURED FROM NEW HEAD HARDENED RAIL. INSULATED JOINTS SHALL BE INSTALLED AS SHOWN IN PLANS OR AS DIRECTED. GOOD USABLE SECOND HAND HEAD HARDENED RAIL WITH 1/4" HEADLOSS MAY BE USED FOR JOINTS MANUFACTURED FOR 1/4" HEADWEAR. INSULATED JOINTS FOR USE IN TÜRNOUTS, RAIL WILL BE BENT FOR CLOSURE OR TURNOUT SIDE.

- TURNOUT SIDE.

  ALL HOLES SHALL BE CHAMFERED.

  1" A490 HUCK BOLTS WITH STAGGERED PATTERN SHALL BE FURNISHED.

  WHEN NECESSARY, 1 1/8" GRADE 8 BOLTS WITH SECURITY LOCKNUTS, LUBRICATED AND TORQUED TO 850 FT LBS, MAY BE SUBSTITUTED FOR HUCK BOLTS.

  INSULATED JOINT PLUGS TO BE MANUFACTURED AND CURED IN A CONTROLLED ENVIRONMENT AT THE MANUFACTURER'S PLANT. NO FABRICATION OF INSULATED JOINT PLUGS IN THE FIELD WILL BE ACCEPTED. AFTER HUCKING OR BOLTING, MANUFACTURER SHALL REMOVE EXCESS EPOXY FROM RAIL AND JOINT BAR MANUFACTURER SHALL ADHERE IDENTIFICATION TAG TO THE WEB FROM RAIL AND JOINT BAR. MANUFACTURER SHALL ADHERE IDENTIFICATION TAG TO THE WEB OF RAIL DEPICTING MANUFACTURER'S NAME, CONTROL NUMBER, LOCATION, MONTH (01) AND YEAR (2XXX) WHERE JOINTS WERE FABRICATED.

#### NOTES: (CONT)

- MANUFACTURER SHALL MARK A BALANCE POINT ON THE HEAD OF RAIL FOR HANDLING. INSULATED JOINT PLUGS SHALL BE CENTERED BETWEEN TIE CRIBS WHEN INSTALLED. SUPPLIERS OF MATERIAL SHOWN ON TRACK STANDARD DRAWINGS SHALL FORMALLY SUBMIT THEIR SHOP DRAWINGS TO SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION FOR APPROVAL. MATERIAL SHIPPED WITHOUT WRITTEN APPROVAL FROM SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION WILL NOT BE ACCEPTED.

  PREFABRICATED JOINTS OF OTHER LENGTHS AS SPECIFIED MAY BE REQUIRED IN TURNOUTS.

  ONLY TOELESS JOINT BARS ARE TO BE USED FASTENED WITH SHAVED E-CLIPS FOR INSULATED JOINTS. SEE SCRRA ES2361.

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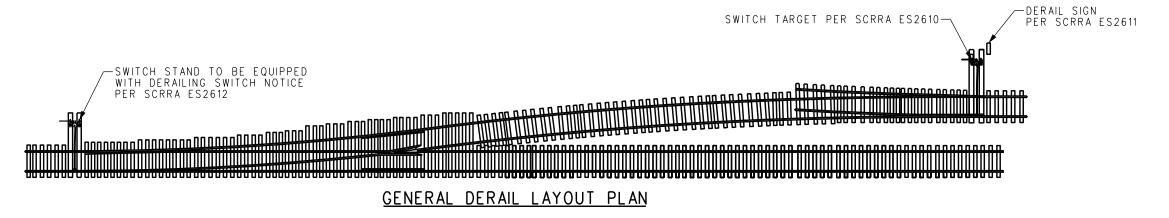
PREFABRICATED	BONDED	INSULATED	JOINT

**ENGINEERING STANDARDS** 

NTS 1 OF

ES2504

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SEE SCRRA ES2702 FOR DISTANCE FROM MAIN TRACK SWITCH TO POINT OF DERAIL AND INSULATED JOINT LOCATIONS.

#### REQUIRED DERAIL NOTES:

- 1. EXCEPT AT THE INTERLOCKINGS, DERAILS ARE REQUIRED AT THE FOLLOWING LOCATIONS UNLESS OTHERWISE AUTHORIZED.

  a. INTERCHANGE TRACKS, REGARDLESS OF GRADE CONDITIONS, WHERE THERE IS HAZARD OF FOREIGN LINE OPERATION CAUSING ENGINES OR CARS TO MOVE FOUL OF MAIN TRACK, SIDING OR OTHER TRACKS.

  b. INDUSTRY TRACKS, WHERE AN INDUSTRY CAN MOVE CARS TO CREATE A HAZARD BY FOULING THE MAIN TRACK, SIDING; OR ANY INDUSTRY TRACKS OR OTHER TRACKS WHERE CARS ARE LEFT UNATTENDED.

  c. SPURS AND OTHER TRACKS ON WHICH CARS ARE LEFT UNATTENDED AND THE UNAUTHORIZED MOVEMENT OF SUCH CARS MAY FOUL MAIN TRACK OR SIDING, EXCEPT WHERE TRACK GRADE ASCENDS

  TOWARD MAIN TRACK OR SIDING AT GREATER THAN OR EQUAL TO 1.50% OR ADDRESS OF CRADE THAT IS USED FOR THE STORAGE OF LIVE FNICINES AND WHERE AN UNAUTHORIZED MOVEMENT OF THE ENGINES COULD FOUL MAIN TRACK.
  - d. ANY TRACK, REGARDLESS OF GRADE. THAT IS USED FOR THE STORAGE OF LIVE ENGINES AND WHERE AN UNAUTHORIZED MOVEMENT OF THE ENGINES COULD FOUL MAIN TRACK

  - e. OTHER LOCATIONS, REGARDLESS OF GRADE, WHERE SPECIAL CONDITIONS REQUIRE DERAIL PROTECTION AND SUCH PROTECTION IS AUTHORIZED BY SCRRA.

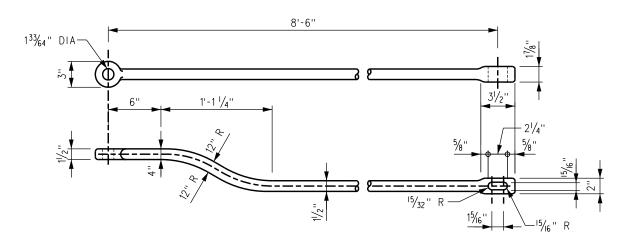
    f. ANY TRACK, USED FOR LOADING, UNLOADING OR STORAGE OF CARS CONTAINING HAZARDOUS MATERIAL AS LISTED IN THE HAZARDOUS MATERIALS REGULATIONS OF THE DEPARTMENT OF TRANSPORTATION, CODE OF FEDERAL REGULATIONS. SUCH OPERATIONS SHALL BE PROTECTED AGAINST INBOUND MOVEMENTS BY DERAILS, SECURED WITH LOCKS AND LOCATED BEYOND THE CLEARANCE POINT AND NOT LESS THAN 50 FEET FROM NEAR END OF THE CAR(S).
- 2. ALL NEW INSTALLATIONS OF DERAILS AS OUTLINED ABOVE SHALL BE THE DOUBLE SWITCH POINT TYPE SCRRA ES2604. EXISTING SLIDING OR HINGED TYPE DERAILS CURRENYLY APPLIED ARE AUTHORIZED
  - a. ON INSIDE OF CURVES OVER 5 DEGREES.

  - b. ON TRACKS WHERE AN UNCONTROLLED CAR COULD REACH A SPEED IN EXCESS OF FOUR (4) MPH.
    c. AT LOCATIONS WHERE A DERAIL IS INSTALLED TO PROTECT AGAINST THE MOVEMENT OF ENGINES OR TRAINS. SLIDING OR HINGED DERAILS ARE AUTHORIZED FOR PROTECTION AT LOCOMOTIVE AND CAR REPAIR FACILITIES WHEN ALSO PROTECED BY BLUE FLAG RULES AND PROCEDURES.
- d. AT ANY OTHER LOCATION WHERE CONDITIONS ARE SUCH THAT THE SWITCH POINT DERAIL SHOULD BE INSTALLED TO ELIMINATE A POTENTIALLY HAZARDOUS SITUATION 3. DOUBLE POINT DERAILS PER SCRRA ES2604 ARE REQUIRED AS NOTED BELOW. AT OTHER LOCATIONS REQUIRING A DERAIL, A SLIDING OR HINGED DERAIL, SCRRA ES2613 OR ES2614 WILL BE USED.

  D. LOCATIONS WHERE UNCONTROLLED MOVEMENTS CAN EXCEED 20 MPH.

  D. LOCATIONS PROTECTING TRACKS HOLDING 15 OR MORE CARS.
- c. DIVERGING TRACK ASCENDS TOWARDS MAIN TRACK AT GRADE LESS THAN 0.5% OR DESCENDS TOWARD THE MAIN TRACK AT ANY GRADIENT.
- d. AT OTHER LOCATIONS DESIGNATED BY SCRRA DIRECTOR OF ENGINEERING AND CONSTRUCTION.
- FOR DETAILS OF CONNECTING RODS FOR SLIDING AND HINGED DERAILS SEE SCRRA ES2602.
- 5. SEE SCRRA ES2610, ES2611 AND ES2612 FOR DERAIL SIGNAGE WHERE REQUIRED.
  6. EXPOSED ENDS OF STOCK RAIL AND DEFLECTING RAILS SHALL BE CUT AND BENT PER DEPRESSED RAIL HEAD DETAIL PER SCRRA ES2604.
  7. HAND OPERATED DERAILS ARE ILLUSTRATED, HOWEVER POWER OPERATED DERAILS WILL BE INSTALLED AS DIRECTED BY SCRRA.

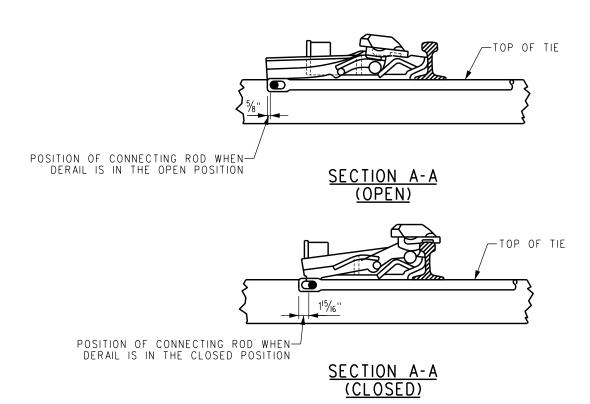
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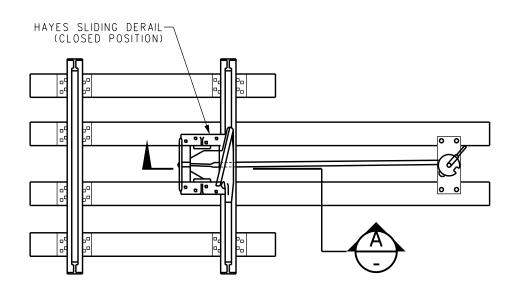


#### CONNECTING ROD FOR USE WITH HAYES DERAIL AND HIGH OR LOW SWITCH STANDS PER SCRRA ES2701 & ES2704

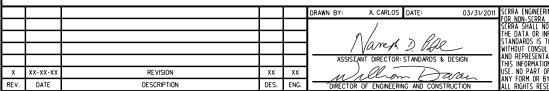
#### NOTE:

SINCE THE THROW OF SWITCH STAND IS ONLY 5", THE SLOTTED HOLE IN ROD IS PROVIDED TO PERMIT MOVEMENT OF  $6\frac{1}{4}$ " REQUIRED FOR PROPER FUNCTIONING OF HAYES SLIDING DERAIL.





GENERAL PLAN FOR CONNECTING ROD WITH HAYES SLIDING DERAIL



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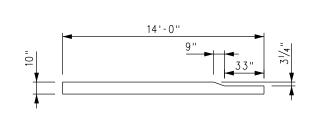
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ENGINEERING STANDARDS	STANDARD 2602
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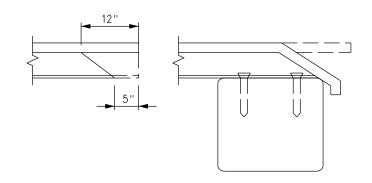
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DAPPING DETAIL - HEADBLOCK TIES

(8"x10"x14'-0")



DEPRESSED RAIL HEAD DETAIL

#### TURNOUT DATA

16'-6" SWITCH GEOMETRY: 9 1/<sub>32</sub> " 1° 4 4 ' 11" VERTEX DISTANCE: SWITCH ANGLE: SWITCH HEEL SPREAD: RADIUS OF CL CURVE: DEGREE OF CL CURVE: CENTRAL ANGLE OF TURNOUT CURVE: 3° 59'18"

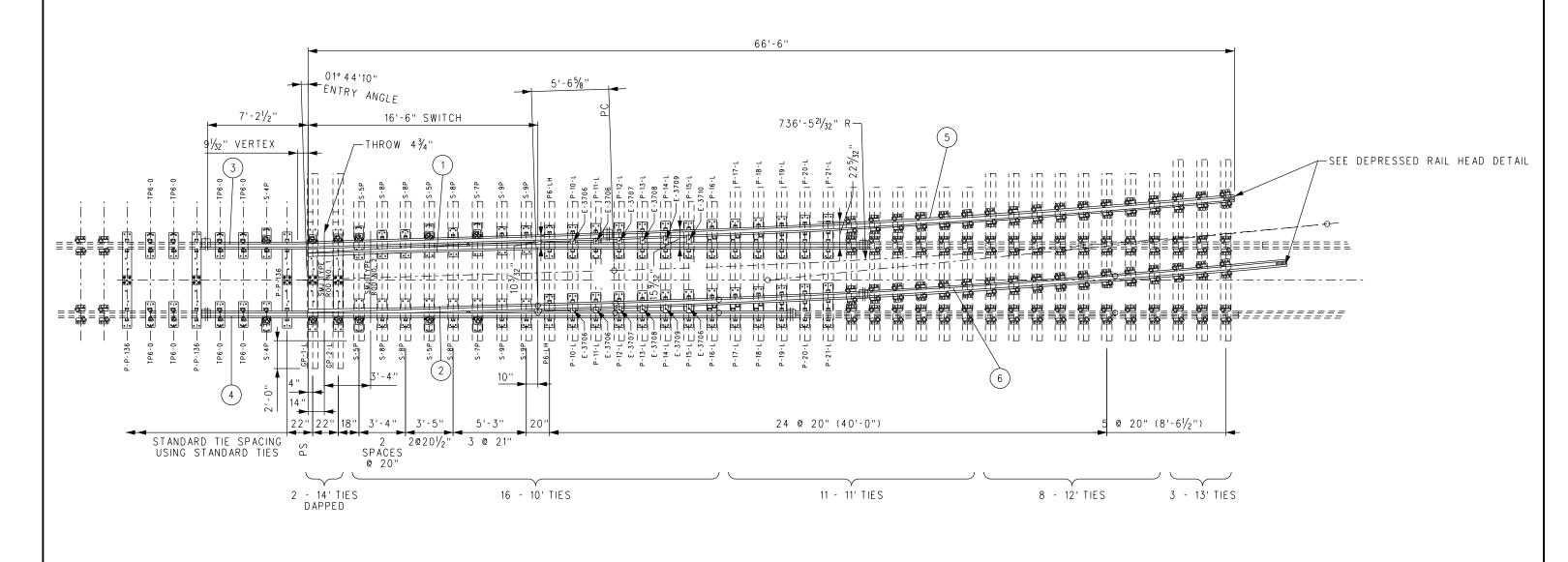
#### **LEGEND**

WELDED JOINTS #

#### NOTES:

- 1. SEE ES2604-02 FOR BILL OF MATERIALS. CIRCLED ITEM NUMBERS APPLY TO BILL OF MATERIAL ITEMS.
- 2. ALL RAIL SHALL HAVE IDENTIFICATION COLOR CODE PAINTED ON WEB CLEAR OF JOINT AREA.

  3. LH AND RH SWITCH POINTS WITH
- MANGANESE TIP.
- 4. TIMBER TIES TO CONFORM TO SCRRA STANDARD SPECIFICATIONS 34 11 34. 5. RH SWITCH POINT DERAIL IS MIRROR IMAGE OF THIS LAYOUT. SEE BILL OF MATERIALS FOR REFERENCE TO SPECIFIC PARTS.



Jares D. Page DIRECTOR: STANDARDS & DESIGN x xx-xx-xx REVISION REV. DATE DESCRIPTION DES. ENG. Plot Driver=> S:\Plot Drivers\pdf.plt

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ALL PIGHTS PERSEVET.

**METROLINK** 

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

**ENGINEERING STANDARDS** 16'-6" DOUBLE POINT DERAIL

(LEFT HAND SHOWN)

NTS 1 OF ES2604-01

BILL OF MATERIAL						BILL OF MATERIAL					
ITEM	LH QTY	RH QTY	DESCRIPTION	DWG NO	SCRRA PART NO	ITEM	LH QTY	RH QTY	DESCRIPTION	DWG NO	SCRRA PART NO
1	1	1	SAMSON POINT, 16'-6"/40'-0" LONG, FLOATING HEEL, MANGANESE TIP, LH	ES2921-08		33	2	-	SWITCH PLATE (P-15-L)	ES2921-13	
2	1	1	SAMSON POINT, 16'-6"/40'-0" LONG, FLOATING HEEL, MANGANESE TIP, RH	ES2921-08		34	-	2	SWITCH PLATE (P-15-R)	ES2921-13	
3	1	-	STOCK RAIL, SAMSON UNDERCUT, BENT & CURVED, 28'-10" LONG, LH/LHTO	ES2921-09		35	2	-	SWITCH PLATE (P-16-L)	ES2921-13	
3	-	1	STOCK RAIL, SAMSON UNDERCUT, BENT & CURVED, 28'-10" LONG, RH/RHTO	ES2921-09		36	-	2	SWITCH PLATE (P-16-R)	ES2921-13	
4	1	-	STOCK RAIL, SAMSON UNDERCUT, STRAIGHT, 42'-0" LONG, RH/LHTO	ES2921-09		37	2	-	SWITCH PLATE (P-17-L)	ES2921-13	
4	-	1	STOCK RAIL, SAMSON UNDERCUT, STRAIGHT, 42'-0" LONG, LH/RHTO	ES2921-09		38	-	2	SWITCH PLATE (P-17-R)	ES2921-13	
5	1	1	CURVED RAIL 44'-11%6" LONG	i		39	2	-	SWITCH PLATE (P-18-L)	ES2921-13	
6	1	1	CURVED RAIL 30'-41/8" LONG	i		40	-	2	SWITCH PLATE (P-18-R)	ES2921-13	
9	1	1	SWITCH ROD *1 ASSEMBLY, 'SMJ' VERTICAL C/W BASKET ASSEMBLY	i		41	2	-	SWITCH PLATE (P-19-L)	ES2921-13	
10	1	1	SWITCH ROD *2 ASSEMBLY, 'SMJ' VERTICAL	-		42	-	2	SWITCH PLATE (P-19-R)	ES2921-13	
11	3	3	GAUGE PLATE, INSULATED (P-P-136)	ES2802-80		43	2	-	SWITCH PLATE (P-20-L)	ES2921-13	
12	1	-	GAUGE PLATE, INS (GP-1-L)	ES2802-81		4 4	-	2	SWITCH PLATE (P-20-R)	ES2921-13	
13	-	1	GAUGE PLATE, INS (GP-1-R)	ES2802-81		45	2	-	SWITCH PLATE (P-21-L)	ES2921-13	
14	1	-	GAUGE PLATE, INS (GP-2-L)	ES2802-82		46	-	2	SWITCH PLATE (P-21-R)	ES2921-13	
15	-	1	GAUGE PLATE, INS (GP-2-R)	ES2802-82		4 7	72	72	TIE PLATE, ROLLED PANDROL, 6" RAIL BASE, CANTED, 1" DIA HOLES	ES2454	
16	2	2	BRACE SLIDE PLATE (S-4P)	ES2802-88		48	4	4	HOLD-DOWN CLIP (E3706)	-	
17	4	4	BRACE SLIDE PLATE (S-5P)	ES2802-85		49	2	2	HOLD-DOWN CLIP (E3707)	-	
18	2	2	BRACE SLIDE PLATE (S-7P)	ES2802-85		50	2	2	HOLD-DOWN CLIP (E3708)	-	
19	2	-	SWITCH HEEL PLATE (P6-LH)	ES2802-83		51	2	2	HOLD-DOWN CLIP (E3709)	-	
20	-	2	SWITCH HEEL PLATE (P6-RH)	ES2802-84		52	2	2	HOLD-DOWN CLIP (E3710)	-	
21	6	6	SLIDE PLATE (S-8P), 1/4" RISER	ES2802-86		53	264	264	PANDROL SPRING CLIP (E2055)	ES2362	
22	4	4	SLIDE PLATE (S-9P), O" RISER	ES2802-86		54	8	8	PANDROL SPRING CLIP, (E2063), FOR JOINT BARS	ES2361	
23	2	-	SWITCH PLATE (P-10-L)	ES2921-13		55	536	536	SCREW SPIKE, 15/16" DIA X 6" LONG	ES2355	
24	-	2	SWITCH PLATE (P-10-R)	ES2921-13		56	12	12	BOLTLESS BRACE, 136RE 'SURFIT'	-	
25	2	-	SWITCH PLATE (P-11-L)	ES2921-13		57	12	12	SERRATED WASHER FOR BOLTLESS BRACE	-	
26	-	2	SWITCH PLATE (P-11-R)	ES2921-13		58	16	16	TIE, HARDWOOD, TREATED, 7" X 9" X 10'-0" LONG	-	
27	2	-	SWITCH PLATE (P-12-L)	ES2921-13		59	11	11	TIE, HARDWOOD, TREATED, 7" X 9" X 11'-0" LONG	-	
28	-	2	SWITCH PLATE (P-12-R)	ES2921-13		60	8	8	TIE, HARDWOOD, TREATED, 7" X 9" X 12'-0" LONG	-	
29	2	-	SWITCH PLATE (P-13-L)	ES2921-13		61	3	3	TIE, HARDWOOD, TREATED, 7" X 9" X 13'-0" LONG	-	
30	-	2	SWITCH PLATE (P-13-R)	ES2921-13		62	2	2	TIE, HARDWOOD, TREATED, 10" X 8" X 14'-0" LONG, DAPPED	-	
31	2	-	SWITCH PLATE (P-14-L)	ES2921-13		63	8	8	TIE PLATE, TP6-0, NO CANT	-	
32	-	2	SWITCH PLATE (P-14-R)	ES2921-13							

Х	xx-xx-xx	REVISION	XX	ХX
REV.	DATE	DESCRIPTION	DES.	ENG.

ASSISTANT DIRECTOR: STANDARDS & DESIGN William Davar
DIRECTOR OF ENGINEERING AND CONSTRUCTION

O3/31/2011

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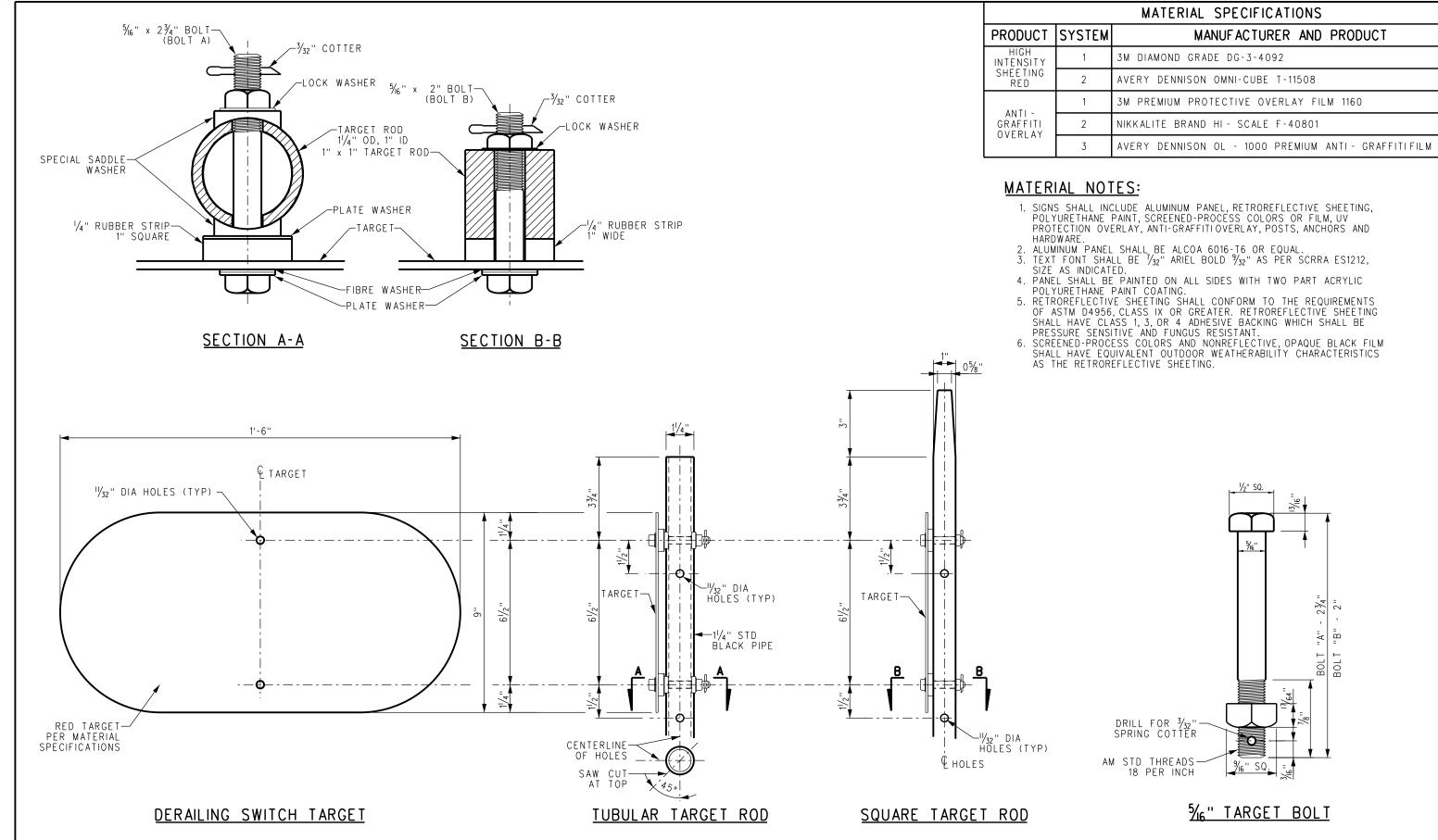
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ENGINEERING	STANDARDS

16'-6" DOUBLE POINT DERAIL BILL OF MATERIAL LH AND RH

NTS CADD FILE: ES2604-02



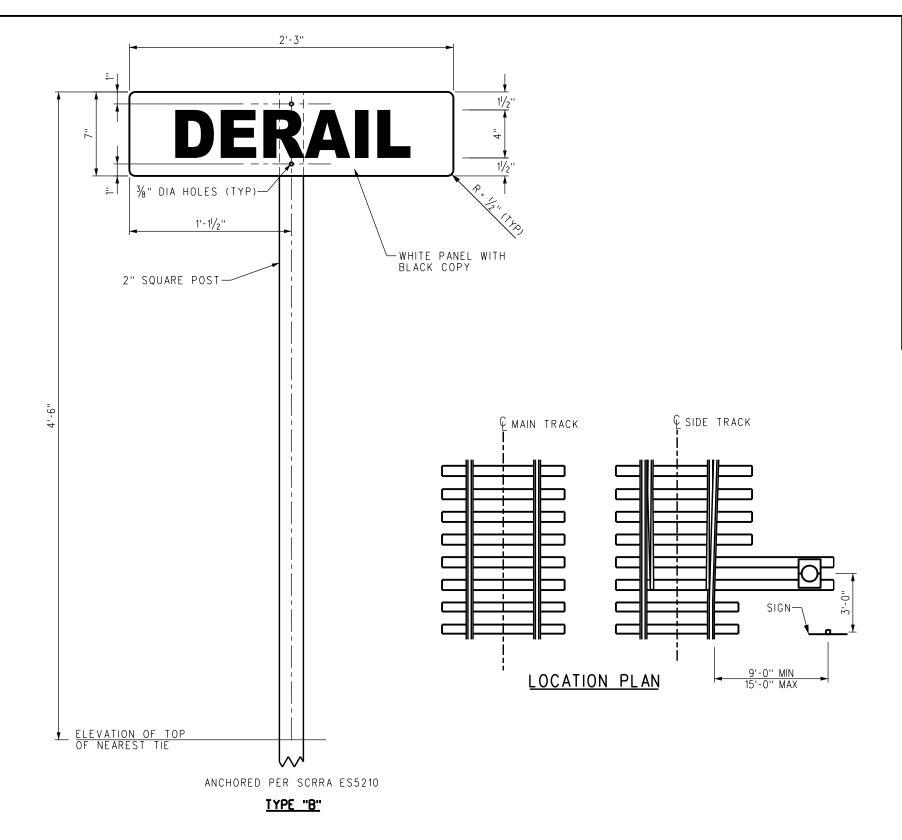
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					ASSISIANT DIRECTOR: STANDARDS & DESIGN
Α	3-22-13	REVISED MATERIAL SPECIFICATIONS	AC	NDP	william Daran
RFV	DATE	DESCRIPTION	DES	FNG	DIRECTOR OF ENCINEERING AND CONSTRUCTION

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### **METROLINK**

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS	STANDARD 2610
	SCALE:  NTS  REVISION SHEET
DERAILING SWITCH TARGET	A 1 OF 1
	CADD FILE: ES2610



MATERIAL SPECIFICATIONS PRODUCT SYSTEM MANUFACTURER AND PRODUCT 3M SCOTCHLITE HIGH INTENSITY PRISMATIC WHITE GRADE 3930 SHEETING HIGH INTENSITY NIPPON CARBIDE RETRO-REFLECTIVE SHEETING TYPE VIII CRYSTAL GRADE SHEETING (WHITE) AVERY DENNISON OMNI-VIEW T-9500 PRISMATIC HIGH INTENSITY SHEETING 3M PROCESS COLOR SERIES 8851 INK COPY GRAPHICS NIPPON CARBIDE GRAFFITI RESISTANT 3803 INK (BLACK) AVERY DENNISON 4930 INK 3M PREMIUM PROTECTIVE OVERLAY FILM 1160 ANTI-GRAFFITI NIKKALITE BRAND HI - SCALE F-40801 OVERLAY AVERY DENNISON OL - 1000 PREMIUM ANTI - GRAFFITIFILM 3 PANEL  $\frac{1}{8}$ " THICK ALUMINUM, ALCOA 6016-T6 OR EQUAL POSTS, **ANCHORS** AS PER SCRRA ES5210 HARDWARE

#### INSTALLATION NOTES

TYPE "B" DERAIL SIGN SHALL BE USED AT ALL DERAILS PER SCRRA ES2601. SIGN SHALL BE LOCATED AS PER LOCATION PLAN AND FACING SO AS TO BE READ FROM ENGINE PULLING OUT OF THE SIDE TRACK. SELECT OFFSET FROM FIELD SIDE OF NEAREST RAIL SUCH THAT UNDERGROUND UTILITIES SHALL NOT BE DAMAGED WHEN SETTING ANCHOR.

#### MATERIAL NOTES:

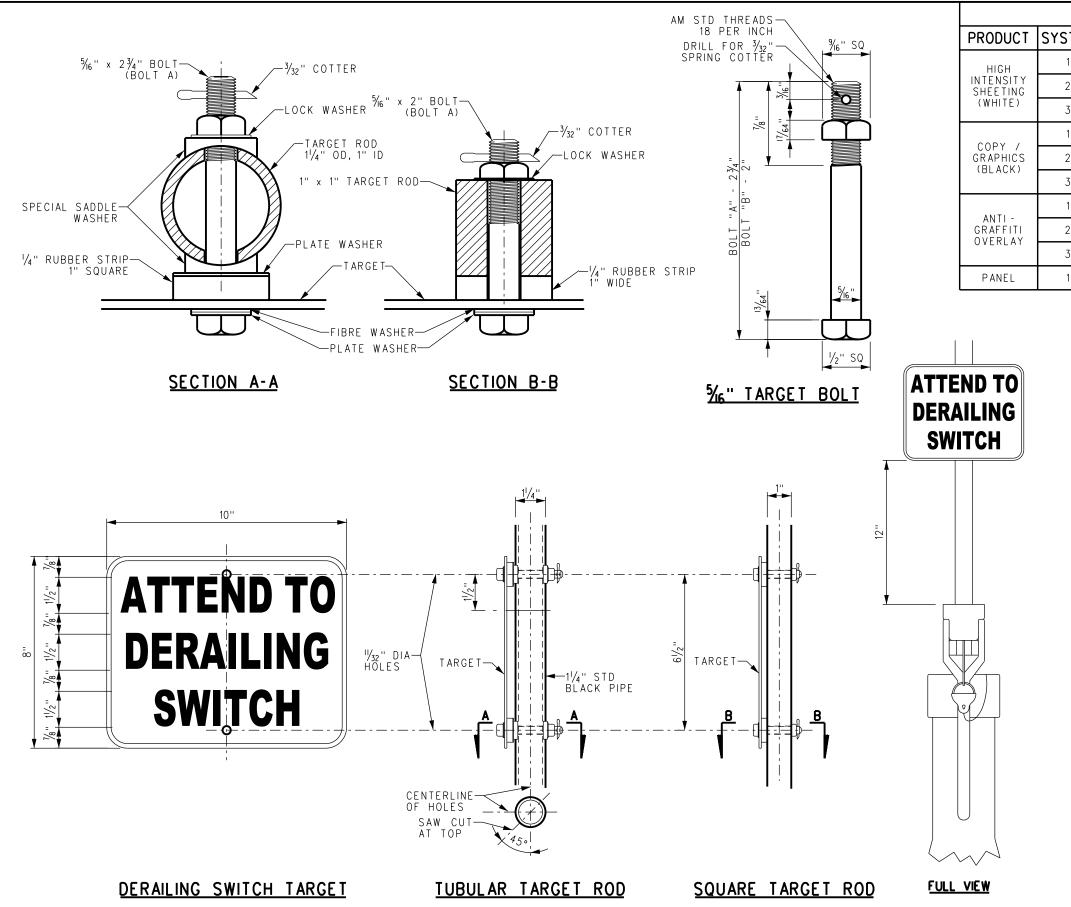
- 1. SIGNS SHALL INCLUDE ALUMINUM PANEL, RETROREFLECTIVE SHEETING, POLYURETHANE PAINT, SCREENED-PROCESS COLORS OR FILM, UV PROTECTION OVERLAY, ANTI-GRAFFITI OVERLAY, POSTS, ANCHORS AND
- 2. ALUMINUM PANEL SHALL BE ALCOA 6016-T6 OR EQUAL.
  3. TEXT FONT SHALL BE 1/32" ARIEL BOLD 1/32" AS PER SCRRA ES1212, SIZE AS INDICATED.
- POSTS, ANCHORS, AND HARDWARE SHALL BE AS PER SCRRA ES5210.
- 5. PANEL SHALL BE PAINTED ON ALL SIDES WITH TWO PART ACRYLIC POLYURETHANE PAINT COATING.
- 6. RETROREFLECTIVE SHEETING SHALL CONFORM TO THE REQUIREMENTS OF ASTM D4956, CLASS IX OR GREATER. RETROREFLECTIVE SHEETING SHALL HAVE CLASS 1, 3, OR 4 ADHESIVE BACKING WHICH SHALL BE PRESSURE SENSITIVE AND FUNGUS RESISTANT.
- 7. SCREENED-PROCESS COLORS AND NONREFLECTIVE OPAQUE BLACK FILM SHALL HAVE EQUIVALENT OUTDOOR WEATHERABILITY CHARACTERISTICS AS THE RETROREFLECTIVE SHEETING.

22 SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY. FOR NON-SCRRA APPROVED USES: SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF THE DATA OR INFORMATION CONTAINED HEREIN. THE SELECTION AND USE OF THESE STANDARDS IS THE SOLE RESPONSIBILITY OF THE USER AND SHOULD NOT BE USED WITHOUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER. ALL WARRANTIES	<b>METROLINK</b>
<ul> <li>AND REPRESENTATIONS OF ANY KIND ARE DISCLAIMED. ANYONE MAKING USE OF THIS INFORMATION AGREES THAT IT ASSUMES ALL LABBULTY ARISING FROM SUCH USS. NO PART OF THESE STANDARDS SHOULD BE REPRODUCED OR DISTRIBUTED IN ANY FORM OR BY ANY MEANS WITHOUT THE PRIOR WRITTEN PERMISSION OF SCRRA. ALL RIGHTS RESERVED.</li> </ul>	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012

**ENGINEERING STANDARDS** NTS TYPE "B" DERAIL SIGN 1 OF

ES2611

TYPE "B" SIGNS CONFORM TO CALIFORNIA STATE LAW



MATERIAL SPECIFICATIONS PRODUCT SYSTEM MANUFACTURER AND PRODUCT 3M SCOTCHLITE HIGH INTENSITY PRISMATIC WHITE GRADE 3930 SHEETING NIPPON CARBIDE RETRO-REFLECTIVE SHEETING TYPE VIII CRYSTAL GRADE 3 AVERY DENNISON OMNI-VIEW T-9500 PRISMATIC HIGH INTENSITY SHEETING 3M PROCESS COLOR SERIES 8851 INK NIPPON CARBIDE GRAFFITI RESISTANT 3803 INK 2 .3 AVERY DENNISON 4930 INK 3M PREMIUM PROTECTIVE OVERLAY FILM 1160 2 NIKKALITE BRAND HI - SCALE F-40801 AVERY DENNISON OL - 1000 PREMIUM ANTI - GRAFFITIFILM 3  $\frac{1}{8}$ " THICK ALUMINUM, ALCOA 6016-T6 OR EQUAL

#### INSTALLATION NOTES

WHERE DERAIL IS PROVIDED TO PREVENT FOULING OF ANY TRACK, DERAILING SWITCH NOTICE SHALL BE PLACED ON STAND OF THAT PARTICULAR SWITCH THROUGH WHICH THE FOULING MOVEMENT WOULD BE MADE.

#### MATERIAL NOTES:

- SIGNS SHALL INCLUDE ALUMINUM PANEL, RETROREFLECTIVE SHEETING, POLYURETHANE PAINT, SCREENED-PROCESS COLORS OR FILM, UV PROTECTION OVERLAY, ANTI-GRAFFITI OVERLAY, POSTS, ANCHORS AND
- 2. ALUMINUM PANEL SHALL BE ALCOA 6016-T6 OR EQUAL. 3. TEXT FONT SHALL BE ARIAL BOLD AS PER SCRRA ES1212, SIZE AS
- 4. PANEL SHALL BE PAINTED ON ALL SIDES WITH TWO PART ACRYLIC POLYURETHANE PAINT COATING.
- 5. RETROREFLECTIVE SHEETING SHALL CONFORM TO THE REQUIREMENTS OF ASTM D4956, CLASS IX OR GREATER. RETROREFLECTIVE SHEETING SHALL HAVE CLASS 1, 3, OR 4 ADHESIVE BACKING WHICH SHALL BE PRESSURE SENSITIVE AND FUNGUS RESISTANT.
- 6. SCREENED-PROCESS COLORS AND NONREFLECTIVE, OPAQUE BLACK FILM SHALL HAVE EQUIVALENT OUTDOOR WEATHERABILITY CHARACTERISTICS

AS THE RETROREFLECTIVE SHEETING.

DRAWN BY: A. CARLOS DATE: INVERSE ENUMEREING, STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY.

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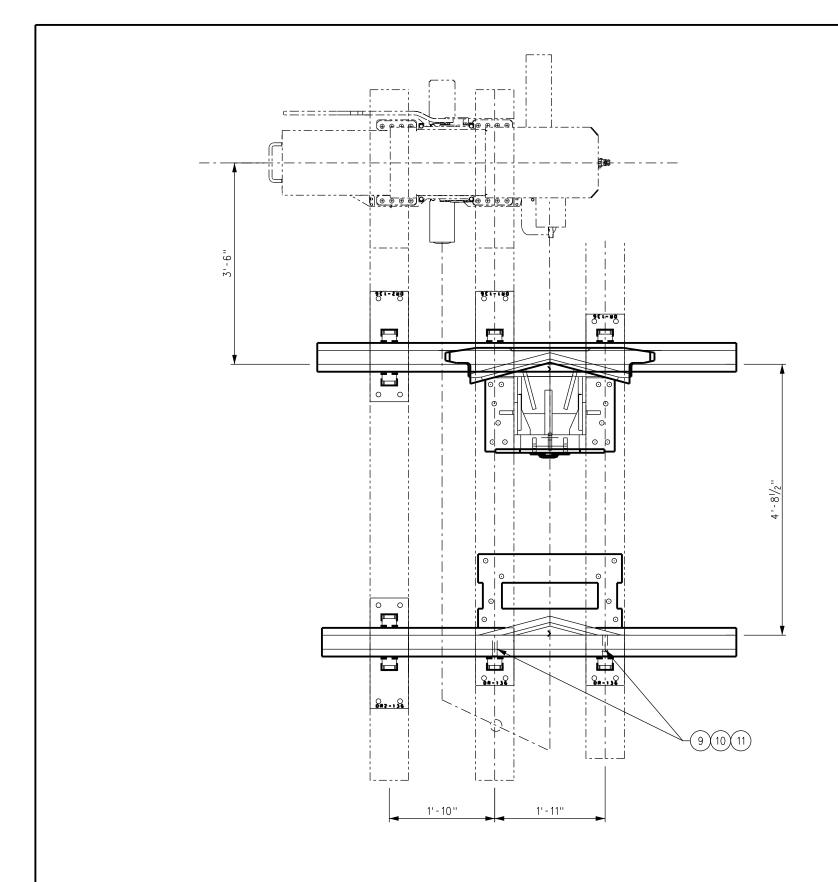
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## **METROLINK**

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA, 90012

**ENGINEERING STANDARDS** NTS DERAIL SWITCH NOTICE 1 OF

ES2612



	BILL OF MATERIAL					
ITEM	QTY	DESCRIPTION	PRODUCT NO	SCRRA PART NO		
1	1	HAYS DERAIL HBXS-8-SS C/W CROWDER	X99-02310			
2	1	RODDING KIT FOR WCH DERAIL FOR USE WITH US&S M23E SW/MACHINE	X99-02314			
3	3	TIE PLATE DR-136	G90-00630			
4	1	TIE PLATE DR1-136	G90-00631			
5	2	TIE PLATE DR2-136	G90-00632			
6	2	TIE HARDWOOD TREATED DAPPED 8" X 10" X 12'-0"	J15-00068			
7	1	TIE HARDWOOD TREATED 8" X 10" X 9'-0"	J15-00069			
8	46	SCREW SPIKE 15/16" X 6"	V50-00010			
9	2	BOLT HEX 1" X 4" GR5	V 01-61010			
10	2	NUT HEVEY HEX 1" GR5	V30-60015			
11	2	WASHER SPRING HEAVY 1"	V35-60217			
12	8	CLIP PANDROL E2055G RH GALVANIZED	X25-00016	·		

#### INSTALLATION REQUIREMENT NOTES:

1. CROWDER WITH SLIDING DERAIL SHOWN. WHEEL CROWDER STROKE IS  $5 \slash\hspace{-0.6em}/_4"$  WITH  $\slash\hspace{-0.6em}/_8"$  DIAMETER PINS.

2. PAINT: SAFETY YELLOW.

3. FOR PROPER THROW OF SWITCH STAND TO DERAIL/CROWDER, ADJUST SWITCH STAND CRANK EYE FOR 51/4" THROW.

MAKE SURE THAT YOUR SWITCH STAND (HEAD BLOCK) TIES

THAT HOLD THE DERAIL ARE HIGH QUALITY. 5. READ THE MANUFACTURER'S INSTRUCTIONS.

6. PLACE THE DERAIL TIGHTLY AGAINST THE RAIL.
7. SPIKE BOTH RAILS TO THE TIES AT THE PROPER GAUGE.
8. FASTEN THE DERAIL AND CROWDER THROUGH ALL THE SCREW

SPIKE HOLES. PRE-DRILL HOLES TO PREVENT THE TIES FROM SPITTING.

HAVE GOOD DRAINAGE AND BALLAST. THE AREA UNDER THE DERAIL MUST BE POCKETED TO PREVENT BINDING IN ADVERSE WEATHER CONDITIONS.

#### INSTALLATION OF CROWDER NOTES:

- PLACE THE WHEEL CROWDER TIGHTLY AGAINST THE WEB OF THE RAIL.

2. RAIL CROWDER MOUNTING BOLT HOLE TO BE MATCH MARKED FROM THE RAIL CROWDER AND DRILLED IN THE FIELD.

3. USE THE WEB SET SCREWS TO ADJUST AND MAINTAIN PROPER WHEEL CROWDER POINT CONTACTS WITH THE RAIL.

WITH BOTH RAIL AND WHEEL CROWDER SECURED AND IN DERAILING POSITION, ATTACH THE CONNECTING ROD TO THE LEFT LUG ON THE DERAIL, THEN CONNECT THE OPPOSITE END OF THE CONNECTING ROD WITH THE TURNBUCKLE INTO THE REVERSING CRANK MECHANISM ON THE BASE OF THE WHEEL CROWDER.

5. ATTACH THE SWITCH STAND CONNECTING ROD OF THE MANUAL OR ELECTRIC

SWITCH STAND TO THE TURNBUCKLE ON THE SWITCH STAND OR ELECTRIC SWITCH STAND. THE OPPOSITE END OF THE CONNECTING ROD CONNECTS TO THE RIGHT HAND LUG ON THE DERAIL. ADJUST THE THROW ON YOUR SWITCH STAND TO A 51/4" THROW. A SHORTER THROW WILL GIVE YOU PRESSURE ON THE CONNECTING ROD OR SWITCH STAND EYE. PRESSURE ON THE EYE AND CONNECTING ROD CAN RESULT IN A FAILURE OF THAT

COMPONENT. ADJUST AS NECESSARY.

6. PLACE COTTER KEYS TO SECURE THE NUTS.

7. INSTALL A SWITCH LOCK.

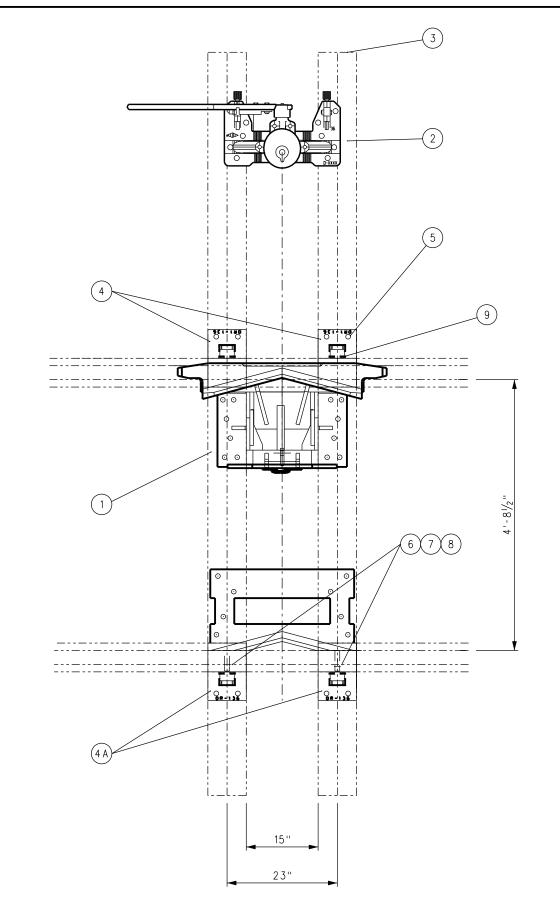
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OF SUCH	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

ENGINEERING STANDARDS	standard 2613
	scale: NTS
BI-DIRECTIONAL DERAIL WITH CROWDER	REVISION SHEET - 1 OF 1
	CADD FILE: ES2613



	BILL OF MATERIAL							
ITEM	QTY	DESCRIPTION	PRODUCT NO	SCRRA PART NO				
1	1	HAYS DERAIL HBXS-8-SS C/W CROWDER	X99-02310					
2	1	36E SWITCH STAND WITH TARGET & BALL HANDLE SCRRA STANDARD TARGET	R36-36094					
3	2	TIE HARDWOOD TREATED, 8" X 12" X 14'-0"	J15-00067					
4	2	TIE PLATE DR1-136						
4 A	2	TIE PLATE DR-136						
5	38	SCREW SPIKE 15/6" X 6"	V50-00010					
6	2	BOLT HEX 1" X 4" GR 5	V 0 1 - 6 10 10					
7	2	NUT HEAVY HEX 1" GR 5	V30-60015					
8	2	WASHER SPRING HEAVY 1"	V35-60217					
9	4	CLIP PANDROL E2055G RH GALVANIZED	X25-00016					

#### INSTALLATION REQUIREMENT NOTES:

1. CROWDER WITH SLIDING DERAIL SHOWN.
WHEEL CROWDER STROKE IS 51/4" WITH 7/8" DIAMETER PINS.
2. PAINT: SAFETY YELLOW.

FAINT SAFETT TELLOW.
 FOR PROPER THROW OF SWITCH STAND TO DERAIL/CROWDER, ADJUST SWITCH STAND CRANK EYE FOR 51/4" THROW.
 MAKE SURE THAT YOUR SWITCH STAND (HEAD BLOCK) TIES THAT HOLD THE DERAIL ARE HIGH QUALITY.

READ THE MANUFACTURER'S INSTRUCTIONS.
PLACE THE DERAIL TIGHTLY AGAINST THE RAIL.
SPIKE BOTH RAILS TO THE TIES AT THE PROPER GAUGE.
FASTEN THE DERAIL AND CROWDER THROUGH ALL THE SCREW SPIKE HOLES. PRE-DRILL HOLES TO PREVENT THE TIES

HAVE GOOD DRAINAGE AND BALLAST. THE AREA UNDER THE DERAIL MUST BE POCKETED TO PREVENT BINDING IN ADVERSE

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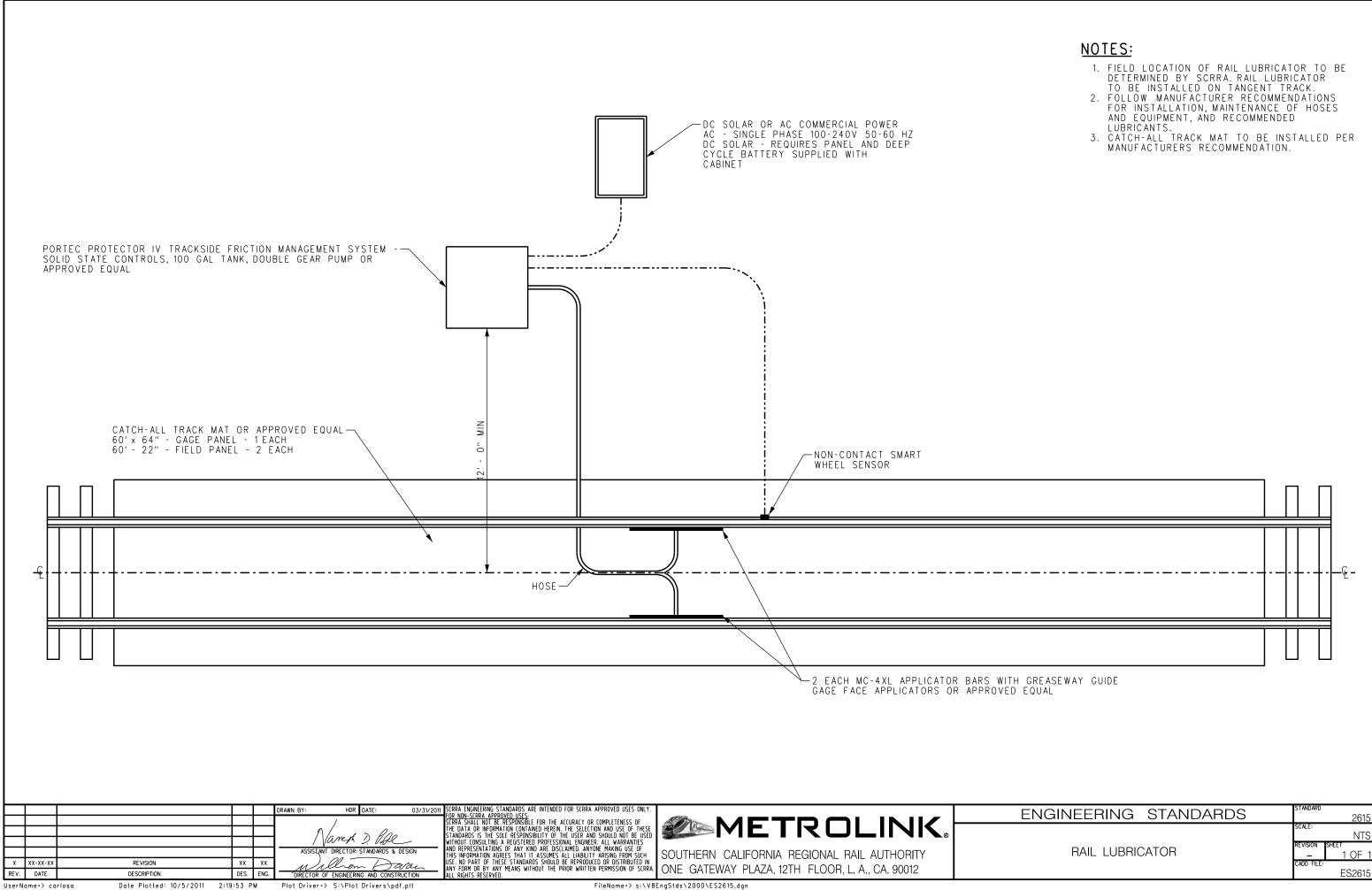
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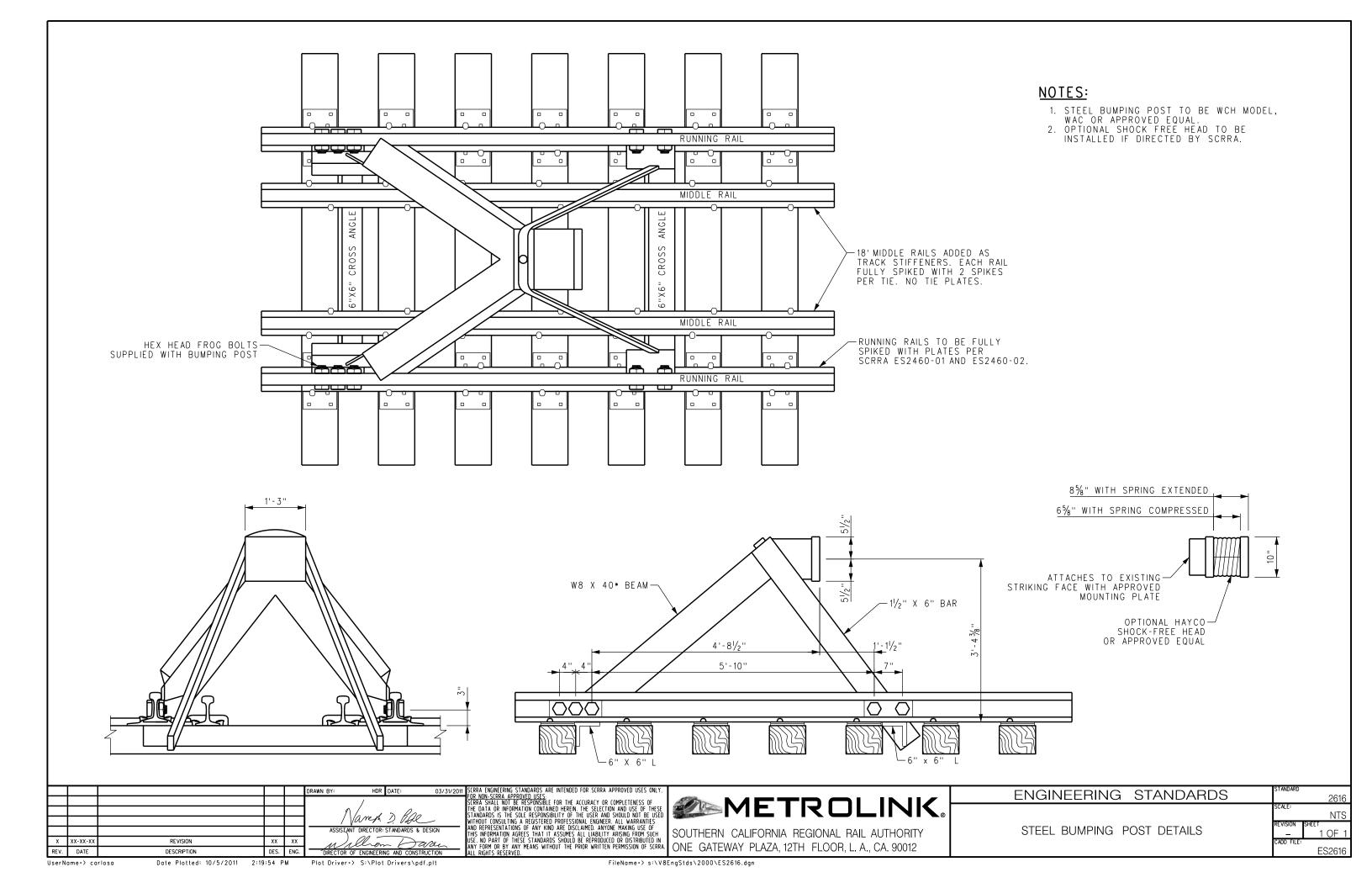
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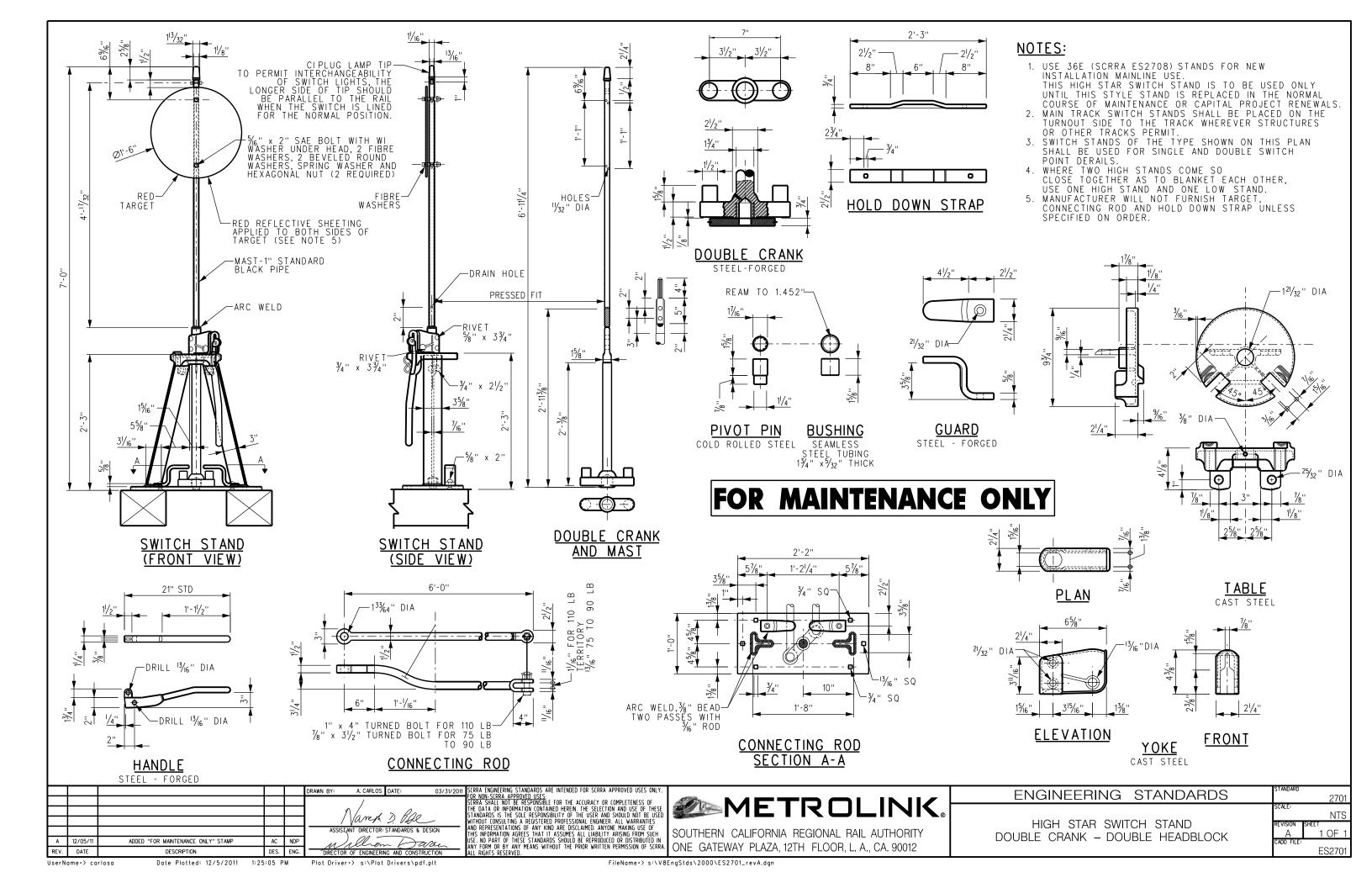
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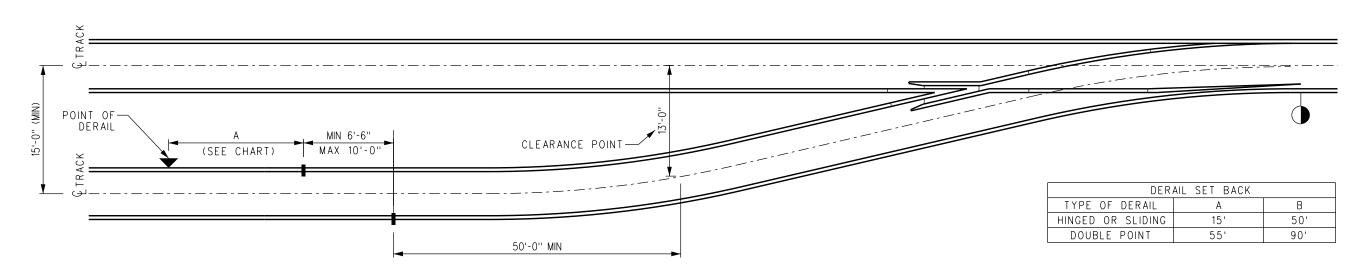
ALIFORNIA REGIONAL RAIL AUTHORITY AY PLAZA, 12TH FLOOR, L. A., CA. 90012

ENGINEERING STANDARDS	2614
	scale: NONE
BI-DIRECTIONAL DERAIL WITH CROWDER WITH 36E SWITCH STAND	REVISION SHEET  - NTS
WITH OOL OWNON ON WE	CADD FILE: ES2614

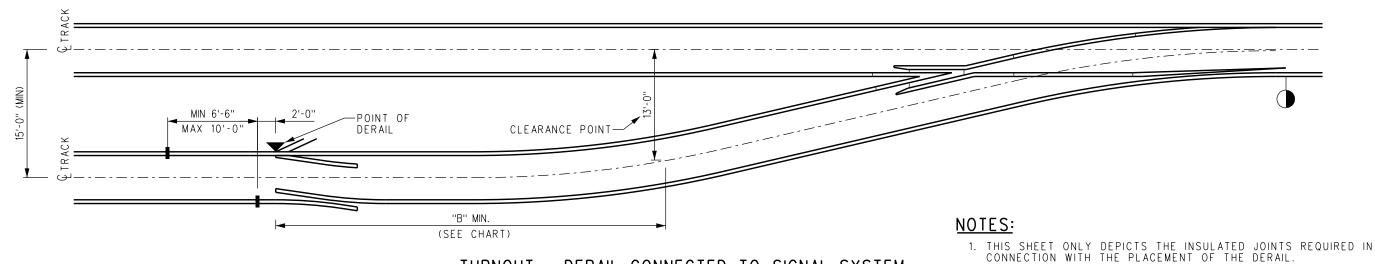








TURNOUT - DERAIL NOT CONNECTED TO SIGNAL SYSTEM - OUTSIDE INSULATED JOINTS



TURNOUT - DERAIL CONNECTED TO SIGNAL SYSTEM

CONNECTION WITH THE PLACEMENT OF THE DERAIL.

SYMBOL" DESIGNATES THE POINT OF DERAIL.

DISTANCE MEASURED BETWEEN END POSTS OF INSULATED JOINTS SHALL BE A MINIMUM OF 6'-6" AND A MAXIMUM OF 10'-0". (FRA REGULATIONS ALLOW 19'-6" STAGGER BUT ABOVE STANDARD SHALL BE APPLIED TO ALL NEW CONSTRUCTION).

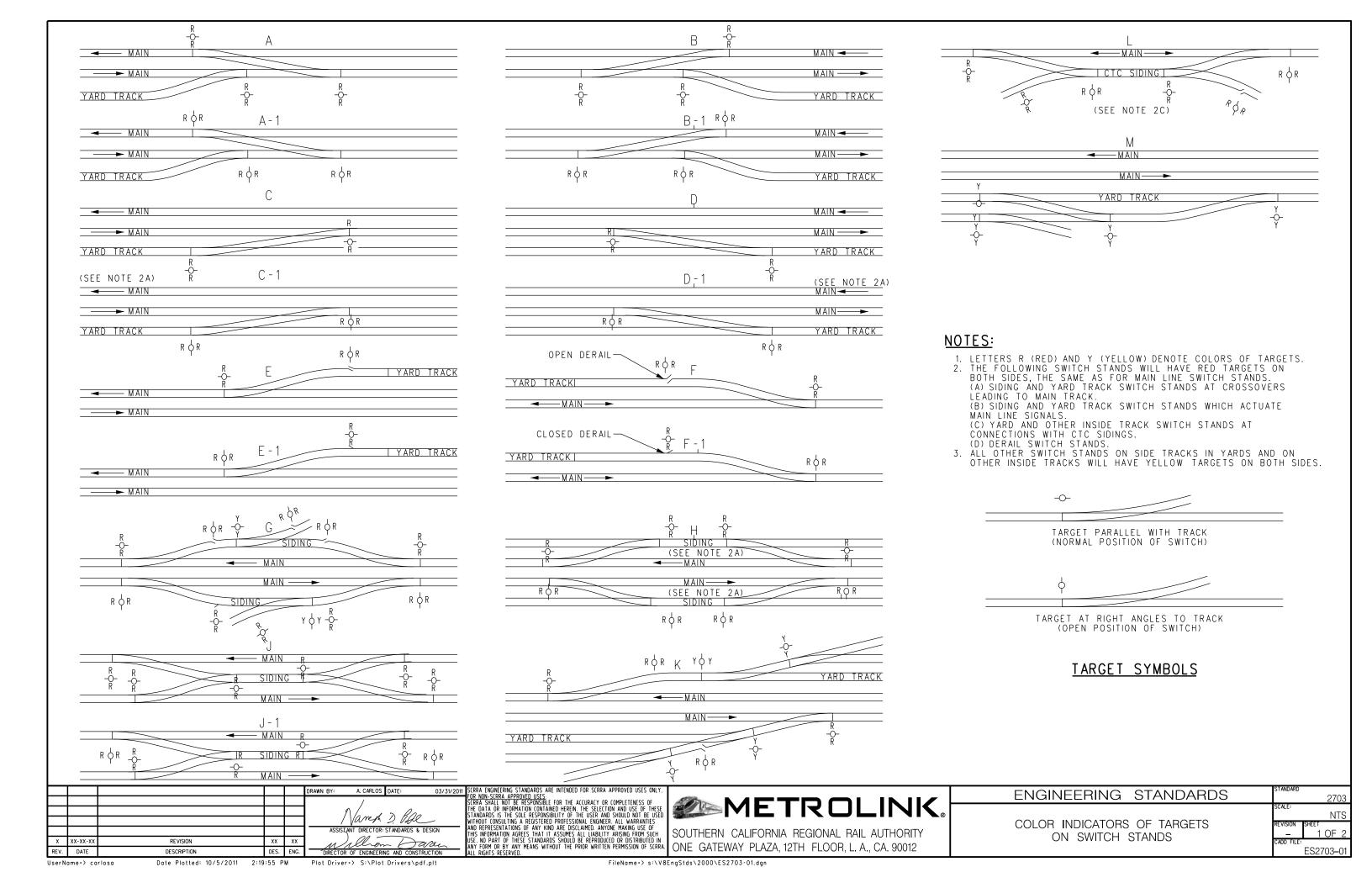
4. DISTANCE FROM CLEARANCE POINT TO INSULATED JOINT SHALL BE A

4. DISTANCE FROM CLEARANCE POINT TO INSULATED JOINT SHALL BE A MINIMUM OF 50'-O".

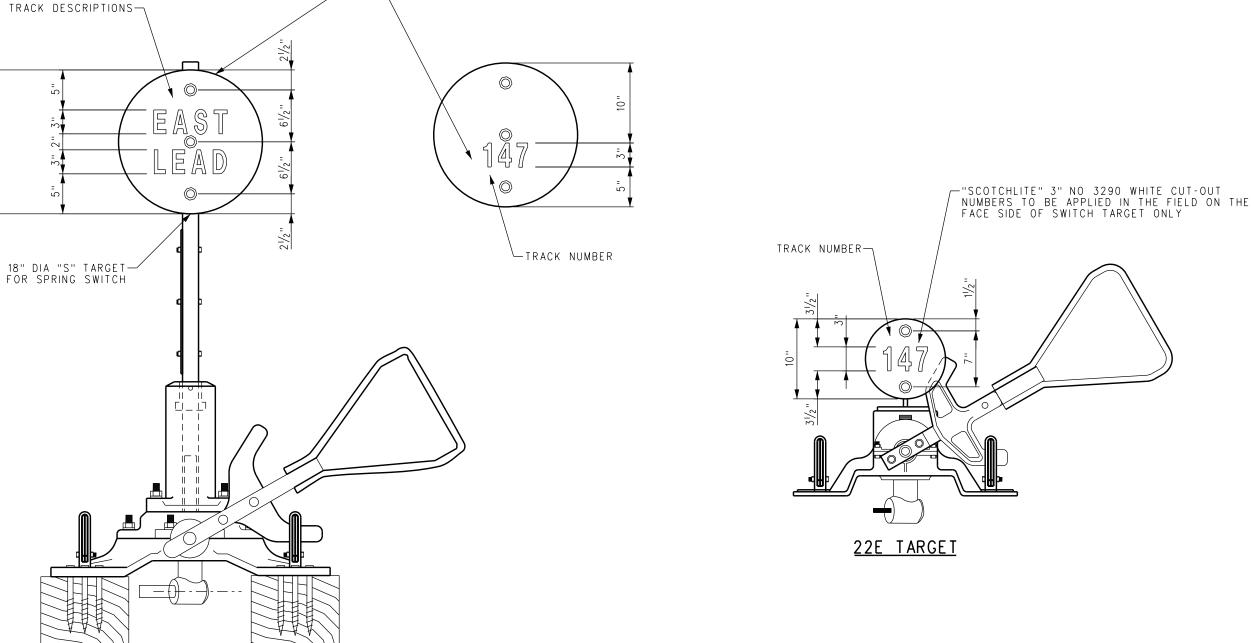
5. SEE ES8220 FOR PLACEMENT OF ALL OTHER NECESSARY INSULATED JOINTS IN CONNECTION WITH TURNOUTS OR OTHER THAN MAIN TRACKS.

6. THE DOUBLE POINT DERAIL WILL BE PLACED ENTIRELY ON TANGENT TRACK (SEE ES2604-01 FOR DOUBLE SWITCH POINT DERAIL DIMENSIONS). CLOSURE CURVES MAY REQUIRE EXTENDING THE DIMENSION "B" LENGTH TO PROVIDE THE NECESSARY TANGENT TRACK LENGTH. THE DIMENSION DEPICTED IN THIS STANDARD IS THE MINIMUM PERMISSIBLE LENGTH.

		DRAWN BY:	II SCRRA ENGINEERING STANDARDS ARE INTENDED FOR SCRRA APPROVED USES ONLY. FOR NON-SCRRA APPROVED USES: SCRRA SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF	METROLINK	ENGINEERING STANDARDS	STANDARD 2702
x xx-xx-xx	REVISION XX XX	Narch D. Pfle  ASSISTANT DIRECTOR: STANDARDS & DESIGN  William Davae	MIICE NO DADT OF THECE CTANDADOS CHOULD BE DEDDODUCED OD DICTDIRUTED IN	SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA, 12TH FLOOR, L. A., CA. 90012	INSULATED JOINT PLACEMENT AND DERAIL LOCATION	REVISION SHEET  - 1 OF 1 CADD FILE:
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TRACK IDENTITY IS TO BE APPLIED TO SWITCH STAND TARGETS IN THE FIELD ONLY AND TARGETS MUST NOT BE ORDERED BEARING ANY TRACK I.D.



- "SCOTCHLITE" 3" NO 3290 WHITE CUT-OUT NUMBERS TO BE APPLIED IN THE FIELD ON THE FACE SIDE OF SWITCH TARGET ONLY

36E TARGET

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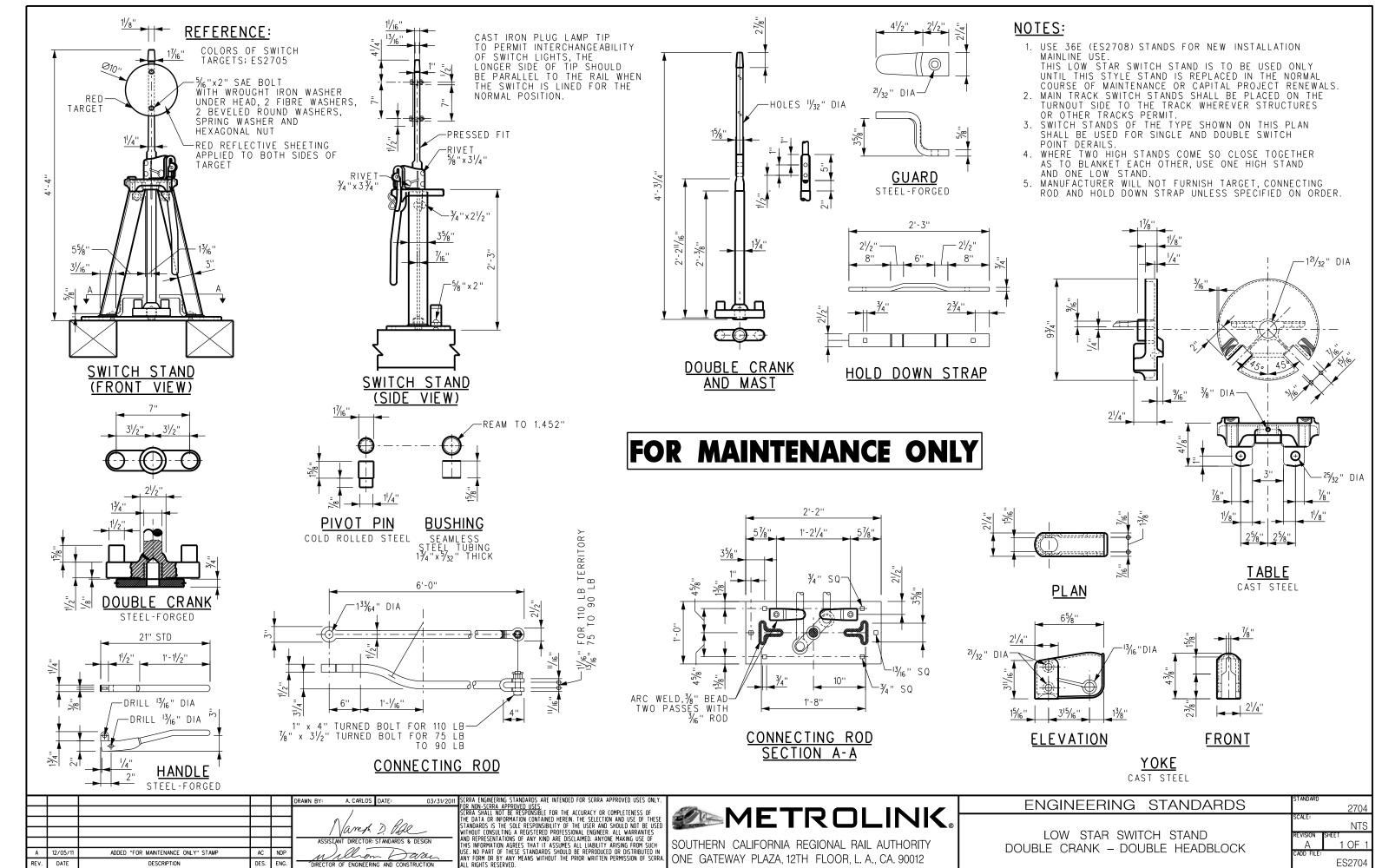
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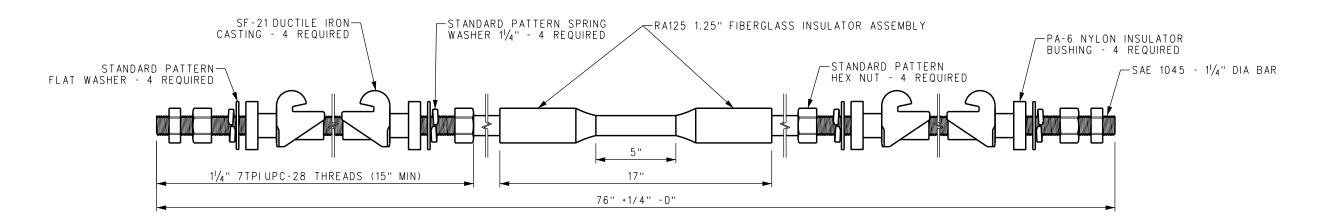
COLOR INDICATORS OF TARGETS ON SWITCH STANDS

ENGINEERING STANDARDS

NTS 2 OF 2 ES2703-02



- 1. ROD SHALL BE SAE1045. THREADS SHALL BE 11/4" 7 UNC-2B.
  2. RAIL ENGAGEMENT FITTINGS (SF-21) SHALL BE OF 60,000 PSI TENSILE, 45,000 PSI YIELD, AND 12% ELONGATION PROPERTIES WITH STANDARD MILL TOLERANCES.
  3. AFTER ASSEMBLING THE RA125, BUFF SMOOTH ALL WRENCH MARKS.
  4. PA-6 INSULATORS ARE POLYMIDE TYPE 6 NYLON. ALL RODS SHALL
- BE SHIPPED ASSEMBLED.
- 5. INCLUDE JAM NUT ON EACH OF ASSEMBLY. 6. CHAMFER ENDS OF ROD BEFORE THREADING.



SPECIAL NOTE TO MANUFACTURER: ALL THREADS TO BE COATED WITH BOSTIK "NEVER-SEIZE".

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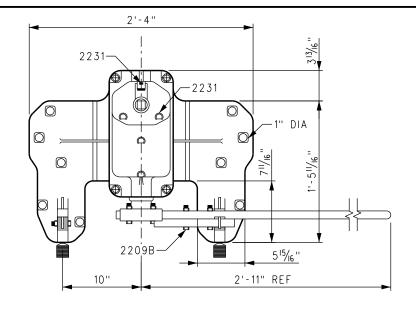
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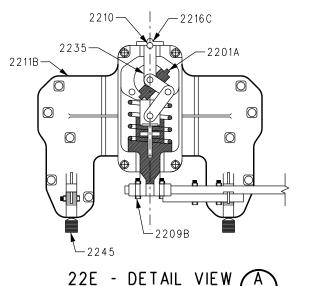
INSULATED GAUGE ROD

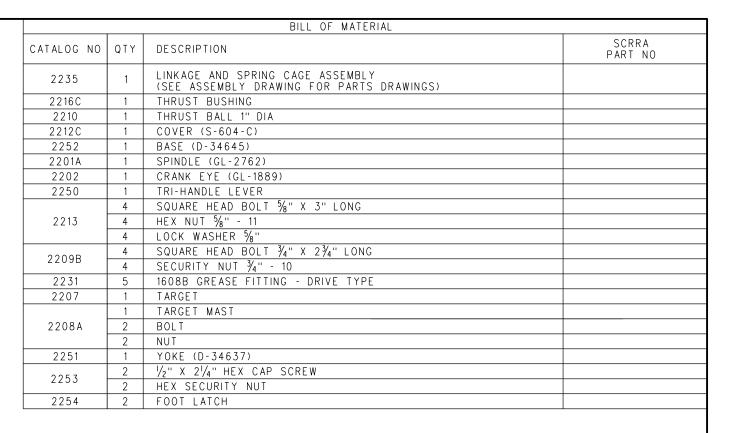
**ENGINEERING STANDARDS** 

NTS 1 OF -ES2706

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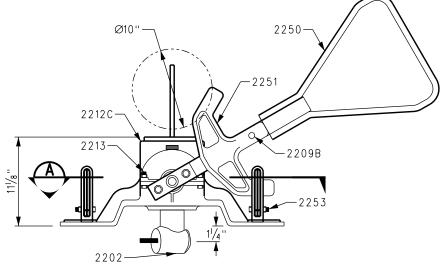






TRACK

#### 22E - TOP VIEW

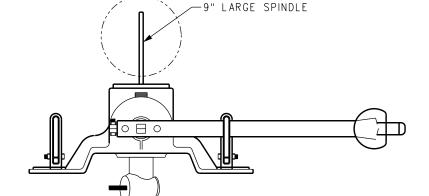




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# 3'-0" 2'-5%' 5" DIA 13/16" DIA ·1//8" SQUARE

#### 22E SWITCH STAND



STAND WITH OPTIONAL 36" STRAIGHT HANDLE

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OPTIONAL 36" STRAIGHT HANDLE (USE FOR TIGHT CLEARANCE ONLY)

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## **METROLINK**

SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

ENGINEERING	STANDARDS	STANDARD 2707
		SCALE:
		NTS
22F SWITCH	CIVVID	REVISION SHEET
ZZL SWITCH	STAND	_ 1 OF 1
		CADD FILE: FS2707

1. 22-E RECOMMENDED USE: YARD AND OTHER THAN MAIN

IRACK

2. IT IS RECOMMENDED THAT SWITCH STANDS BE INSPECTED AND LUBRICATED AT LEAST ONCE A YEAR. ADD OIL IN "OIL CUPS" WITH ANY GOOD GRADE ENGINE OIL. RECOMMEND OIL WITH GRAPHITE CONTENT SAE 60.

3. IF SWITCH STAND IS DISASSEMBLED, REGREASING OF ALL INTERNAL PARTS IS REQUIRED. APPLY GREASE LIBERALLY IN "THRUST BUSHING" CAVITY, BOTH ENDS "SPRING BASE", "SPINDIE" SLOT AND ALL BEARING SURFACES.

SWITCH STAND TO BE INSTALLED USING SCREW SPIKES

FOR CONNECTING ROD ASSEMBLY SEE SCRRA ES2108 6. STRAIGHT HANDLE TO BE PAINTED SAFETY YELLOW.

SEE SCRRA ES2703-01

SEE SCRRA ES2703-02

"SPINDLE" SLOT, AND ALL BEARING SURFACES

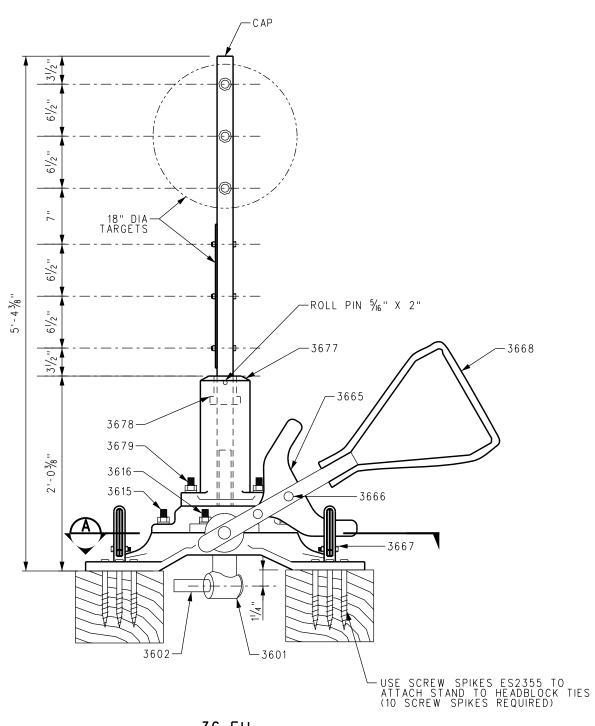
(TEXACO NO 904 GREASE)

FOR SWITCH TARGET DETAILS

FOR TRACK IDENTIFICATION

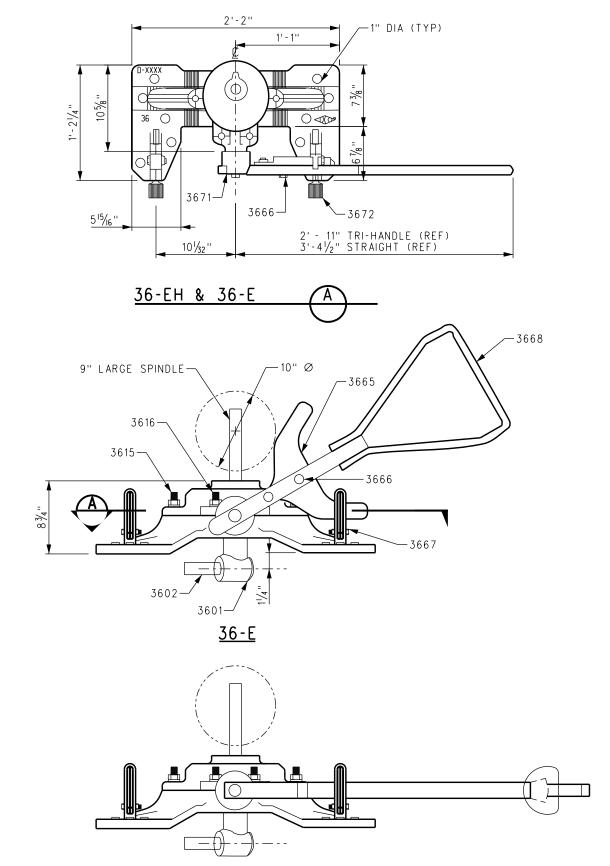
(SCRRA ES2355) FOR SCREW SPIKES

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<u>36-EH</u>

1. FOR BILL OF MATERIALS SEE SHEET ES2708, SHEET 2 OF 2



#### STAND WITH OPTIONAL STRAIGHT HANDLE

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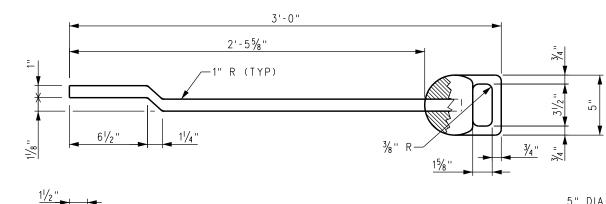
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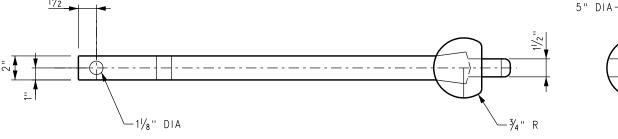
SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY

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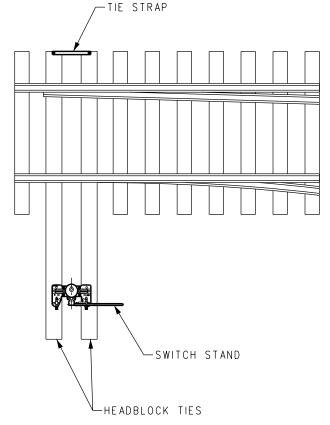
ENGINEERING STANDARDS NTS 36E & 36EH SWITCH STANDS 1 OF 2 ES2708-01

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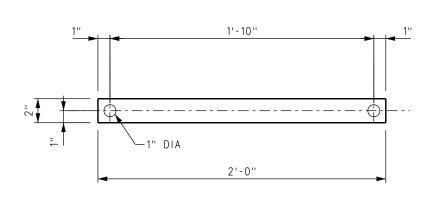












#### TIE STRAP TIE MOUNTING KIT

(12) SCREW SPIKES ES2355 (10 FOR STAND AND 2 FOR STRAP) (1) ½" X 2" X 2' STEEL STRAP

### NOTES:

BILL OF MATERIAL

- SEE ES2708-01 FOR REST OF THE DRAWING.
   36-E RECOMMENDED USE: MAIN TRACK CROSS-OVERS AND YARD TRACKS OR OTHER THEN MAIN
- LINE TRACKS.
- 36-EH RECOMMENDED USE: MAIN TRACK
- 4. FOR MAIN LINE INSTALLATION USE MOUNTING KIT.
  APPLY TIE STRAP ON HEADBLOCK TIES ON
  OPPOSITE SIDE OF TRACK FROM SWITCH STANDS.
  5. LUBRICATE INTERNALLY AT LEAST ONCE A YEAR.
  6. REFERENCE THE FOLLOWING DRAWINGS:
- - -SCREW SPIKES ES2358 -SWITCH TARGET DETAILS ES2703-01 & 02

SCRRA

PART NO

-CONNECTING ROD ASSEMBLY - ES2108

7. STRAIGHT HANDLE TO BE PAINTED SAFETY YELLOW.

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DESCRIPTION

SPINDLE

COVER (S-479)

BASE (D-34678)

CRANKEYE (GL-1889)

SQ HD BOLT 3/4" X 31/4" LONG SQ HD BOLT 3/4" X 43/4" LONG

1/2" X 21/4" HEX CAP SCREW

HEX SECURITY NUT ¾" - 10

HEX SECURITY NUT - 1/2"

HEX NUT HEAVY WFI 1/8" - SLOTTED

SQ HEAD BOLT 1/2" X 21/2" LONG 2" THDS

SPINDLE EXTENSION #66

SPRING WASHER - 1/2'

ANCO HEX NUT - 1/2'

SPRING WASHER 3/4"

FLAT WASHER 11/8" GREASE FITTING GREASE FITTING ROLL PIN 5/6" X 2'

TRI-HANDLE LEVER

HUB (D-34629)

FOOT LATCH

CRANK BUSHING

CRANK WASHER

CRANK GL-1908

STIFFENER S-480

ADAPTER S-481

HEX NUT 1/2"

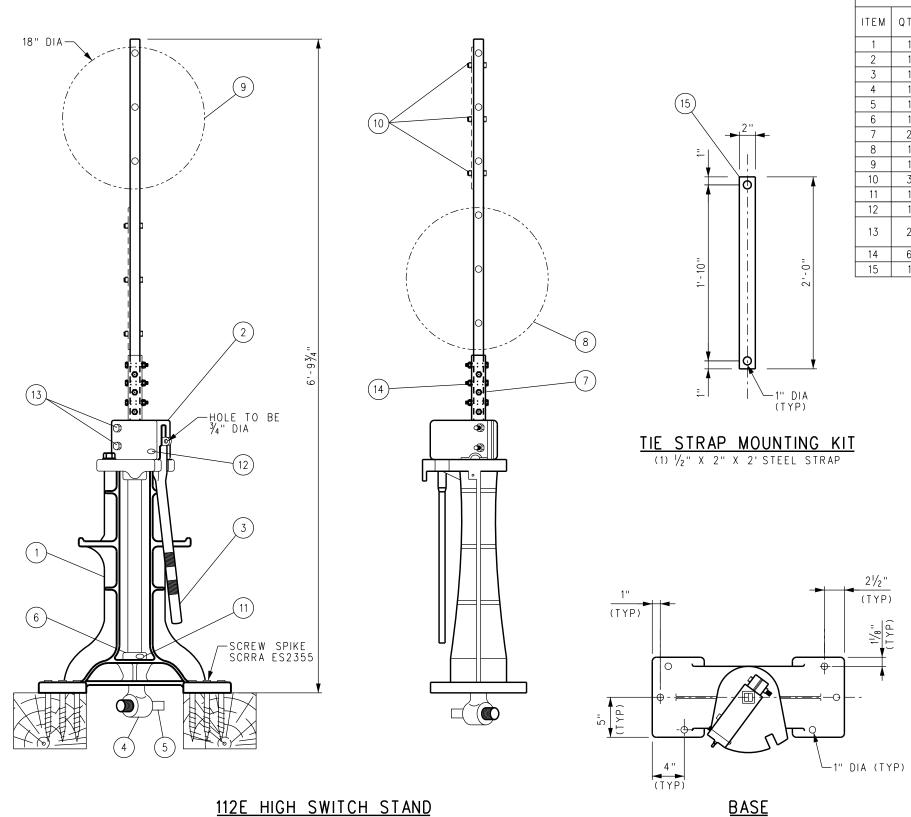
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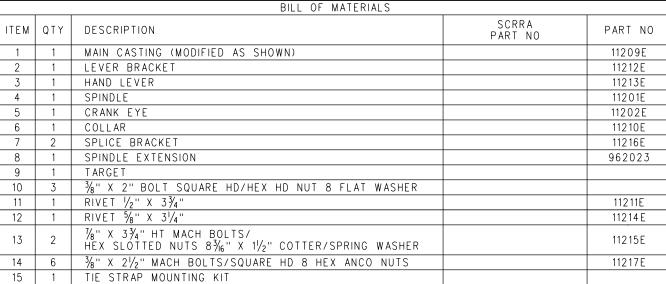
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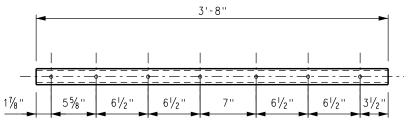
3/4" BUTTON RIVETS

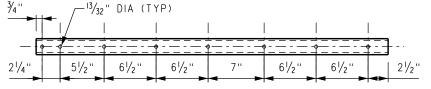
ENGINEERING STANDARDS	STANDARD 2708
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36E & 36EH SWITCH STANDS	REVISION SHEET  - 2 OF 2  CADD FILE:

ES2708-02









 $\frac{1}{6}$ " INSIDE,  $1\frac{3}{6}$ " OUTSIDE SQUARE TUBING

#### TARGET SPINDLE EXTENSION

#### NOTES:

- 1. RECOMMENDED USE, MAIN TRACK FOR SWITCH STAND
- MOUNTING KIT.

- APPLY TIE STRAP ON HEADBLOCK TIES ON OPPOSITE SIDE OF TRACK FROM SWITCH STANDS.

  3. MINIMUM CONNECTING ROD LENGTH IS 6'-0¾".

  4. FOR SWITCH TARGET DETAILS: SCRRA ES2703-01 FOR TRACK IDENTIFICATIONS: SCRRA ES2703-02 FOR CONNECTING ROD ASSEMBLY: SCRRA ES2108

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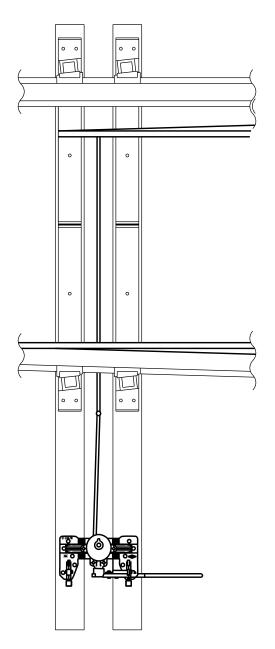
ENGINEERING STANDARDS NTS 112E HIGH SWITCH STAND 1 OF ES2709

CRANKEYE AND CONNECTING ROD CLEVIS SHOULD BE GREASED PRIOR TO INSTALLATION OR ADJUSTMENT. START WITH ABOUT 1" OF THREADS SHOWING ON CONNECTING ROD TURNBUCKLE (TIGHTEN JAM NUT)





MEASURE THROW BETWEEN SWITCH POINT & STOCK RAIL AT FIRST ROD

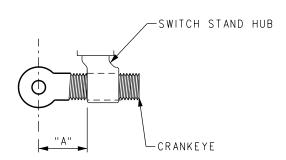


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SET CRANKEYE SETTING AT DISTANCE "A" FOR MEASURED OPENING AND CORRECT STAND

	'' A''	"A"
THROW OF SWITCH	RACOR 22E	RACOR 36E
4 1/2 "	21/16"	2 %6''
4 5/8"	2 3/6"	2 11/16 "
4 3/4"	2 1/4 "	2 3/4 "
4 1/8 "	2 1/6"	2 13/16 ''
5"	2 7/16 "	215/16"
51/8"	21/2"	3"
51/4"	25/8"	31/8"
5 3/8 "	211/16"	33/6"
5½" 5¾"	2 13/16 ''	3 <sup>5</sup> / <sub>16</sub> ''
5 5/8"	-	3 3/8 "
5 3/4"	-	3 7/16 "

"A" WORKS FOR ALL ROD LENGTHS



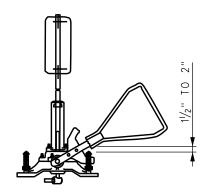
RACOR 22E & 36E STANDARD SIDE FOR CRANKEYE

#### NOTES:

- 1. USE 15/6" SCREW SPIKES OR APPROPRIATE
  PIM SCREWS WHEN INSTALLING NEW SWITCH
  STANDS ON TIMBER OR CONCRETE TIES.
  2. FIELD INSPECTION OF STAND IS RECOMMENDED
  AT LEAST ANNUALLY OR MORE WHERE STAND
- IS USED FREQUENTLY.
  OIL CUPS: USE SAE 40, ADD OIL FREQUENTLY.
  GREASE SHOULD BE LG312 LITHIUM GRADE 2.
  REGREASING OF ALL INTERNAL PARTS IS RECOMMENDED BEFORE REASSEMBLY AFTER INSPECTIONS.
- DIFFERENCES BETWEEN CRANKEYE MEASUREMENTS ON THIS DRAWING AND FINAL ADJUSTMENTS ARE PROBABLY DUE TO TOLERANCES (LOST MOTION) IN CONNECTING ROD/HEAD ROD CONNECTIONS.

MOVE SWITCH POINTS TO HALF-THROWN POSITION (OPENING EQUAL ON BOTH SIDES). AND STAND LEVER IN VERTICAL POSITION. CENTER STAND ON HEADBLOCK TIES AND SPIKE OR LAG TO TIES.

HAND THROW SWITCH TO BOTH SIDES SEVERAL TIMES. WHEN POINT CONTACTS STOCK RAIL. LEVER SHOULD NOT BE MORE THAN 11/2" TO 2" ABOVE FINAL POSITIONS ON TOP OF LEVER REST FOR BOTH POSITIONS.



#### ELEVATION

IF NOT, ADJUST AS FOLLOWS:

WHEN NEAR POINT FITS PROPERLY AND FAR POINT IS TOO TIGHT: SHORTEN CRANKEYE SETTING AND SHORTEN CONNECTING ROD CLEVIS.

WHEN NEAR POINT FITS PROPERLY AND FAR POINT IS LOOSE: LENGTHEN CRANKEYE SETTING AND LENGTHEN CONNECTING ROD CLEVIS.

WHEN FAR POINT FITS PROPERLY AND NEAR POINT IS TOO TIGHT: SHORTEN CRANKEYE SETTING AND LENGTHEN CONNECTING ROD CLEVIS.

WHEN FAR POINT FITS PROPERLY AND NEAR POINT IS LOOSE: LENGTHEN CRANKEYE SETTING AND SHORTEN CONNECTING ROD CLEVIS.

WHEN BOTH POINTS ARE TIGHT: SHORTEN CRANKEYE SETTING AND DO NOT CHANGE CONNECTING ROD CLEVIS.

WHEN BOTH POINTS ARE LOOSE: LENGTHEN CRANKEYE SETTING AND DO NOT CHANGE CONNECTING ROD CLEVIS.

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REV.	DATE	DESCRIPTION	DES.	ENG.

DRAWN BY: 03/31/2011 Jares D. Page ASSISIANT DIRECTOR: STANDARDS & DESIGN Mor Davan

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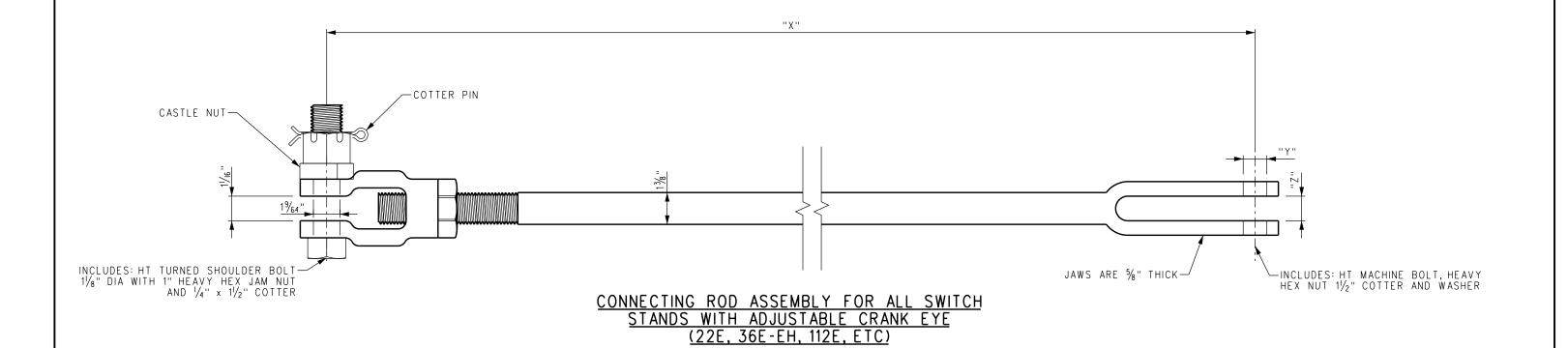
SOUTHERN CALIFORNIA REGIONAL RAIL AUTHORITY ONE GATEWAY PLAZA. 12TH FLOOR. L. A., CA. 90012

SWITCH STANDS HAND THROW ADJUSTMENTS AND INSTALLATION INSTRUCTIONS

ENGINEERING STANDARDS

NTS 1 OF CADD FILE ES2710

2710



"Y" - 11/64" FOR 3/4" AND 1" HEAD RODS - 1%4" FOR 11/4" HEAD RODS

"Z" - <sup>13</sup>/<sub>16</sub>" FOR <sup>3</sup>/<sub>4</sub>" HEAD RODS - 11/16" FOR 1" HEAD RODS - 15/6" FOR 11/4" HEAD RODS

RAIL SIZE	"X"	HEAD ROD THICKNESS
90-115 LB	3'-4"	1"
132-136 LB	3'-4"	11/4"
90-115 LB	5'	1"
132-136 LB	5'	11/4"
90-115 LB	7'	1"
132-136 LB	13'-9"	11/4"
132-136 LB	7'	11/4"

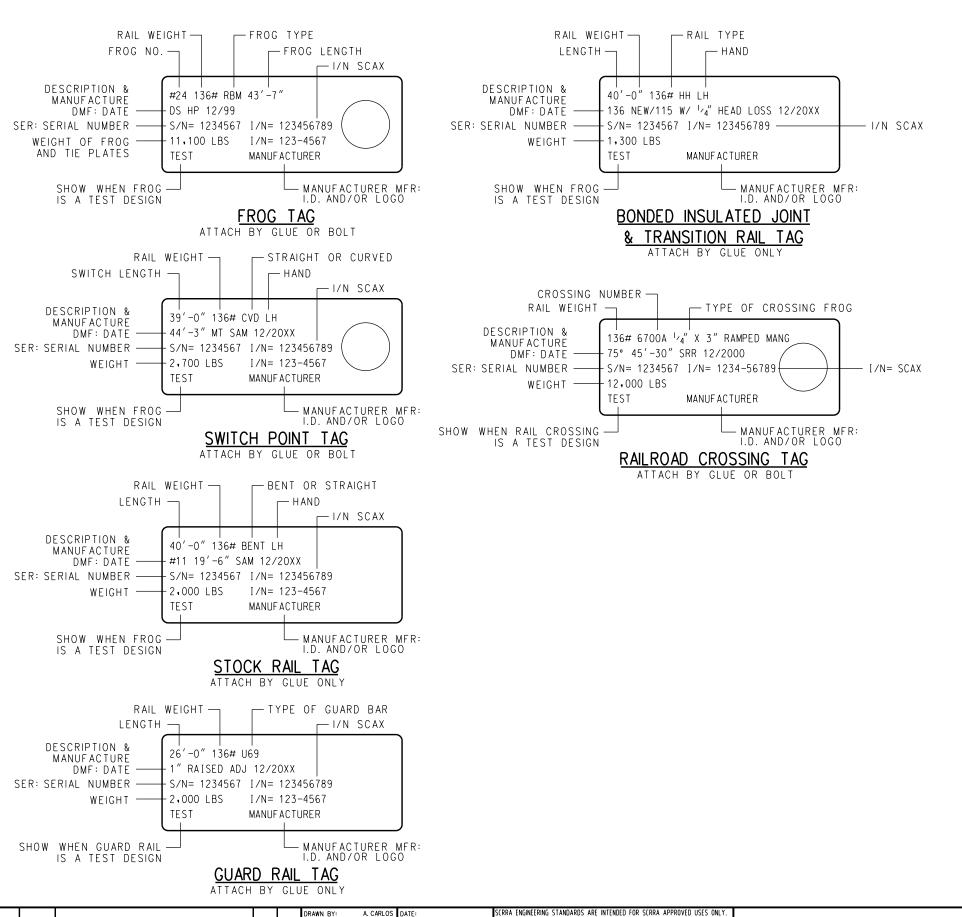
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ENGINEERING STANDARDS	STANDARD 2712
	scale: NTS
CONNECTING ROD ASSEMBLY	REVISION SHEET - 1 OF 1
	CADD FILE: ES2712



#### **ABBREVIATIONS:**

#### **TURNOUT FROG:**

COMMON STANDARD HEEL CS DS HP MPF RBM SAM SPR SSG WHM TH = DIRECT SUPPORT = HEAVY POINT = MOVEABLE POINT FROG = RAIL BOUND MANGANESE = SAMSON

SPRING

OR SMSG = SOLID SELF GUARDED = WELDED HEEL MANGANESE = TAPERED HEEL

#### STOCK RAIL:

STR

SAM SPR

CVD OR CV = CURVED = SAMSON = STRAIGHT

**MISCELLANEOUS:** 

= CURVED

= STRAIGHT

SAMSON = ŠPRING

MANGANESE TIP

#### CROSSING FROG:

= 1 RAIL BOLTED = 2 RAIL BOLTED = 3 RAIL BOLTED P-RAII 3-RAIL = ARTICULATED MANGANESE FB LBM OWLS MI RM

= FLANGE BEARING = LAPPED BEAM MANGANESE = ONE WAY LOW SPEED = MANGANESE INSERT = REVERSIBLE MANGANESE

= SOLID MANGANESE = STRAIGHT RAIL REVERSIBLE

#### = LH = RH

HAND:

SWITCH:

CVD OR CV

= ADJUSTABLE = DEEP HEAD HARDENED = HOOK FLANGE DHH OR HH HF S/N STD = SERIAL NUMBER = STANDARD

- ITEM NUMBER

#### NOTES:

1. TAGS TO BE MADE OF 0.025 THICK

STAINLESS STEEL PLATE.

2. LETTERS TO BE RAISED OR INDENT PUNCH %" HIGH WITH %6" SPACE, BAR CODE ½" HIGH. BETWEEN LIÑES AND AT TOP AND BOTTOM OF I.D. TAG. LETTERS TO BE CLEARLY LEGIBLE FROM A DISTANCE OF SIX FEET.

3. ACTUAL SIZE OF I.D. TAG MAY VARY WITH AMOUNT OF INFORMATION REQUIRED.

4. USE ABBREVIATIONS AS SHOWN ABOVE 5. I.D. TAG TO BE APPLIED WITH PERMANENT EPOXY ADHESIVE. WIPE OFF EXCESS EPOXY.

IDENTIFICATION TAGS FOR

**ENGINEERING STANDARDS**  $1\frac{1}{6}$ " = 1'-0" 1 OF TRACK COMPONENTS ES2715

**METROLINK** 

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DESCRIPTION

0 02/29/12

REV. DATE

DES. ENG.

Varet D. Pope

ASSISIANT DIRECTOR: STANDARDS & DESIGN

Illiam Darae